

final report

July 13, 2020

Traffic Impact Study

1007 South English Station Road
Louisville, KY

RECEIVED

AUG 24 2020

**PLANNING & DESIGN
SERVICES**

Prepared for

Louisville Metro Planning Commission



DIANE B. ZIMMERMAN
Traffic Engineering, LLC

12803 High Meadows Pike
Prospect, KY 40059
502.648.1858
dianebzim@att.net



Table of Contents

INTRODUCTION	2
Figure 1. Site Map	2
EXISTING CONDITIONS	2
Figure 2. Existing Peak Hour Volumes	3
FUTURE CONDITIONS	3
Figure 3. 2023 No Build Peak Hour Volumes	4
TRIP GENERATION	4
Table 1. Peak Hour Trips Generated by Site	4
Figure 4. Trip Distribution Percentages	5
Figure 5. Peak Hour Trips Generated by Site	6
Figure 6. Build Peak Hour Volumes	7
ANALYSIS	7
Table 2. Peak Hour Level of Service	8
CONCLUSIONS	8
APPENDIX	9

INTRODUCTION

The development plan for an apartment community and patio homes on South English Station Road in Louisville, KY shows 168 apartment units and 58 patio homes. **Figure 1** displays a map of the site. Access to the community will be from two entrances on South English Station Road. The purpose of this study is to examine the traffic impacts of the development upon the adjacent highway system. For this study, the impact area was defined to be the entrances on South English Station Road.



Figure 1. Site Map

EXISTING CONDITIONS

South English Station Road is maintained by Louisville Metro with an estimated 2020 ADT of 3,100 vehicles per day south of Lake Village Drive as estimated from the turning movement count. The count was divided by 0.12, the estimated K factor for South English Station Road. The road is a two-lane road with nine-foot lanes with a one-foot shoulder. The speed limit is 35 mph. There are no sidewalks.

Peak hour traffic counts for the intersection of South English Station Road at Lake Village Drive was obtained on Tuesday, March 3, 2020. The a.m. peak hour was 7:15 to 8:15 and the p.m. peak hour was 4:45 to 5:45. **Figure 2** illustrates the existing a.m. and p.m. peak hour traffic volumes. The Appendix contains the full count data for the intersection.

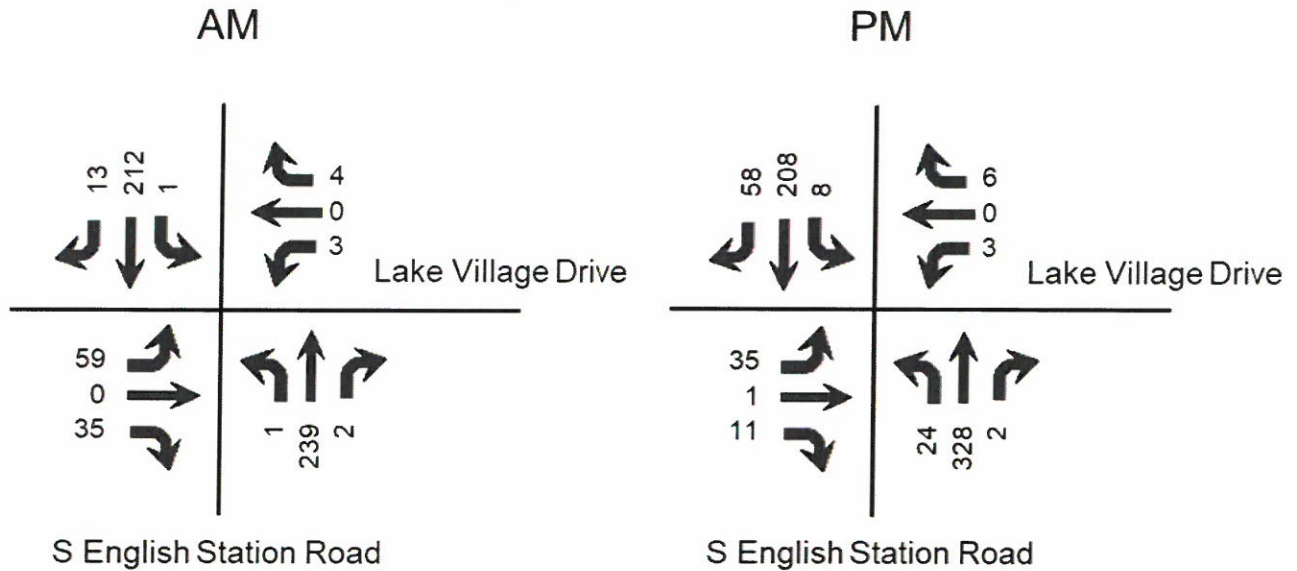


Figure 2. Existing Peak Hour Volumes

FUTURE CONDITIONS

The project completion date is 2023. An annual growth rate of 2.0 percent was applied to the thru 2020 volumes. Additionally, trip generation for 72 additional single-family homes in Signature Point and 105 single family homes just south of I 64 was included on South English Station Road. **Figure 3** displays the 2023 No Build peak hour volumes.

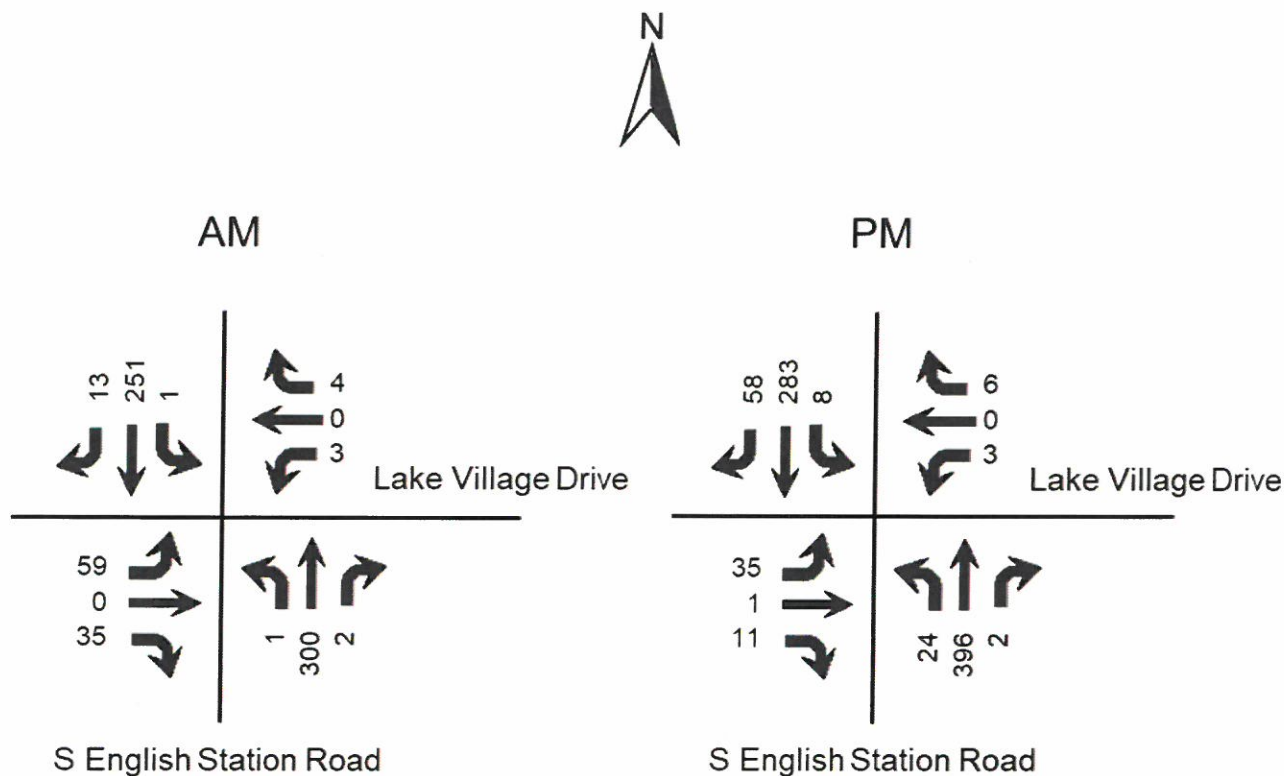


Figure 3. 2023 No Build Peak Hour Volumes

TRIP GENERATION

The Institute of Transportation Engineers Trip Generation Manual, 10th Edition contains trip generation rates for a wide range of developments. The land uses of “Multifamily Housing Mid-Rise (221)” and “Multifamily Housing Low-Rise (220)” were reviewed and determined to be the best match. The trip generation results are listed in **Table 1**. The trips were assigned to the highway network with the percentages shown in **Figure 4**. **Figure 5** shows the trips generated by this development and distributed throughout the road network during the peak hours. **Figure 6** displays the individual turning movements for the peak hours when the development is completed.

Table 1. Peak Hour Trips Generated by Site

Land Use	A.M. Peak Hour			P.M. Peak Hour		
	Trips	In	Out	Trips	In	Out
Multifamily Housing Mid-Rise (168 units)	57	15	42	73	45	28
Multifamily Housing Low-Rise (58 units)	28	6	22	36	23	13
TOTAL	85	21	64	109	68	41



Figure 4. Trip Distribution Percentages

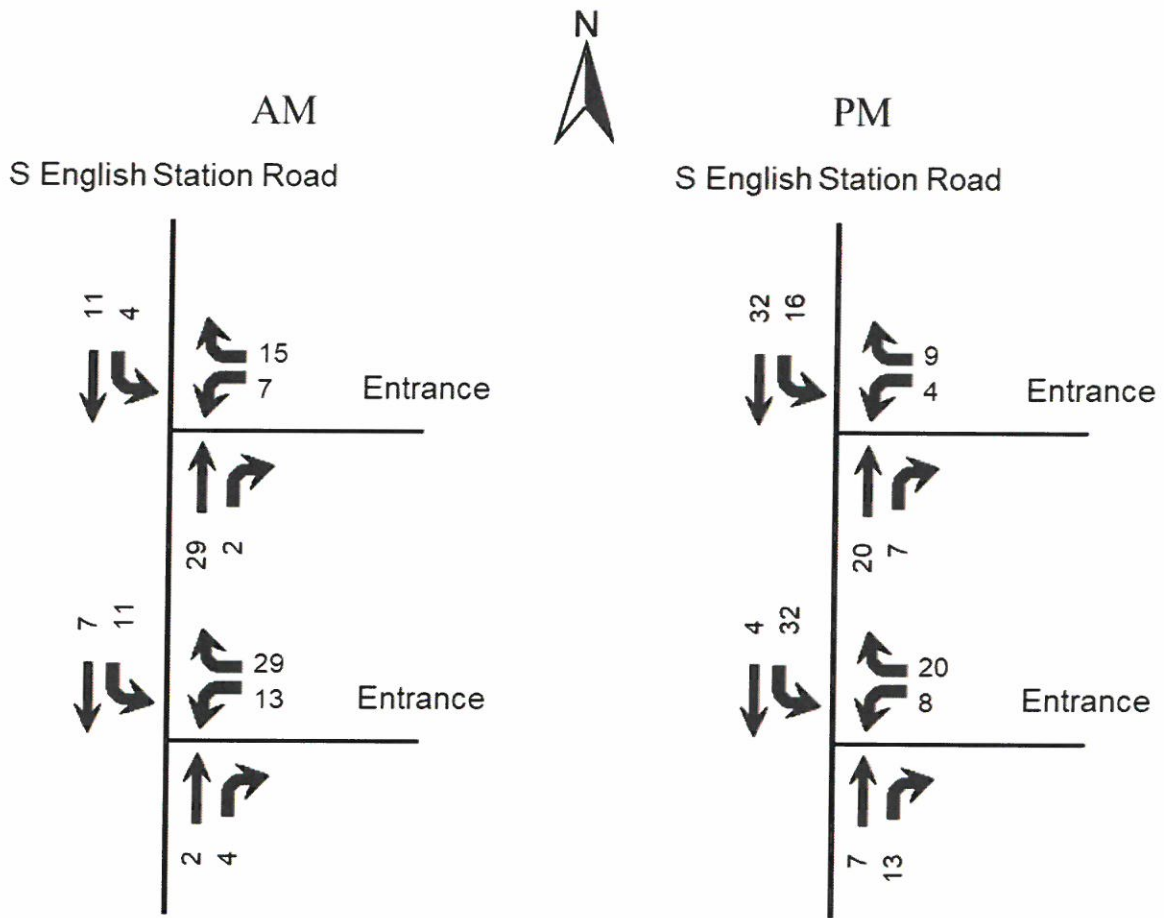


Figure 5. Peak Hour Trips Generated by Site

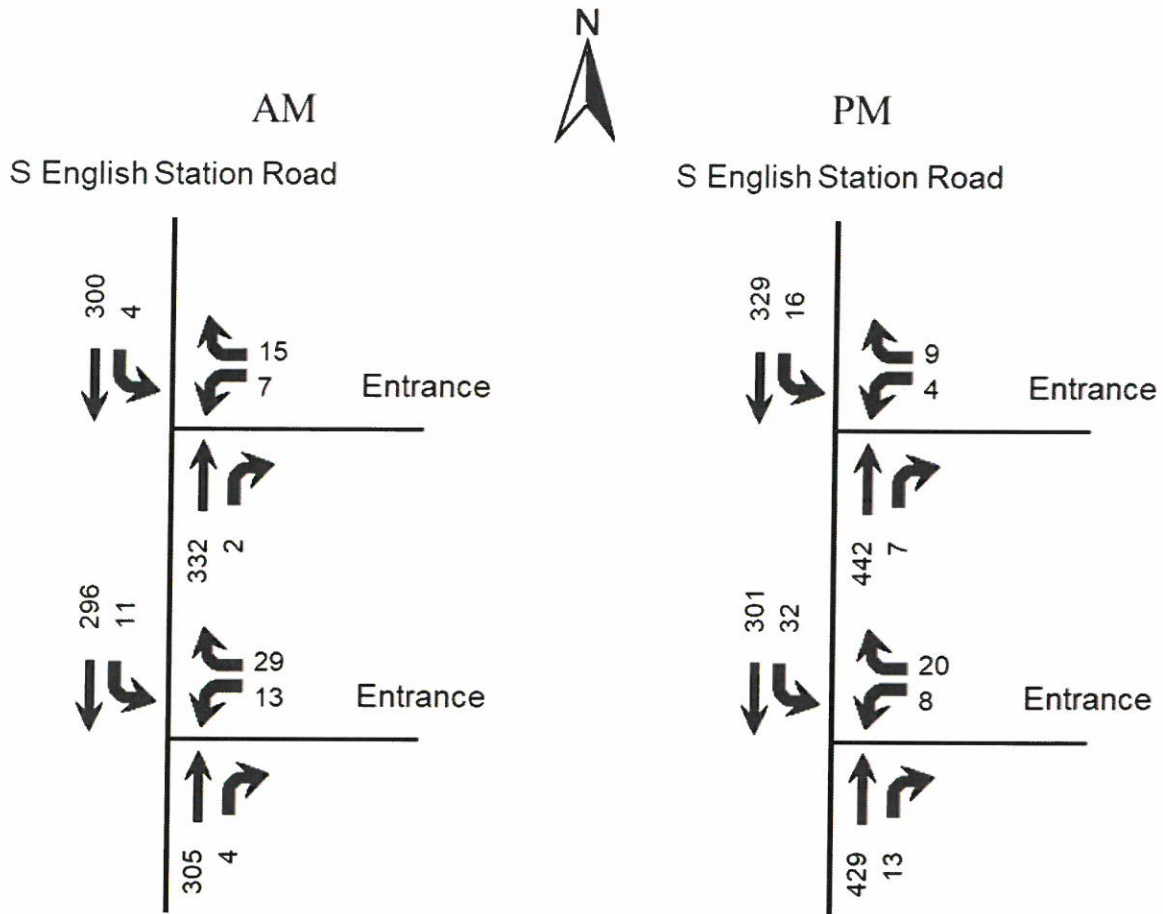


Figure 6. Build Peak Hour Volumes

ANALYSIS

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a "Level of Service". Level of Service is a ranking scale from A through F, "A" is the best operating condition and "F" is the worst. Level of Service results depend upon the facility that is analyzed. In this case, the Level of Service is based upon the total delay experienced at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersections were determined using procedures detailed in the Highway Capacity Manual, 6th edition. Future delays and Level of Service were determined for the intersections using the HCS Streets (version 7.8.5) software. The delays and Level of Service are summarized in **Table 2**.

Table 2. Peak Hour Level of Service

Approach	A.M.			P.M.		
	2020 Existing	2023 No Build	2023 Build	2020 Existing	2023 No Build	2023 Build
S English Station Road at Patio Homes						
Entrance Westbound			B 11.7			B 13.8
S English Station Road Southbound (left)			A 8.0			A 8.5
S English Station Road at Apartment Entrance						
Entrance Eastbound			B 11.7			B 14.0
S English Station Road Southbound (left)			A 8.0			A 8.6

Key: Level of Service, Delay in seconds per vehicle

The entrances were evaluated for turn lanes using the Kentucky Transportation Cabinet Highway Design Guidance Manual dated March, 2017. Using the volumes in Figure 6, no turn lanes are required at the entrances.

CONCLUSIONS

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2023, there will be a minimal impact to the existing highway network. No improvements are required.

APPENDIX

1007 South English Station Road
Traffic Impact Study

Traffic Counts

Jefferson County, KY
Classified Turn Movement Count



Marr Traffic
Transportation Data Collection

41 Peabody Street, Nashville, TN 37210
10 Glenlake Parkway, Suite 130, Atlanta, GA 30328
555 Fayetteville Street, Suite 201, Raleigh, NC 27601
1229 South Shelby Street, Louisville, KY 40203
6565 North MacArthur Boulevard, Suite 225, Dallas, TX 75039

hello@marrtraffic.com
www.marrtraffic.com

1 (800) 615-3765

Site 1 of 1
S English Station Rd (North)
Lake Village Dr
S English Station Rd (South)
English Park Cir

Lat/Long Weather
38.224740°, -85.494760° Fair
55°F

Date
Tuesday, March 3, 2020

	Southbound S English Station Rd (North)						Westbound Lake Village Dr						Northbound S English Station Rd (South)						Eastbound English Park Cr						Int	
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App		
	0700 - 0715	0	1	22	0	0	23	0	0	0	2	0	2	0	0	0	22	1	0	23	1	14	0	3		0
0715 - 0730	0	0	43	1	0	44	0	1	0	3	0	4	0	0	63	1	0	64	0	22	0	7	0	29	141	
0730 - 0745	0	0	53	5	0	58	0	0	0	0	0	0	0	0	71	0	0	71	0	17	0	8	0	25	154	
0745 - 0800	0	0	54	3	0	57	0	1	0	1	0	2	0	0	55	1	0	56	0	12	0	12	0	24	139	
0800 - 0815	0	1	62	4	0	67	0	1	0	0	0	1	0	1	50	0	0	51	0	8	0	8	0	16	135	
0815 - 0830	0	0	37	1	0	38	0	0	0	3	0	3	0	1	42	0	0	43	0	15	1	7	3	26	110	
0830 - 0845	0	0	24	4	0	28	0	1	0	2	0	3	0	0	42	0	0	42	0	14	0	2	0	16	89	
0845 - 0900	0	0	24	3	0	27	0	0	0	0	0	0	0	1	28	2	0	31	0	18	0	0	0	18	76	
1600 - 1615	0	2	29	14	0	45	0	0	0	2	0	2	0	2	32	1	0	35	0	1	0	0	0	1	83	
1615 - 1630	0	4	39	13	3	59	0	0	0	1	0	1	0	3	46	0	0	49	0	6	0	0	0	6	115	
1630 - 1645	0	2	43	12	0	57	0	0	0	3	0	3	0	3	53	0	0	56	0	6	0	2	0	8	124	
1645 - 1700	0	1	43	15	0	59	0	0	0	1	0	1	0	5	55	0	0	60	0	12	0	1	0	13	133	
1700 - 1715	0	0	46	12	0	58	0	1	0	1	0	2	0	4	86	1	0	91	0	5	1	0	0	6	157	
1715 - 1730	0	3	60	13	0	76	0	0	0	1	0	1	0	10	104	0	0	114	0	11	0	2	0	13	204	
1730 - 1745	0	4	59	18	0	81	0	2	0	3	0	5	0	5	83	1	0	89	0	7	0	8	0	15	190	
1745 - 1800	0	2	24	15	0	41	0	0	0	2	0	2	0	4	50	1	0	55	0	6	0	2	0	8	106	

0715 - 0730	0	0	43	1	0	44	0	1	0	3	0	4	0	0	63	1	0	64	0	22	0	7	0	29	141
0730 - 0745	0	0	53	5	0	58	0	0	0	0	0	0	0	0	71	0	0	71	0	17	0	8	0	25	154
0745 - 0800	0	0	54	3	0	57	0	1	0	1	0	2	0	0	55	1	0	56	0	12	0	12	0	24	139
0800 - 0815	0	1	62	4	0	67	0	1	0	0	0	1	0	1	50	0	0	51	0	8	0	8	0	16	135
AM PEAK	0	1	212	13	0	226	0	3	0	4	0	7	0	1	239	2	0	242	0	59	0	35	0	94	569
1645 - 1700	0	1	43	15	0	59	0	0	0	1	0	1	0	5	55	0	0	60	0	12	0	1	0	13	133
1700 - 1715	0	0	46	12	0	58	0	1	0	1	0	2	0	4	86	1	0	91	0	5	1	0	0	6	157
1715 - 1730	0	3	60	13	0	76	0	0	0	1	0	1	0	10	104	0	0	114	0	11	0	2	0	13	204
1730 - 1745	0	4	59	18	0	81	0	2	0	3	0	5	0	5	83	1	0	89	0	7	0	8	0	15	190
PM PEAK	0	8	208	58	0	274	0	3	0	6	0	9	0	24	328	2	0	354	0	35	1	11	0	47	684

HCS Reports

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	S English Station at Pati							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	7/13/2020							East/West Street	Entrance							
Analysis Year	2023							North/South Street	S English Station							
Time Analyzed	AM Peak							Peak Hour Factor	0.92							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Sunshine															
Lanes																
<p>Major Street: North-South</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						7		15			332	2		4	300	
Percent Heavy Vehicles (%)						1		1						1		
Proportion Time Blocked																
Percent Grade (%)						0										
Right Turn Channelized																
Median Type Storage						Undivided										
Critical and Follow-up Headways																
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.41		6.21						4.11		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.51		3.31						2.21		
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)						24								4		
Capacity, c (veh/h)						563								1201		
v/c Ratio						0.04								0.00		
95% Queue Length, Q ₉₅ (veh)						0.1								0.0		
Control Delay (s/veh)						11.7								8.0		
Level of Service (LOS)						B								A		
Approach Delay (s/veh)						11.7								0.1		
Approach LOS						B										

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	S English Station at Pati							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	7/13/2020							East/West Street	Entrance							
Analysis Year	2023							North/South Street	S English Station							
Time Analyzed	PM Peak							Peak Hour Factor	0.84							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Sunshine															
Lanes																
<p style="text-align: center;">Major Street: North-South</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						4		9			442	7		16	329	
Percent Heavy Vehicles (%)						1		1						1		
Proportion Time Blocked																
Percent Grade (%)						0										
Right Turn Channelized																
Median Type Storage					Undivided											
Critical and Follow-up Headways																
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.41		6.21						4.11		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.51		3.31						2.21		
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)						15								19		
Capacity, c (veh/h)						424								1038		
v/c Ratio						0.04								0.02		
95% Queue Length, Q ₉₅ (veh)						0.1								0.1		
Control Delay (s/veh)						13.8								8.5		
Level of Service (LOS)						B								A		
Approach Delay (s/veh)						13.8								0.6		
Approach LOS						B										

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	S English Station at Apt							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	7/13/2020							East/West Street	Entrance							
Analysis Year	2023							North/South Street	S English Station							
Time Analyzed	AM Peak							Peak Hour Factor	0.92							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Sunshine															
Lanes																
<p style="text-align: center;">Major Street: North-South</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						13		29			305	4		11	296	
Percent Heavy Vehicles (%)						1		1						1		
Proportion Time Blocked																
Percent Grade (%)							0									
Right Turn Channelized																
Median Type Storage						Undivided										
Critical and Follow-up Headways																
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.41		6.21						4.11		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.51		3.31						2.21		
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)						46								12		
Capacity, c (veh/h)						581								1229		
v/c Ratio						0.08								0.01		
95% Queue Length, Q ₉₅ (veh)						0.3								0.0		
Control Delay (s/veh)						11.7								8.0		
Level of Service (LOS)						B								A		
Approach Delay (s/veh)						11.7								0.4		
Approach LOS						B										

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	S English Station at Apt							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	7/13/2020							East/West Street	Entrance							
Analysis Year	2023							North/South Street	S English Station							
Time Analyzed	PM Peak							Peak Hour Factor	0.84							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Sunshine															
Lanes																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						8		20			429	13		32	301	
Percent Heavy Vehicles (%)						1		1						1		
Proportion Time Blocked																
Percent Grade (%)						0										
Right Turn Channelized																
Median Type Storage						Undivided										
Critical and Follow-up Headways																
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.41		6.21						4.11		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.51		3.31						2.21		
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)						33								38		
Capacity, c (veh/h)						432								1046		
v/c Ratio						0.08								0.04		
95% Queue Length, Q ₉₅ (veh)						0.2								0.1		
Control Delay (s/veh)						14.0								8.6		
Level of Service (LOS)						B								A		
Approach Delay (s/veh)						14.0								1.2		
Approach LOS						B										

