# PLANNING COMMISSION'S LAND DEVELOPMENT & TRANSPORTATION COMMITTEE

January 12, 2023 – Continued from  $\rightarrow$  November 10, 2022

## CASE # 22-ZONEPA-0017

Change in Zoning: R4, Single-Family -> R6, Multi-Family

### **Property Owner**

Hubert L. Hester Living Trust

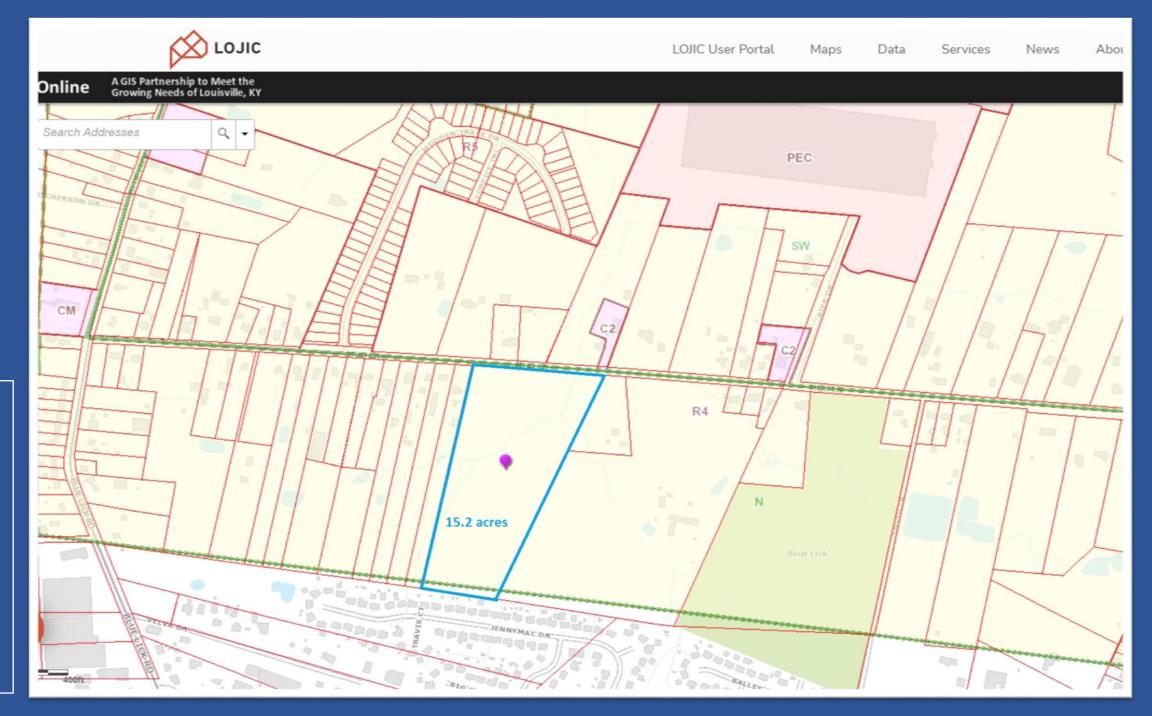
### **REPRESENTATIVES**

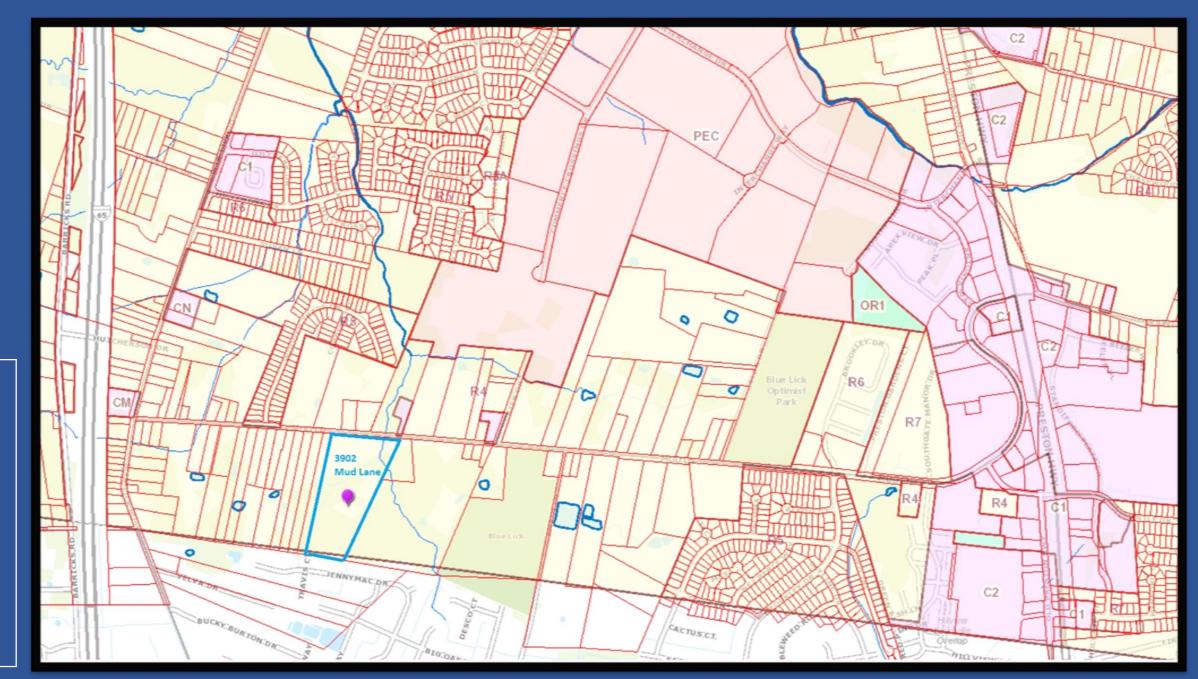
JON BAKER – WYATT, TARRANT & COMBS

DEREK TRIPLETT – LAND DESIGN & DEVELOPMENT, INC.















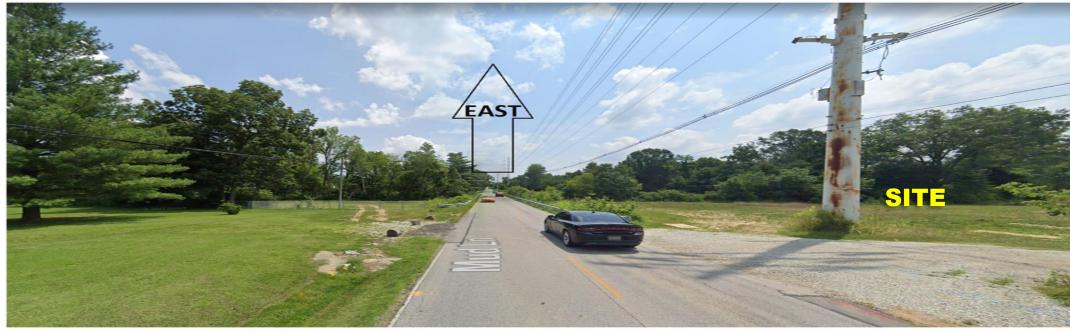














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# SITE RENDERING



# Traffic Impact Study Report 3902 Mud Lane Apartments

Louisville, Jefferson Co., KY

### Prepared For:

Hubert L. Hester Trust

### Prepared By:



adam kirk engineering

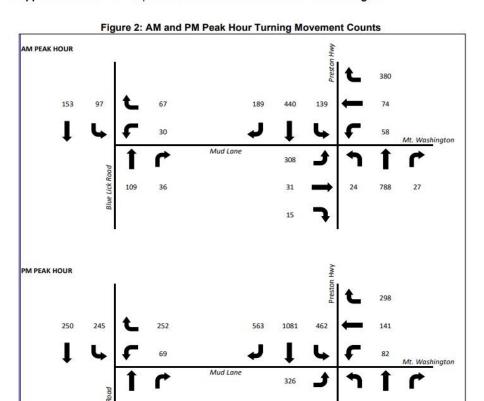
Adam Kirk Engineering 137 McClelland Springs Drive Georgetown, KY 40324 859.421.2567 adam@adamkirkpe.com

October 7, 2022 Revised January 9, 2023

### **EXISTING CONDITIONS**

Mud Lane is a two-lane roadway with a posted speed of 35 mph. East of Cody Lane Mud Lane is widened to a 3-lane section with a center left turn lane. Significant residential development to the south exists with access to the east of the proposed site, at Cody Lane and Brookley Drive. The intersection of Mud Lane at Blue Lick Road is a 'T' intersection with stop control on Mud Lane. The intersection of Mud Lane at Preston Highway is signal controlled and aligns with Mt. Washington Road to the east. Left and right auxiliary turn lanes are present on Mud Lane at the intersection.

AM and PM turning movement counts were collected on Thursday September 29, 2022 between 7-9 a.m. and 4-6 p.m. at the study intersections. Full turn movement count data is provided in **Appendix B**. AM and PM peak hour traffic volumes are summarized in **Figure 2**.



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Trip Generation was conducted in accordance with the ITE Trip Generation Web Based App, 11<sup>th</sup> edition. Trip Generation utilized **ITE Land Use Code 221Multi-family Housing (mid-rise)**. Based on this land use and the proposed 252 multi-family homes, the development is expected to generate 99 vehicles per hour during both the AM and PM peak hour of the adjacent street traffic,. **Table 1** summarizes the trip generation and **Appendix C** contains output from the ITE Trip Generation Manual. No reductions for pass-by trips or internal trip capture trips were made.

	ITE Land Use Code	Land Use Description	Ind. Var. (X)	Ind. Var. Units	Entering/ Exiting	AM Trips Generated	PM Trips Generated
		Multi-Family			Total	99	99
	221	Residential (mid	252	dwelling units	entering	23	60
L		rise)		driito	exiting	76	39

### TRAFFIC FORECASTING

Historic traffic counts were available for Mud Lane at station 056283, which is immediately east of the proposed access and east of Cody Lane and Brookley Drive. Based on this data, historic traffic patterns indicate a growth rate of 2.5 percent per year on Mud Lane. Historic traffic counts on Preston Highway were also reviewed and identified a growth rate of -0.36, traffic volumes on Preston Highway were assumed to remain constant for future year analysis. **Appendix D** contains the historic traffic data and output from the KYTC Traffic forecasting spreadsheet. 2023 No Build and 2033 No Build traffic volumes are summarized in **Figures 3 and 4**.

3902 Mud Lane Apartments Traffic Impact Study

### TRIP DISTRIBUTION METHODOLOGY

Generated trips were distributed onto the roadway network based on recorded travel patterns on Mud Lane as identified through the traffic data collection detailed above. Existing traffic patterns indicated 80% of traffic was to/from the east (towards Preston Highway) during both the AM peak period, and 65 percent of traffic to/from the east during the PM peak period. Traffic destined to the west towards Blue Lick Road was assigned to the western access (Access Point 1) and the remaining traffic to the eastern access (Access Point 2) Trip distribution is shown in **Figure 5**. **Figures 6 and 7** show the final build traffic volumes for 2023 and 2033.

# Updated Traffic Distribution



3902 Mud Lane Apartments Traffic Impact Study

### TRIP DISTRIBUTION METHODOLOGY

Generated trips were distributed onto the roadway network based on recorded travel patterns and discussions with Planning and Design Staff. A final distribution of 70 percent of traffic to/from Preston Highway and 30 percent of traffic to/from Blue Lick Road was used for both the AM and PM peak periods. Traffic destined to the west towards Blue Lick Road was assigned to the western access (Access Point 1) and the remaining traffic to the eastern access (Access Point 2) Trip distribution is shown in Figures 5 and 6. Figures 6 and 7 show the final build traffic volumes

### **CAPACITY ANALYSIS**

Capacity analysis for the existing, no build and build scenarios was completed for the study intersection during the AM and PM peak hours using HCM methodologies as applied Synchro Capacity Software version 10. **Table 1** summarizes the LOS, and delay for the No Build and Build scenarios. Full capacity analysis output is provided in **Appendix E**.

Table 1: Capacity Analysis Summary (Opening Year 2023)

		AM	Peak		PM Peak					
Intersection/Movement	No Build		Build		No Build		Build			
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay		
Mud Lane at Blue Lick Road										
Westbound	В	11.1	В	11.3	E	44	E	48.5		
Southbound (Left Turn)	Α	3.4	Α	3.5	Α	5.7	Α	5.8		
Mud Lane at Access 1										
Northbound	-	-	В	10.0	-	-	В	14.7		
Westbound (Left Turn)	-	-	Α	0.0	-	-	Α	0		
Mud Lane at Access 2										
Northbound	-	-	Α	9.3	-	-	В	10.7		
Westbound (Left Turn)	-	-	Α	1.3	-	-	Α	1.2		
Mud Lane at Preston Highway	/									
Intersection	D	39.8	D	41.7	D	53.9	E	55.7		
Eastbound	Е	61.5	E	62.3	F	87.5	F	89		
Westbound	E	58.3	E	59.1	E	71.3	E	73.9		
Northbound	С	31.6	С	34.0	Е	58.0	E	59.3		
Southbound	С	26.3	С	27.4	D	38.7	D	40.3		

Table 2: Capacity Analysis Summary (Design Year 2033)

		AMI	Peak		PM Peak					
Intersection/Movement	No Build		Build		No Build		Build			
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay		
Mud Lane at Blue Lick Road										
Westbound	В	12.2	В	12.5	F	214.5	F	238.4		
Southbound (Left Turn)	Α	3.6	Α	3.6	Α	6.6	Α	6.7		
Mud Lane at Access 1										
Northbound	-	-	В	10.4	-	-	С	17.1		
Westbound (Left Turn)	-	-	Α	0.0	-	-	Α	0		
Mud Lane at Access 2										
Northbound	-	-	Α	9.5	-		В	11.3		
Westbound (Left Turn)	-	-	Α	1.1	-		Α	1.1		
Mud Lane at Preston Highway	,									
Intersection	D	39	D	45.1	E	62.7	E	65.3		
Eastbound	E	66.6	E	63.8	F	92.4	F	100.1		
Westbound	D	53.8	E	61.0	E	79.7	F	81.1		
Northbound	С	29.4	D	38.7	E	70.3	E	73.5		
Southbound	С	24.2	С	29.8	D	46.2	D	47		

### **CAPACITY ANALYSIS**

Capacity analysis for the existing, no build and build scenarios was completed for the study intersection during the AM and PM peak hours using HCM methodologies as applied Synchro Capacity Software version 10. **Table 1** summarizes the LOS, and delay for the No Build and Build scenarios. Full capacity analysis output is provided in **Appendix E**.

Table 1: Capacity Analysis Summary (Opening Year 2023)

Table I	. Gapac		5-32	mary (Ope	oning re	700000				
		AM P	'eak		PM Peak					
Intersection/Movement	No Build		Build		No Build		Build			
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay		
Mud Lane at Blue Lick Road						10		dr.		
Westbound	В	11.1	В	11.4	E	49.6	F	64.2		
Southbound (Left Turn)	Α	3.4	Α	3.5	Α	5.7	Α	5.9		
Mud Lane at Access 1										
Northbound	12	- 42	В	10.1	<u>101</u>		В	14.9		
Westbound (Left Turn)	14	2	Α	0.0	12	-	Α	0		
Mud Lane at Access 2			te:							
Northbound			Α	9.3		-	В	10.7		
Westbound (Left Turn)	-	- 3	Α	1.0	-	-	Α	0.9		
Mud Lane at Preston Highway		100	i i	181		700	-	800		
Intersection	D	39.8	D	41.7	D	53.9	E	55.7		
Eastbound	E	61.5	E	62.3	F	87.5	F	89		
Westbound	E	58.3	E	59.1	E	71.3	Е	73.9		
Northbound	С	31.6	С	34.0	E	58.0	E	59.3		
Southbound	С	26.3	С	27.4	D	38.7	D	40.3		

Table 2: Capacity Analysis Summary (Design Year 2033)

		AM F	Peak		PM Peak					
Intersection/Movement	No Build		Build		No Build		Build			
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay		
Mud Lane at Blue Lick Road										
Westbound	В	12.3	В	12.8	F	261.9	F	319.2		
Southbound (Left Turn)	Α	3.6	Α	3.7	Α	6.6	Α	6.8		
Mud Lane at Access 1			70.00					All		
Northbound	15		В	10.5	17		С	17.5		
Westbound (Left Turn)	- 2	- 2	Α	0.0	-	-	Α	0		
Mud Lane at Access 2				41	9 -	-31¢ y	91			
Northbound	-	1.5	Α	9.5	-	-	В	11.4		
Westbound (Left Turn)			Α	0.9		-	Α	0.8		
Mud Lane at Preston Highway						99		in the second		
Intersection	D	39	D	45.1	E	62.7	E	65.3		
Eastbound	E	66.6	E	63.8	F	92.4	F	100.1		
Westbound	D	53.8	E	61.0	E	79.7	F	81.1		
Northbound	С	29.4	D	38.7	E	70.3	E	73.5		
Southbound	С	24.2	С	29.8	D	46.2	D	47		

2023 Capacity Analysis									
Intersection	Movement	Exisiting (TWSC)		Build (TWSC)		Exisiting (AWSC)		Build (AWSC)	
		LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
AM Peak Hour									
	Intersection	-	-	-	-	-	*	2	-
Blue Lick Road at	westbound	В	11.1	В	11.4	Α	8.2	Α	8.5
Mud Lane	northbound					Α	8.4	Α	8.5
	southbound (left turn)	Α	3.4	Α	3.5	Α	9.7	В	10
PM Peak Hour									
	Intersection	-		-	3-0	-			
Blue Lick Road at	westbound	Ε	49.6	F	64.2	С	16.6	С	18.8
Mud Lane	northbound			0		С	16.5	С	18.5
EXT 91-8 (170 F20 F20 F20 F20 F20 F20 F20 F20 F20 F2	southbound (left turn)	Α	5.7	Α	5.9	D	34.3	E	46
2033 Capacity Analysis									
Internation .	100000000000000000000000000000000000000	Exisitin	g (TWSC)	Build (TWSC)		Exisiting (AWSC)		Build (AWSC)	
Intersection	Movement	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
AM Peak Hour									
	Intersection	-	-	-	-	-	-	-	-
Blue Lick Road at	westbound	В	12.3	В	12.8	А	8.8	Α	9.1
Mud Lane	northbound					Α	9	Α	9.2
	southbound (left turn)	Α	3.6	Α	3.7	В	11	В	11.3
DAA Daak Hawa	# O.T.		:/			197	tille sales	1. T.	
PM Peak Hour	Intersection	-	-	-	775	-	-		
PIVI PEAK HOUR	intersection		1	_	319.2	D	27.3	D	29.5
Blue Lick Road at	westbound	F	261.9	F	319.2		27.3	-	25.5
SE		F	261.9	F	319.2	D	29.5	D	30.9

Alternative analysis was conducted for the intersection of Blue Lick Road and Mud Lane to evaluate if an All-Way Stop Control (AWSC) would improve delay at the intersection. **Table 3** summarizes this analysis. This analysis showed that AWSC would decrease delay on Mud Lane, but would also increase delay on the previously uncontrolled approaches of Blue Lick Road. As Blue Lick Road (KY 1450) is a state route, modifications and alternatives at this intersection should be coordinated with KYTC.

### **TURN LANE WARRANT ANALYSIS**

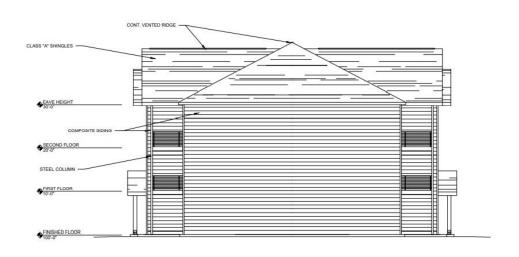
Auxiliary turn lane warrant analysis was conducted in accordance with KYTC Auxiliary Turn Lane policy, as applied by the Warrant Calcs Interactive excel spreadsheet provided on the KYTC Division of Design website. Based on this analysis, a left turn lane is warranted at Access Point 2, the eastern most access point, based on PM peak hour volumes. Right turn lanes were not warranted at either access point. Output from the warrant analysis is provided in **Appendix F**.

### **RECOMMENDATIONS**

No additional improvements beyond the proposed access improvements and the left turn at Access Point 2 have been identified at this time.



### 01 FRONT & REAR ELEVATIONS SCALE: 1/8" = 1'-0"





# HOUSING EXAMPLES





