

final report

June 22, 2021

Traffic Impact Study

Preston Highway at Interchange Drive
Louisville, KY

Prepared for

Louisville Metro Planning Commission
Kentucky Transportation Cabinet



DIANE B. ZIMMERMAN
Traffic Engineering LLC

12803 High Meadows Pike
Prospect, KY 40059
502 648 1858
dianebzim@att.net



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INTRODUCTION

The development plan for three lots on the northeast corner of Preston Highway and Interchange Drive in Louisville, KY shows a gas station with convenience store and two fast-food restaurants. **Figure 1** displays a map of the site. Access to the development will be from Interchange Drive, and a right-in/right-out on Preston Highway. The purpose of this study is to examine the traffic impacts of the development upon the adjacent highway system. For this study the impact area was defined to be the intersections of Preston Highway with Interchange Drive, and Cooper Chapel Road, and Mt. Washington Road.

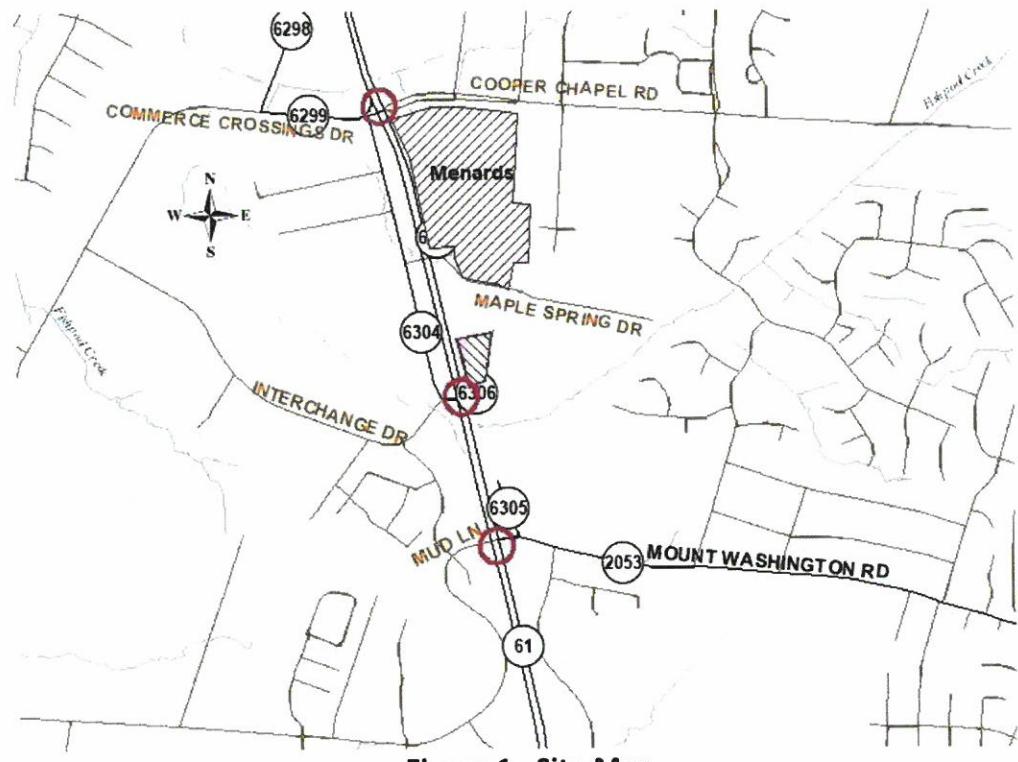


Figure 1. Site Map

EXISTING CONDITIONS

Preston Highway (KY 61) is maintained by the Kentucky Transportation Cabinet (KYTC) with an estimated 2021 ADT of 34,600 vehicles per day south of Cooper Chapel Road, as estimated from the turning movement count using a K factor of 9.90. The road is a four-lane road with twelve-foot lanes with ten-foot paved shoulders. Northbound widens to three lanes north of Maple Springs Drive to the interchange with I 265. The southbound third lane drops 300 feet south of the intersection at Commerce Crossings Drive. The posted speed limit is 50 mph. There are no sidewalks. The intersections with Commerce Crossing Drive, Interchange Drive, and Mt. Washington Road are controlled with a traffic signal. At the intersection with Cooper Chapel Road, there are dual left turn lanes on each approach, except northbound. There is a right turn lane eastbound and southbound; and dual right turn lanes on westbound Cooper Chapel Road. At the intersection with Interchange Drive there are left turn lanes on all approaches and right turn lanes on the southbound and eastbound approaches. At the intersection with Mt. Washington Road there are left and right turn lanes on all approaches, except westbound Mt. Washington Road. Mt. Washington Road has a shared left/thru lane. Preston Highway is served by TARC.

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Peak hour traffic counts for the intersections were obtained on Tuesday, April 13, 2021 (see Appendix A). The a.m. peak hour occurred between 7:15 and 8:15 and the p.m. peak hour varied between the intersections. **Figure 2** illustrates the existing a.m. and p.m. peak hour traffic volumes.

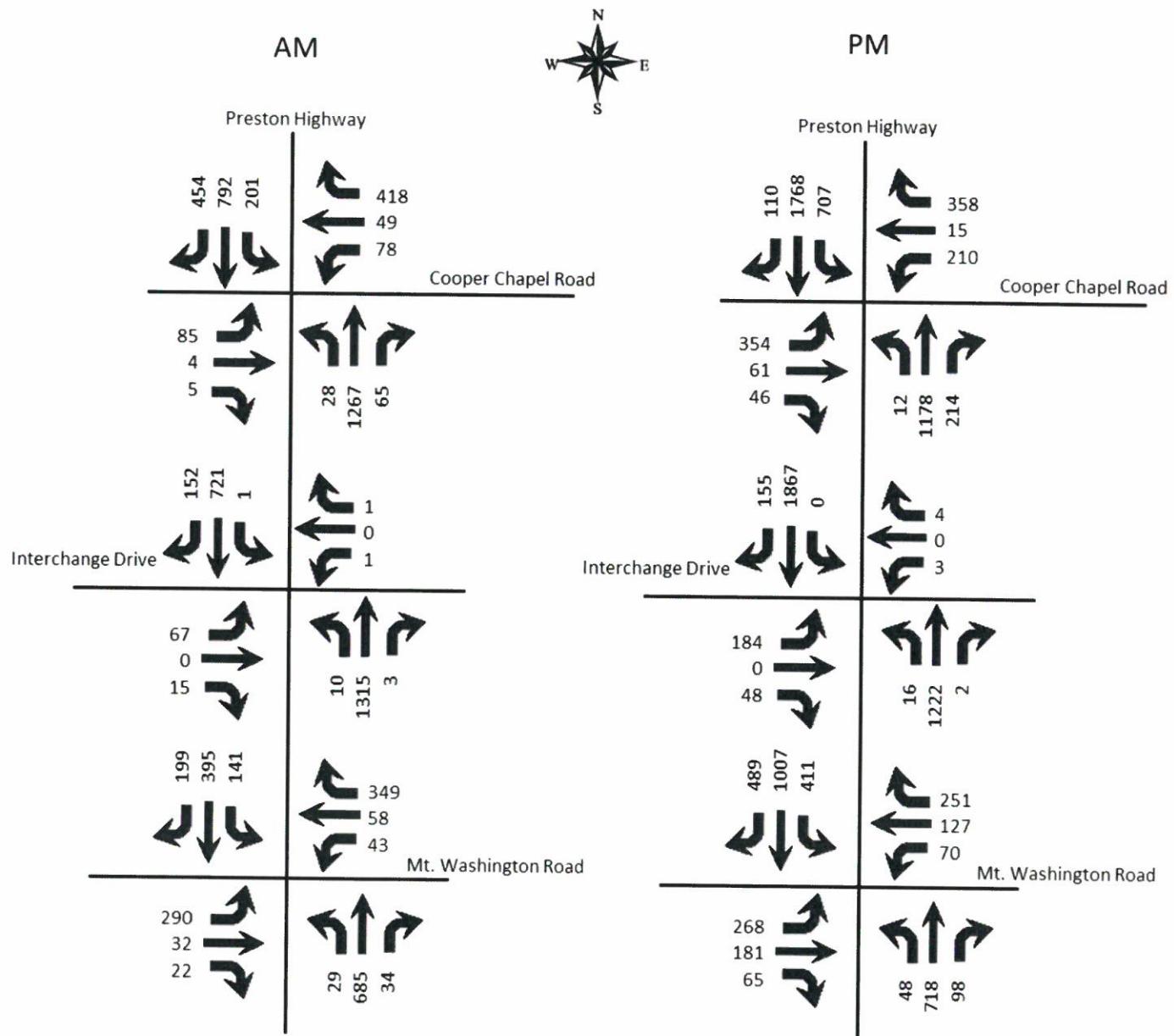


Figure 2. Existing Peak Hour Volumes

FUTURE CONDITIONS

The requested analysis year for this project is 2022. To predict traffic volumes in 2022, one half percent annual growth in traffic was added to the 2021 volumes. This growth rate is the same as was used in the Menards Traffic Impact Study dated April 9, 2019. **Figure 3** displays the 2022 No Build volumes.

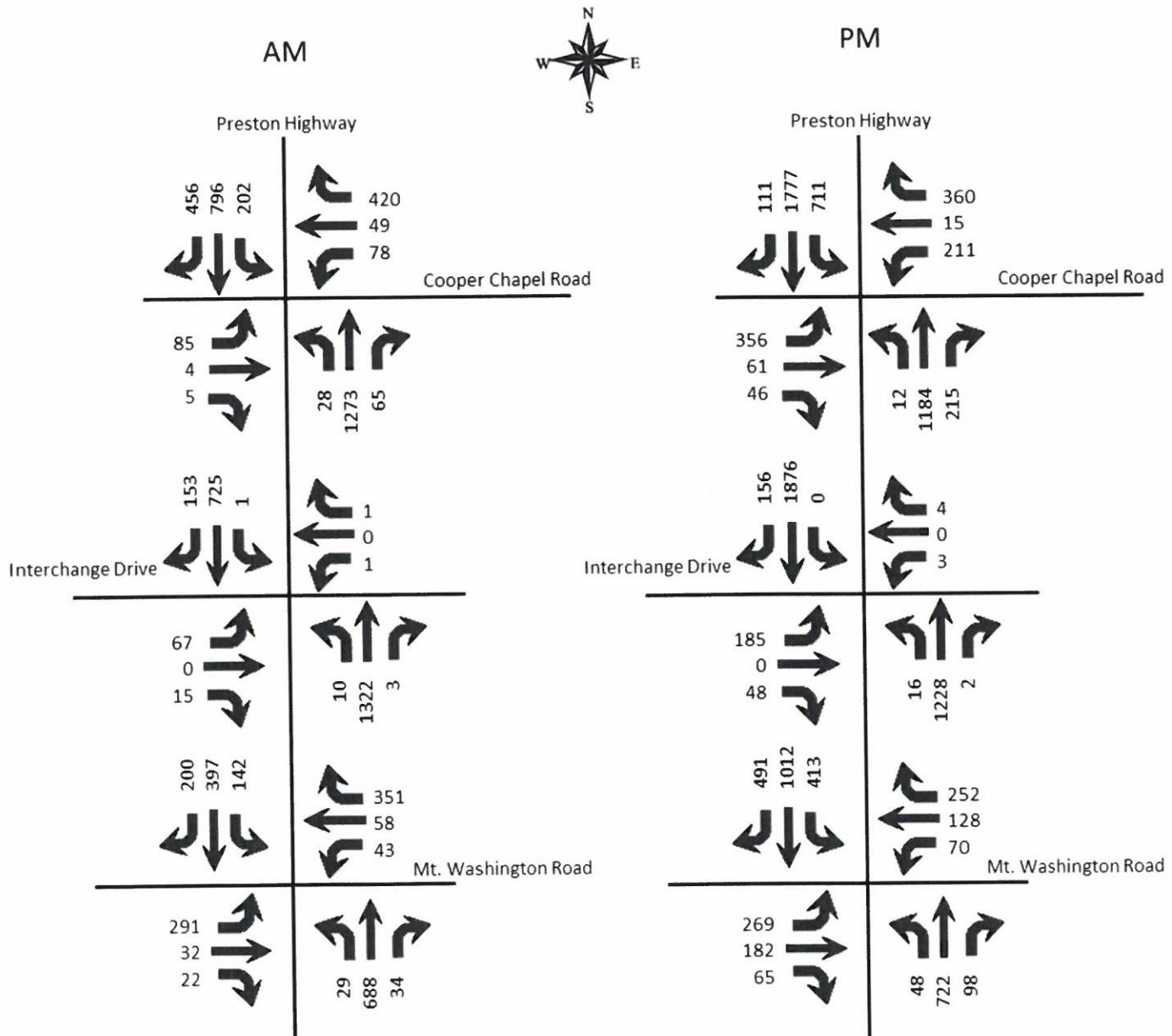


Figure 3. 2022 No Build Peak Hour Volumes

TRIP GENERATION

The Institute of Transportation Engineers Trip Generation Manual, 10th Edition contains trip generation rates for a wide range of developments. The land uses were reviewed and determined to be the best match. The trip generation

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results are listed in **Table 1**. The primary trips were assigned to the highway network with the percentages shown in **Figure 4**. The pass-by trips are assigned using the existing traffic passing the site. These trips are shown in parenthesis. **Figure 5** shows the trips generated by this development and distributed throughout the road network for the year 2022 during the peak hours. **Figure 6** displays the individual turning movements for the year 2022 for the peak hours when the development is completed.

Table 1. Peak Hour Trips Generated by Site

Land Use	A.M. Peak Hour				P.M. Peak Hour			
	Trips	In	Out	Pass-by	Trips	In	Out	Pass-by
Convenience Market with Gas (16 fueling spots)	449	225	224	278	367	184	183	205
Fast-Food with Drive-Through (6,740 sq ft)	271	138	133	133	220	114	106	110
Total	720	363	357	411	587	298	289	315

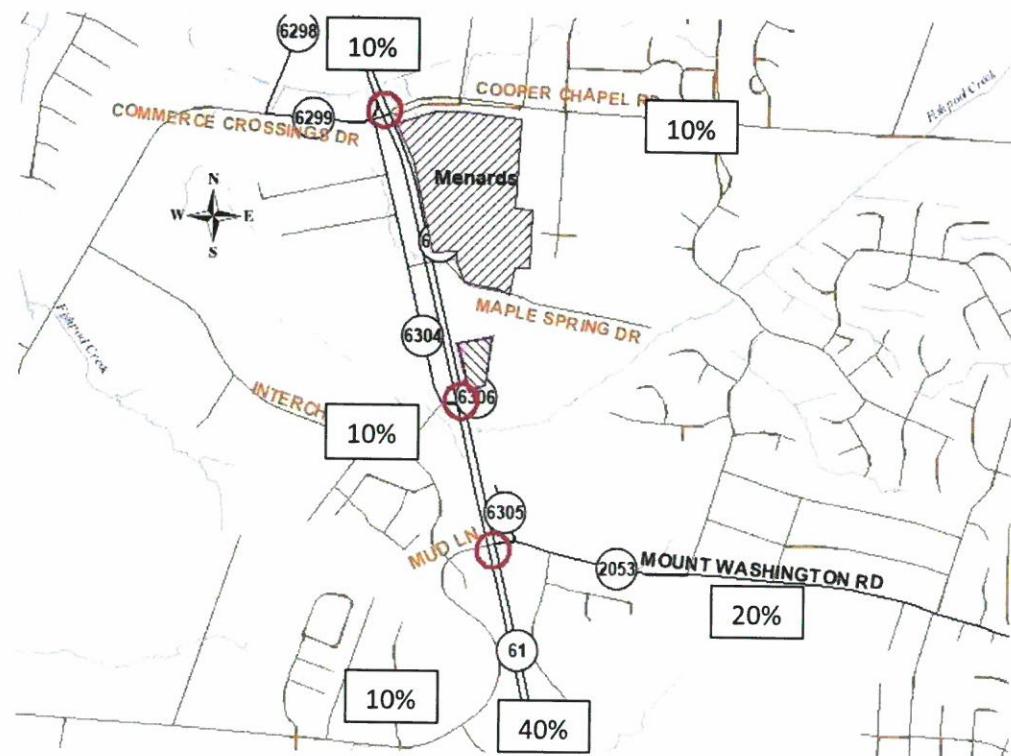


Figure 4. Trip Distribution Percentages

Preston Highway at Interchange Drive
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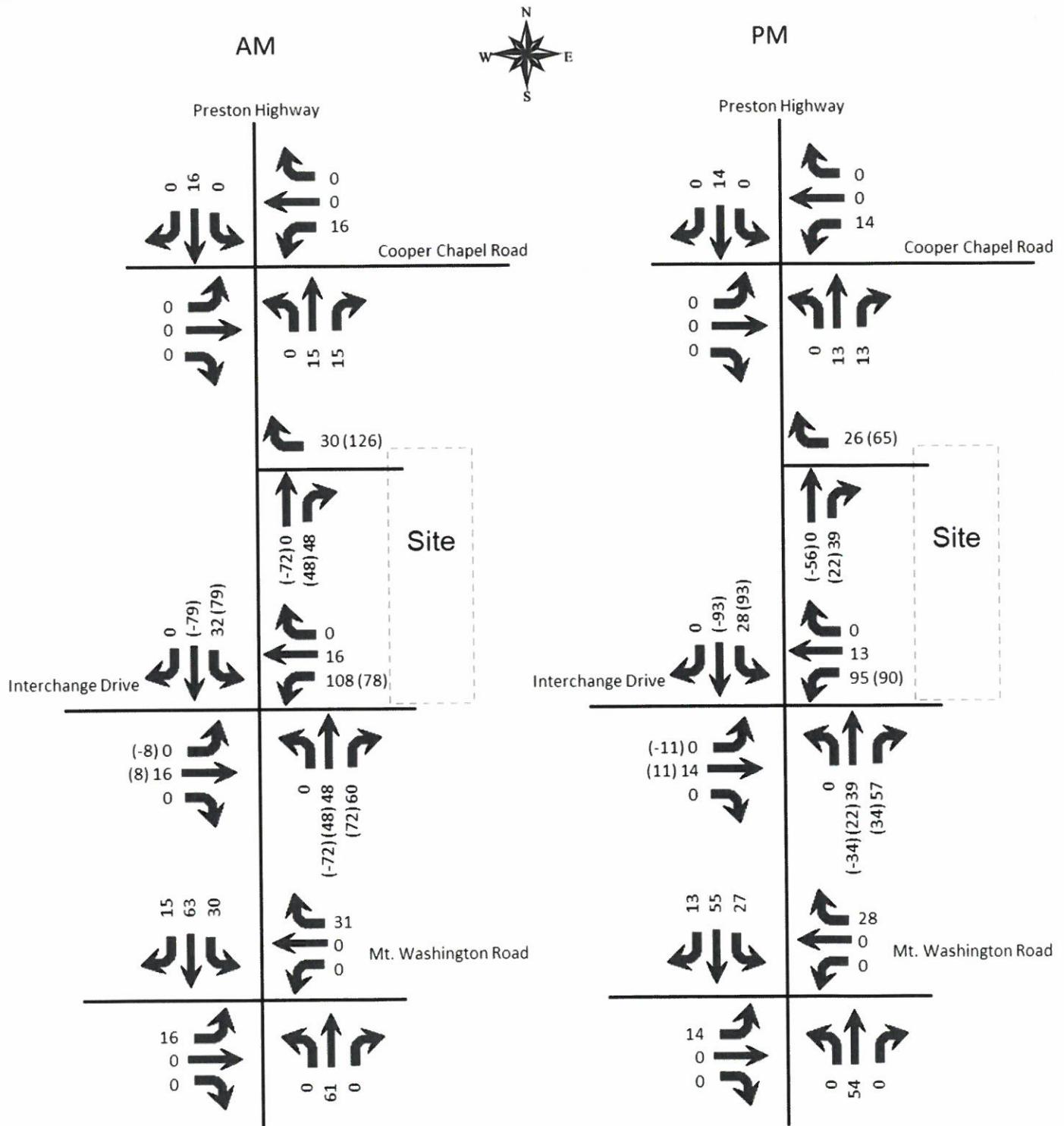


Figure 5. Peak Hour Trips Generated by Site

Preston Highway at Interchange Drive
Traffic Impact Study

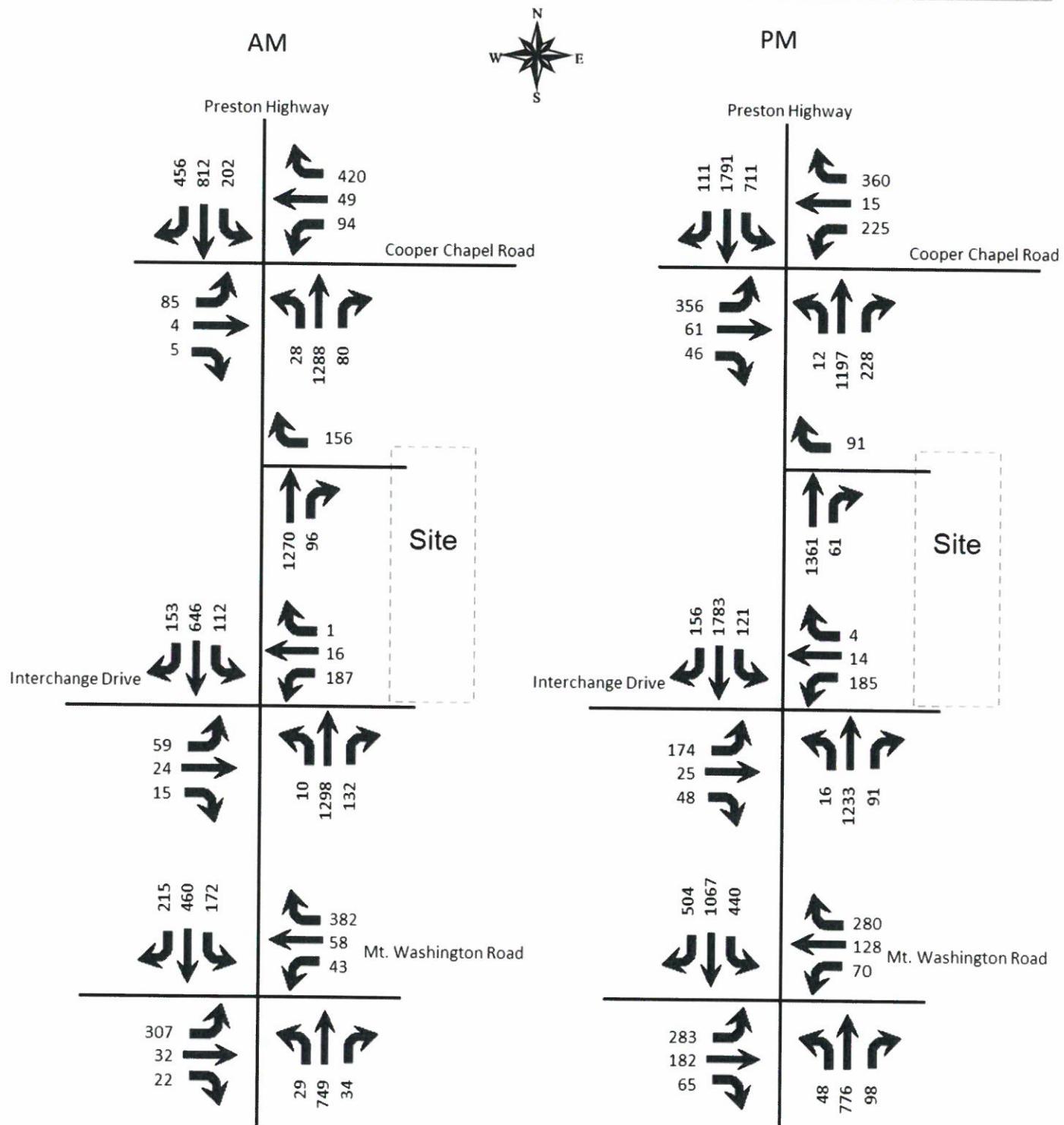


Figure 6. 2022 Peak Hour Build

ANALYSIS

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a "Level of Service". Level of Service is a ranking scale from A through F, "A" is the best operating condition and "F" is the worst. Level of Service results depend upon the facility that is analyzed. In this case, the Level of Service is based upon the total delay experienced at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersections were determined using procedures detailed in the Highway Capacity Manual, 6th edition. Future delays and Level of Service were determined for the intersections using the HCS Streets (version 7.9.5) software. The delays and Level of Service are summarized in **Table 2**.

Table 2. Peak Hour Level of Service

Approach	A.M.			P.M.		
	2021 Existing	2022 No Build	2022 Build	2021 Existing	2022 No Build	2022 Build
Preston Highway at Cooper Chapel Road	C 29.6	C 29.1	C 29.1	D 44.5	D 44.7	D 37.8
Commerce Crossings Eastbound	E 59.2	E 59.8	E 58.9	E 77.6	E 77.5	E 77.7
Cooper Chapel Road Westbound	D 43.1	D 42.1	D 42.1	E 60.3	E 60.2	E 60.6
Preston Highway Northbound	C 25.6	C 25.2	C 25.0	D 52.7	D 53.4	C 27.7
Preston Highway Southbound	C 25.5	C 25.1	C 25.0	C 30.7	C 30.8	C 30.8
Preston Highway at Interchange Drive	A 7.4	A 7.4	C 26.4	B 19.4	B 19.3	D 37.0
Interchange Drive Eastbound	E 72.6	E 72.6	E 69.4	E 75.0	E 75.0	E 78.1
Entrance Westbound	F 87.9	F 87.9	E 72.8	F 84.8	F 84.8	F 87.3
Preston Highway Northbound	A 4.1	A 4.1	B 19.2	A 8.8	A 8.7	C 28.6
Preston Highway Southbound	A 5.9	A 5.9	C 22.2	B 19.2	B 19.3	C 32.4
Preston Highway at Mt. Washington Road	D 43.9	D 44.8	D 44.8	D 45.3	D 45.4	D 49.7
Mud Lane Eastbound	E 59.3	E 59.4	E 60.3	E 79.7	E 79.9	F 81.9
Mt. Washington Road Westbound	D 49.4	D 49.9	D 50.0	E 61.8	E 61.8	E 58.5
Preston Highway Northbound	D 42.8	D 43.4	D 47.3	D 51.6	D 51.9	D 53.8
Preston Highway Southbound	C 34.7	C 36.3	C 33.0	C 28.7	C 28.7	C 36.9

Key: Level of Service, Delay in seconds per vehicle

Preston Highway at Interchange Drive
Traffic Impact Study

The entrance that aligns with Interchange Drive was analyzed as a split phase signal, to allow the continued operation of the dual left turn lanes on Interchange Drive. The exit should have a dedicated left turn lane and a shared thru and right turn lane. The entrances were evaluated for turn lanes using the Kentucky Transportation Cabinet Highway Design Guidance Manual dated July, 2020. The traffic impact policy requires using volumes for ten years beyond build-out, or 2032. The 2032 volumes were determined applying a one-half percent annual growth rate from 2021. Figure 7 illustrates the 2032 No Build volumes. Figure 8 illustrates the 2032 Build Volumes. Using the volumes in Figure 8, a right turn lane will be required at the right-in/right-out entrance on Preston Highway. The right turn lane will be designed to KYTC standards. Table 3 summarizes the delay and Level of Service for 2032.

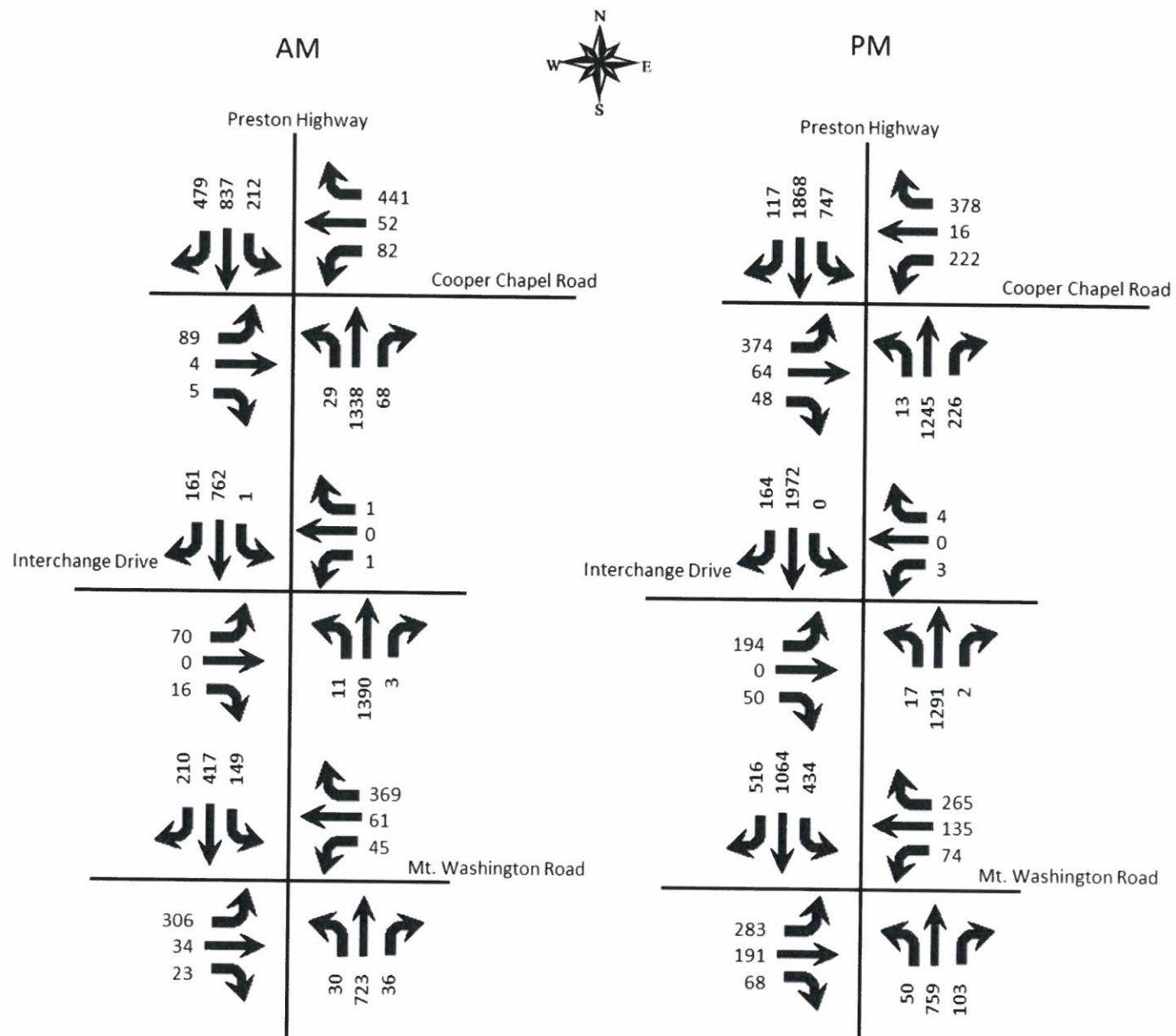


Figure 7. 2032 Peak Hour No Build

Preston Highway at Interchange Drive
Traffic Impact Study

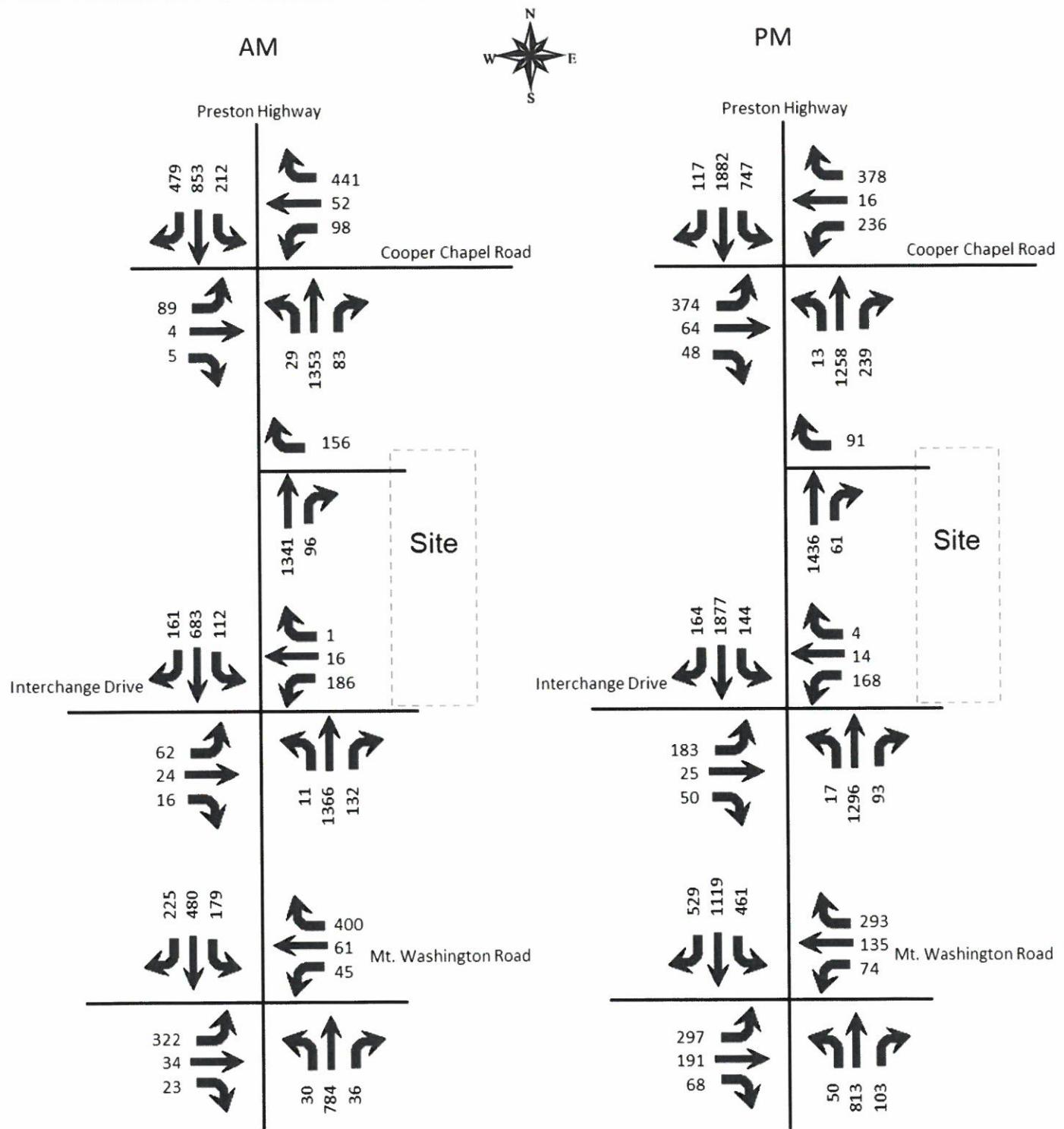


Figure 8. 2032 Peak Hour Build

Table 3. Peak Hour Level of Service

Approach	A.M.			P.M.		
	2021 Existing	2032 No Build	2032 Build	2021 Existing	2032 No Build	2032 Build
Preston Highway at Cooper Chapel Road	C 29.6	C 30.7	C 30.7	D 44.5	D 46.6	D 39.3
Commerce Crossings Eastbound	E 59.2	E 64.1	E 63.3	E 77.6	E 76.8	E 77.0
Cooper Chapel Road Westbound	D 43.1	D 44.8	D 44.8	E 60.3	E 59.2	E 59.6
Preston Highway Northbound	C 25.6	C 26.7	C 26.4	D 52.7	D 57.9	C 30.6
Preston Highway Southbound	C 25.5	C 26.1	C 26.0	C 30.7	C 32.5	C 32.4
Preston Highway at Interchange Drive	A 7.4	A 7.5	C 27.4	B 19.3	B 18.9	D 39.6
Interchange Drive Eastbound	E 72.6	E 73.0	E 69.8	E 75.0	E 75.2	E 78.2
Entrance Westbound	F 87.9	F 87.9	E 72.9	F 84.8	F 84.8	E 78.7
Preston Highway Northbound	A 4.1	A 4.2	C 21.5	A 8.8	A 7.9	C 32.8
Preston Highway Southbound	A 5.9	A 5.9	C 22.0	B 19.2	B 19.1	D 35.8
Preston Highway at Mt. Washington Road	D 43.9	D 44.9	D 45.6	D 45.3	D 48.8	D 52.9
Mud Lane Eastbound	E 59.3	E 60.3	E 61.3	E 79.7	F 83.5	F 85.9
Mt. Washington Road Westbound	D 49.4	D 50.5	D 51.0	E 61.8	E 62.1	E 59.0
Preston Highway Northbound	D 42.8	D 44.4	D 49.2	D 51.6	D 53.3	D 54.7
Preston Highway Southbound	C 34.7	C 34.9	C 32.4	C 28.7	C 33.7	D 41.4

Key: Level of Service, Delay in seconds per vehicle

An additional comparison was made for the Interchange Drive intersection without the right-in right-out. The absence of the second entrance requires all the site traffic to use the signal at Interchange Drive. The results are shown in **Table 4**. In both a.m. and p.m. peaks the intersection functions better with the proposed right-in/ right-out.

Table 4. 2032 Comparison with and without Right-in/Right-out

Approach	A.M.		P.M.	
	2032 Build	2032 Build No Right-in/Right-out	2032 Build	2032 Build No Right-in/Right-out
Preston Highway at Interchange Drive	C 27.4	C 30.6	D 39.6	D 40.5
Interchange Drive Eastbound	E 69.8	E 69.8	E 78.2	E 78.2
Entrance Westbound	E 72.9	E 71.3	E 78.7	E 77.9
Preston Highway Northbound	C 21.5	C 22.8	C 32.8	C 33.1
Preston Highway Southbound	C 22.0	C 22.5	D 35.8	D 35.8

Key: Level of Service, Delay in seconds per vehicle

CONCLUSIONS

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2022 and 2032, there will be an impact to the existing highway network. The proposed right-in/right-out provides improved operating conditions when compared to without the additional access. A right turn lane will be required at the right-in/right-out entrance on Preston Highway. The exit opposite Interchange Drive should have two lanes with a dedicated left and a shared thru/right. The southbound left turn lane will need additional storage.

APPENDIX

Preston Highway at Interchange Drive
Traffic Impact Study

Traffic Counts

Classified Turn Movement Count || All vehicles

Preston Highway, KY



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Site 1 of 3
KY-61 Preston Hwy (South)
KY-61 Preston Hwy (North)
Commerce Crossings Dr
Cooper Chapel Rd

Date
Tuesday, April 13, 2021
Lat/Long
38.103518°, -85.672625°

Weather
Cloudy
61°F

0700 - 0900 (Weekday 2h Session) (13-04-2021)

All vehicles

TIME	Northbound					Southbound					Eastbound					Westbound					
	KY-61 Preston Hwy (South)					KY-61 Preston Hwy (North)					Commerce Crossings Dr					Cooper Chapel Rd					
	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Int Total
0700 - 0715	2	306	13	0	321	25	144	88	1	258	20	0	2	0	22	15	9	130	0	154	755
0715 - 0730	8	327	16	0	351	34	189	86	1	310	23	1	2	0	26	16	8	109	0	133	820
0730 - 0745	5	365	19	0	389	56	191	93	0	340	24	1	1	0	26	9	14	117	0	140	895
0745 - 0800	10	298	15	0	323	59	221	173	0	453	13	1	1	0	15	24	16	97	0	137	928
Hourly Total	25	1296	63	0	1384	174	745	440	2	1361	80	3	6	0	89	64	47	453	0	564	3398
0800 - 0815	5	277	15	0	297	52	191	102	0	345	25	1	1	0	27	29	11	95	0	135	804
0815 - 0830	6	259	18	0	283	54	155	67	0	276	15	1	3	0	19	21	3	82	0	106	684
0830 - 0845	4	267	26	0	297	39	187	59	0	285	15	5	2	0	22	28	3	106	0	137	741
0845 - 0900	6	253	25	0	284	44	199	51	0	294	28	3	3	0	34	38	7	84	0	129	741
Hourly Total	21	1056	84	0	1161	189	732	279	0	1200	83	10	9	0	102	116	24	367	0	507	2970
Grand Total	46	2352	147	0	2545	363	1477	719	2	2561	163	13	15	0	191	180	71	820	0	1071	6368
Approach %	1.81	92.42	5.78	0.00	-	14.17	57.67	28.07	0.08	-	85.34	6.81	7.85	0.00	-	16.81	6.63	76.56	0.00	-	
Intersection %	0.72	36.93	2.31	0.00	39.97	5.70	23.19	11.29	0.03	40.22	2.56	0.20	0.24	0.00	3.00	2.83	1.11	12.88	0.00	16.82	
PHF	0.70	0.87	0.86	0.00	0.87	0.85	0.90	0.66	0.25	0.80	0.85	1.00	0.63	0.00	0.87	0.67	0.77	0.89	0.00	0.97	0.93

1600 - 1800 (Weekday 2h Session) (13-04-2021)

All vehicles

TIME	Northbound					Southbound					Eastbound					Westbound					
	KY-61 Preston Hwy (South)					KY-61 Preston Hwy (North)					Commerce Crossings Dr					Cooper Chapel Rd					
	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Int Total
1600 - 1615	1	297	49	0	347	173	436	46	2	657	97	12	8	0	117	45	5	116	0	166	1287
1615 - 1630	3	261	55	0	319	173	411	48	1	633	60	18	10	0	88	60	4	113	0	177	1217
1630 - 1645	4	313	57	0	374	159	380	35	2	576	120	22	17	0	159	45	4	93	1	143	1252
1645 - 1700	1	297	50	1	349	186	445	26	0	657	61	11	9	0	81	54	4	78	0	136	1223
Hourly Total	9	1168	211	1	1389	691	1672	155	5	2523	338	63	44	0	445	204	17	400	1	622	4979
1700 - 1715	2	272	51	0	325	156	454	39	0	649	122	15	14	0	151	56	6	87	0	149	1274
1715 - 1730	4	296	56	0	356	206	489	10	1	706	51	13	6	0	70	54	1	100	0	155	1287
1730 - 1745	5	300	47	0	352	158	435	28	1	622	60	8	7	0	75	62	1	109	0	172	1221
1745 - 1800	2	280	58	0	340	119	401	23	0	543	36	6	4	0	46	52	3	96	0	151	1080
Hourly Total	13	1148	212	0	1373	639	1779	100	2	2520	269	42	31	0	342	224	11	392	0	627	4862
Grand Total	22	2316	423	1	2762	1330	3451	255	7	5043	607	105	75	0	787	428	28	792	1	1249	9841
Approach %	0.80	83.85	15.31	0.04	-	26.37	68.43	5.06	0.14	-	77.13	13.34	9.53	0.00	-	34.27	2.24	63.41	0.08	-	
Intersection %	0.22	23.53	4.30	0.01	28.07	13.51	35.07	2.59	0.07	51.24	6.17	1.07	0.76	0.00	8.00	4.35	0.28	8.05	0.01	12.69	
PHF	0.69	0.94	0.94	0.25	0.94	0.86	0.90	0.71	0.38	0.92	0.73	0.69	0.68	0.00	0.72	0.93	0.63	0.90	0.25	0.94	0.98

Preston Highway at Interchange Drive
Traffic Impact Study

Classified Turn Movement Count || All vehicles

Preston Highway, KY



www.marrtraffic.com

Site 2 of 3

KY-61 Preston Hwy (South)
KY-61 Preston Hwy (North)
Old Preston Hwy
Local Rd

Date
Tuesday, April 13, 2021

Weather
Cloudy
61°F

Lat/Long
38.096348°, -85.670213°

0700 - 0900 (Weekday 2h Session) (13-04-2021)

All vehicles

TIME	Northbound					Southbound					Eastbound					Westbound						
	KY-61 Preston Hwy (South)					KY-61 Preston Hwy (North)					Old Preston Hwy					Local Rd						
	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Int	Total
0700 - 0715	5	314	0	0	319	0	100	52	0	152	2.9	2.10	2.11	2.12	Total	2.13	2.14	2.15	2.16	App	0	0
0715 - 0730	0	331	0	0	331	0	147	65	0	212	22	0	2	0	Total	0	0	0	0	0	0	496
0730 - 0745	4	398	1	0	403	0	175	33	0	208	19	0	3	0	Total	0	0	0	0	0	0	567
0745 - 0800	4	299	1	1	305	0	199	32	0	231	14	0	7	0	Total	1	0	0	0	1	1	634
Hourly Total	13	1342	2	1	1358	0	621	182	0	803	78	0	14	0	Total	0	0	1	0	1	1	558
0800 - 0815	0	287	1	1	289	0	200	22	1	223	12	0	3	0	Total	1	0	1	0	2	2	225
0815 - 0830	2	257	0	0	259	1	167	14	0	182	9	0	0	0	Total	0	0	0	0	0	0	527
0830 - 0845	0	283	0	0	283	1	198	13	0	212	20	0	3	0	Total	9	0	0	0	0	0	450
0845 - 0900	2	279	1	0	282	0	234	16	0	250	11	0	1	1	Total	13	0	0	0	0	0	518
Hourly Total	4	1106	2	1	1113	2	799	65	1	867	52	0	7	1	Total	60	0	0	0	0	0	545
Grand Total	17	2448	4	2	2471	2	1420	247	1	1670	130	0	21	1	Total	152	1	0	1	0	2	4295
Approach %	0.69	99.07	0.16	0.08	-	0.12	85.03	14.79	0.06	-	85.53	0.00	13.82	0.66	-	50.00	0.00	50.00	0.00	-		
Intersection %	0.40	57.00	0.09	0.05	57.53	0.05	33.06	5.75	0.02	38.88	3.03	0.00	0.49	0.02	Total	3.54	0.02	0.00	0.02	0.00	0.05	
PHF	0.50	0.83	0.75	0.50	0.82	0.00	0.90	0.58	0.25	0.95	0.76	0.00	0.54	0.00	Total	0.85	0.25	0.00	0.25	0.00	0.50	0.90

1600 - 1800 (Weekday 2h Session) (13-04-2021)

All vehicles

TIME	Northbound					Southbound					Eastbound					Westbound					Int	Total
	KY-61 Preston Hwy (South)					KY-61 Preston Hwy (North)					Old Preston Hwy					Local Rd						
	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Left	Thru	Right	U-Turn	App	Int	Total
1600 - 1615	0	295	2	0	297	0	391	32	0	423	2.9	2.10	2.11	2.12	Total	82	1	1	1	0	3	805
1615 - 1630	6	289	2	0	297	1	467	29	0	497	36	0	16	0	Total	52	0	0	3	0	3	849
1630 - 1645	1	310	0	0	311	0	424	33	2	459	62	0	17	0	Total	79	0	0	1	0	1	850
1645 - 1700	1	325	1	0	327	0	445	45	0	490	44	0	13	0	Total	57	1	0	1	0	2	876
Hourly Total	8	1219	5	0	1232	1	1727	139	2	1869	209	0	61	0	Total	270	2	1	6	0	9	3380
1700 - 1715	3	286	0	0	289	0	464	35	0	499	43	0	12	0	Total	55	1	0	2	0	3	846
1715 - 1730	3	296	1	2	302	0	481	39	0	520	45	0	13	0	Total	58	1	0	1	0	2	882
1730 - 1745	7	315	0	0	322	0	477	36	0	513	52	0	10	0	Total	62	0	0	0	0	0	897
1745 - 1800	7	297	0	1	305	0	397	41	0	438	35	0	3	0	Total	38	0	0	0	0	0	781
Hourly Total	20	1194	1	3	1218	0	1819	151	0	1970	175	0	38	0	Total	213	2	0	3	0	5	3406
Grand Total	28	2413	6	3	2450	1	3546	290	2	3839	384	0	99	0	Total	483	4	1	9	0	14	6786
Approach %	1.14	98.49	0.24	0.12	-	0.03	92.37	7.55	0.05	-	79.50	0.00	20.50	0.00	-	28.57	7.14	64.29	0.00	-		
Intersection %	0.41	35.56	0.09	0.04	36.10	0.01	52.25	4.27	0.03	56.57	5.66	0.00	1.46	0.00	Total	7.12	0.06	0.01	0.13	0.00	0.21	
PHF	0.50	0.94	0.50	0.25	0.95	0.00	0.97	0.86	0.00	0.97	0.88	0.00	0.92	0.00	Total	0.94	0.75	0.00	0.50	0.00	0.58	0.98

Preston Highway at Interchange Drive
Traffic Impact Study

Classified Turn Movement Count || All vehicles

Preston Highway, KY



www.marrtraffic.com

Site 3 of 3

KY-61 N Preston Hwy
KY-61 Preston Hwy
Mud Ln
Mt. Washington Rd

Date

Tuesday, April 13, 2021

Weather

Cloudy
61°F

Lat/Long

38.092809°, -85.669177°

0700 - 0900 (Weekday 2h Session) (13-04-2021)

All vehicles

TIME	Northbound					Southbound					Eastbound					Westbound					Int Total	
	KY-61 N Preston Hwy					KY-61 Preston Hwy					Mud Ln					Mt. Washington Rd						
	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total		
0700 - 0715	11	148	5	0	164	18	53	29	0	100	68	9	0	0	77	7	18	100	0	125	466	
0715 - 0730	13	165	8	0	186	34	73	46	0	153	79	7	7	0	93	9	14	93	0	116	548	
0730 - 0745	4	203	6	0	213	30	96	40	0	166	95	9	7	0	111	8	17	96	0	121	611	
0745 - 0800	6	176	13	0	195	40	113	60	0	213	56	8	2	0	66	7	8	73	0	88	562	
Hourly Total	34	692	32	0	758	122	335	175	0	632	298	33	16	0	347	31	57	362	0	450	2187	
0800 - 0815	6	141	7	0	154	37	113	53	0	203	60	8	6	0	74	19	19	87	0	125	556	
0815 - 0830	6	133	12	0	151	31	95	41	0	167	46	16	3	0	65	6	17	82	0	105	488	
0830 - 0845	7	147	17	0	171	19	113	52	0	184	69	6	8	0	83	12	20	73	0	105	543	
0845 - 0900	3	147	8	0	158	39	138	53	0	230	61	10	3	0	74	15	20	63	0	98	560	
Hourly Total	22	568	44	0	634	126	459	199	0	784	236	40	20	0	296	52	76	305	0	433	2147	
Grand Total	56	1260	76	0	1392	248	794	374	0	1416	534	73	36	0	643	83	133	667	0	883	4334	
Approach %	4.02	90.52	5.46	0.00	-	17.51	56.07	26.41	0.00	-	83.05	11.35	5.60	0.00	-	9.40	15.06	75.54	0.00	-		
Intersection %	1.29	29.07	1.75	0.00	32.12	5.72	18.32	8.63	0.00	32.67	12.32	1.68	0.83	0.00	14.84	1.92	3.07	15.39	0.00	20.37		
PHF	0.56	0.84	0.65	0.00	0.88	0.88	0.87	0.83	0.00	0.86	0.76	0.89	0.79	0.00	0.77	0.57	0.76	0.91	0.00	0.90	0.93	

1600 - 1800 (Weekday 2h Session) (13-04-2021)

All vehicles

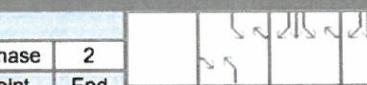
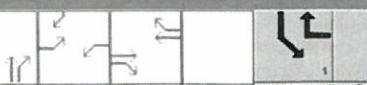
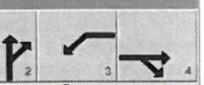
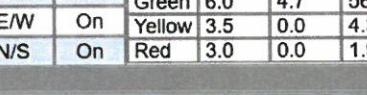
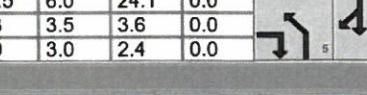
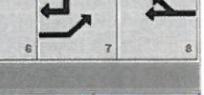
TIME	Northbound					Southbound					Eastbound					Westbound					Int Total	
	KY-61 N Preston Hwy					KY-61 Preston Hwy					Mud Ln					Mt. Washington Rd						
	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total	Left	Thru	Right	U-Turn	App Total		
1600 - 1615	5	153	23	0	181	101	197	87	0	385	65	46	13	0	124	24	30	72	0	126	816	
1615 - 1630	12	161	27	0	200	119	269	123	0	511	71	40	19	0	130	14	28	62	0	104	945	
1630 - 1645	18	196	34	0	248	95	232	123	0	450	74	32	10	0	116	21	32	62	1	116	930	
1645 - 1700	9	205	31	0	245	100	242	116	0	458	50	46	19	0	115	17	33	59	0	109	927	
Hourly Total	44	715	115	0	874	415	940	449	0	1804	260	164	61	0	485	76	123	255	1	455	3618	
1700 - 1715	11	163	22	0	196	97	244	120	0	461	73	47	15	0	135	18	32	63	0	113	905	
1715 - 1730	13	177	25	0	215	108	261	126	0	495	63	51	15	0	129	15	27	61	0	103	942	
1730 - 1745	15	173	20	0	208	106	260	127	0	493	82	37	16	0	135	20	35	68	0	123	959	
1745 - 1800	12	159	28	0	199	100	193	121	0	414	71	51	12	0	134	14	46	69	0	129	876	
Hourly Total	51	672	95	0	818	411	958	494	0	1863	289	186	58	0	533	67	140	261	0	468	3682	
Grand Total	95	1387	210	0	1692	826	1898	943	0	3667	549	350	119	0	1018	143	263	516	1	923	7300	
Approach %	5.61	81.97	12.41	0.00	-	22.53	51.76	25.72	0.00	-	53.93	34.38	11.69	0.00	-	15.49	28.49	55.90	0.11	-		
Intersection %	1.30	19.00	2.88	0.00	23.18	11.32	26.00	12.92	0.00	50.23	7.52	4.79	1.63	0.00	13.95	1.96	3.60	7.07	0.01	12.64		
PHF	0.80	0.88	0.79	0.00	0.88	0.95	0.96	0.96	0.00	0.96	0.82	0.89	0.86	0.00	0.95	0.88	0.91	0.92	0.00	0.91	0.97	

Preston Highway at Interchange Drive
Traffic Impact Study

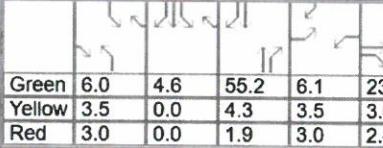
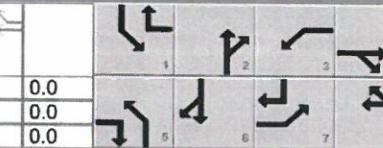
HCS Reports

HCS7 Signalized Intersection Results Summary											
General Information								Intersection Information			
Agency	Diane B. Zimmerman Traffic Engineering							Duration, h	0.250		
Analyst	DBZ		Analysis Date	Jun 2, 2021				Area Type	Other		
Jurisdiction			Time Period	AM Peak				PHF	0.91		
Urban Street	Preston Highway		Analysis Year	2021				Analysis Period	1> 7:15		
Intersection	Cooper Chapel Road		File Name	AM 21 Preston.xus							
Project Description	Stern										
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Demand (v), veh/h				85	4	5	78	49	418	28	1267
										65	201
Signal Information											
Cycle, s	125.7	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	Yes	Simult. Gap E/W	On								
Force Mode	Fixed	Simult. Gap N/S	On								
Timer Results											
Assigned Phase				EBL	EBT		WBL	WBT	NBL	NBT	SBL
				7	4		3	8	5	2	1
Case Number											6
Phase Duration, s				2.0	3.0		2.0	3.0	2.0	4.0	2.0
Change Period, (Y+R _c), s				13.5	30.4		13.5	30.4	12.5	64.5	17.4
Max Allow Headway (MAH), s				6.5	6.0		6.5	6.0	6.5	6.2	6.5
Queue Clearance Time (g _a), s				5.6	5.8		5.1	5.8	3.0	4.9	4.0
Green Extension Time (g _e), s				5.5	2.2		5.1	19.7	4.1	25.7	10.0
Phase Call Probability				0.7	4.2		0.4	4.6	0.0	32.6	0.8
Max Out Probability				1.00	1.00		1.00	1.00	1.00	1.00	1.00
				0.00	0.03		0.00	0.01	0.00	0.21	0.00
											0.17
Movement Group Results											
Approach Movement				EB		WB		NB		SB	
				L	T	R	L	T	R	L	T
Assigned Movement				7	4	14	3	8	18	5	2
Adjusted Flow Rate (v), veh/h											16
Adjusted Saturation Flow Rate (s), veh/h/in				93	4	5	86	54	459	30	964
											469
Queue Service Time (g _s), s				1647	1900	1425	1875	1900	1403	1753	1856
Cycle Queue Clearance Time (g _c), s											1807
Green Ratio (g/C)				3.5	0.2	0.2	3.1	3.0	17.7	2.1	23.7
Capacity (c), veh/h											23.7
Volume-to-Capacity Ratio (X)				3.5	0.2	0.2	3.1	3.0	17.7	2.1	23.7
Back of Queue (Q), ft/in (95 th percentile)											8.0
Back of Queue (Q), veh/in (95 th percentile)				73.4	5.1	2.9	65	63.9	258.3	42.8	371
Queue Storage Ratio (RQ) (95 th percentile)				2.8	0.2	0.1	2.5	2.6	10.2	1.7	14.5
Uniform Delay (d ₁), s/veh				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Incremental Delay (d ₂), s/veh											0.51
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				61.4	41.0	36.3	60.0	42.4	40.1	58.8	24.8
Level of Service (LOS)					E	D	E	D	D	E	C
Approach Delay, s/veh / LOS				59.2		E	43.1		D	25.6	C
Intersection Delay, s/veh / LOS							29.6				C
Multimodal Results											
Pedestrian LOS Score / LOS				EB		WB		NB		SB	
				2.60		C	3.28		C	2.57	C
Bicycle LOS Score / LOS				0.66		A	1.48		A	1.31	A
											1.18
											A

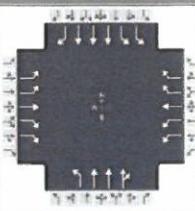
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																																																										
General Information						Intersection Information																																																				
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h		0.250																																																		
Analyst	DBZ		Analysis Date		Jun 2, 2021		Area Type		Other																																																	
Jurisdiction			Time Period		AM Peak		PHF		0.91																																																	
Urban Street	Preston Highway		Analysis Year		2022 No Build		Analysis Period		1 > 7:15																																																	
Intersection	Cooper Chapel Road		File Name		AM 22 NB Preston.xus																																																					
Project Description	Stern																																																									
Demand Information			EB		WB		NB		SB																																																	
Approach Movement			L	T	R	L	T	R	L	T	R																																															
Demand (v), veh/h			85	4	5	78	49	420	28	1273	65	202	796	456																																												
Signal Information			  																																																							
Cycle, s	122.4	Reference Phase	2		  																																																					
Offset, s	0	Reference Point	End		<table border="1"> <tr> <td>Green</td><td>6.0</td><td>4.7</td><td>56.5</td><td>6.0</td><td>24.1</td><td>0.0</td><td>1</td><td>2</td><td>3</td><td>4</td><td>5</td><td>6</td><td>7</td><td>8</td> </tr> <tr> <td>Yellow</td><td>3.5</td><td>0.0</td><td>4.3</td><td>3.5</td><td>3.6</td><td>0.0</td><td>1</td><td>2</td><td>3</td><td>4</td><td>5</td><td>6</td><td>7</td><td>8</td> </tr> <tr> <td>Red</td><td>3.0</td><td>0.0</td><td>1.9</td><td>3.0</td><td>2.4</td><td>0.0</td><td>1</td><td>2</td><td>3</td><td>4</td><td>5</td><td>6</td><td>7</td><td>8</td> </tr> </table>									Green	6.0	4.7	56.5	6.0	24.1	0.0	1	2	3	4	5	6	7	8	Yellow	3.5	0.0	4.3	3.5	3.6	0.0	1	2	3	4	5	6	7	8	Red	3.0	0.0	1.9	3.0	2.4	0.0	1	2	3	4	5	6	7	8
Green	6.0	4.7	56.5	6.0	24.1	0.0	1	2	3	4	5	6	7	8																																												
Yellow	3.5	0.0	4.3	3.5	3.6	0.0	1	2	3	4	5	6	7	8																																												
Red	3.0	0.0	1.9	3.0	2.4	0.0	1	2	3	4	5	6	7	8																																												
Uncoordinated	Yes	Simult. Gap E/W	On																																																							
Force Mode	Fixed	Simult. Gap N/S	On																																																							
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT																																																
Assigned Phase			7	4	3	8	5	2	1	6																																																
Case Number			2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0																																																
Phase Duration, s			12.6	30.1	12.5	30.0	12.5	62.7	17.2	67.4																																																
Change Period, (Y+R c), s			6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2																																																
Max Allow Headway (MAH), s			5.6	5.8	5.1	5.8	3.0	4.9	4.0	4.9																																																
Queue Clearance Time (g s), s			5.4	2.2	5.1	19.3	4.0	25.3	9.9	15.1																																																
Green Extension Time (g e), s			0.7	4.3	0.4	4.6	0.0	31.1	0.8	32.9																																																
Phase Call Probability			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00																																																
Max Out Probability			0.00	0.03	0.00	0.01	0.00	0.27	0.00	0.21																																																
Movement Group Results			EB		WB		NB		SB																																																	
Approach Movement			L	T	R	L	T	R	L	T	R																																															
Assigned Movement			7	4	14	3	8	18	5	2	12	1	6	16																																												
Adjusted Flow Rate (v), veh/h			93	4	5	86	54	462	30	967	471	222	875	171																																												
Adjusted Saturation Flow Rate (s), veh/h/in			1647	1900	1425	1675	1900	1403	1753	1856	1807	1689	1658	1585																																												
Queue Service Time (g s), s			3.4	0.2	0.2	3.1	2.9	17.3	2.0	23.3	23.3	7.9	13.1	6.7																																												
Cycle Queue Clearance Time (g c), s			3.4	0.2	0.2	3.1	2.9	17.3	2.0	23.3	23.3	7.9	13.1	6.7																																												
Green Ratio (g/C)			0.05	0.20	0.25	0.05	0.20	0.28	0.05	0.46	0.46	0.09	0.50	0.55																																												
Capacity (c), veh/h			164	374	700	164	372	795	86	1712	834	295	2486	871																																												
Volume-to-Capacity Ratio (X)			0.569	0.012	0.008	0.523	0.145	0.581	0.351	0.565	0.565	0.752	0.352	0.197																																												
Back of Queue (Q), ft/in (95 th percentile)			73	4.9	2.8	64.4	61.9	252.2	41.4	364.5	350.8	159	215.3	103																																												
Back of Queue (Q), veh/in (95 th percentile)			2.7	0.2	0.1	2.5	2.5	9.9	1.6	14.2	14.0	6.1	8.3	4.1																																												
Queue Storage Ratio (RQ) (95 th percentile)			0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.29	0.22	0.51																																												
Uniform Delay (d 1), s/veh			56.9	39.6	34.9	56.9	40.8	37.7	56.4	24.1	24.1	54.6	18.6	13.9																																												
Incremental Delay (d 2), s/veh			5.2	0.0	0.0	3.5	0.3	1.1	0.8	0.3	0.7	3.9	0.1	0.2																																												
Initial Queue Delay (d 3), s/veh			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																																												
Control Delay (d), s/veh			62.2	39.7	34.9	60.4	41.1	38.8	57.1	24.4	24.8	58.5	18.7	14.1																																												
Level of Service (LOS)			E	D	C	E	D	D	E	C	C	E	B	B																																												
Approach Delay, s/veh / LOS			59.8	E	42.1	D	25.2	C	25.1	C																																																
Intersection Delay, s/veh / LOS							29.1			C																																																
Multimodal Results			EB		WB		NB		SB																																																	
Pedestrian LOS Score / LOS			2.60	C	3.28	C	2.57	C	2.42	B																																																
Bicycle LOS Score / LOS			0.66	A	1.48	A	1.31	A	1.19	A																																																

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250				
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type	Other				
Jurisdiction			Time Period	AM Peak		PHF	0.91				
Urban Street	Preston Highway		Analysis Year	2022 Build		Analysis Period	1 > 7:15				
Intersection	Cooper Chapel Road		File Name	AM 22 B Preston.xus							
Project Description	Stern										
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Demand (v), veh/h				85	4	5	94	49	420	28	1288
										80	202
											812
											456
Signal Information				EB		WB		NB		SB	
Cycle, s	120.8	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	Yes	Simult. Gap E/W	On								
Force Mode	Fixed	Simult. Gap N/S	On								
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				7	4	3	8	5	2	1	6
Case Number				2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0
Phase Duration, s				12.6	29.6	12.6	29.7	12.5	61.4	17.1	66.0
Change Period, ($Y+R_c$), s				6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2
Max Allow Headway (MAH), s				5.6	5.8	5.1	5.8	3.0	4.9	4.0	4.9
Queue Clearance Time (g_s), s				5.4	2.2	5.7	19.0	3.9	24.3	9.8	15.3
Green Extension Time (g_e), s				0.7	4.3	0.5	4.6	0.0	30.8	0.8	32.3
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Max Out Probability				0.00	0.03	0.00	0.00	0.00	0.25	0.00	0.21
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Assigned Movement				7	4	14	3	8	18	5	2
Adjusted Flow Rate (v), veh/h				93	4	5	103	54	462	29	941
Adjusted Saturation Flow Rate (s), veh/h/in				1647	1900	1425	1675	1900	1403	1753	1856
Queue Service Time (g_s), s				3.4	0.2	0.2	3.7	2.8	17.0	1.9	22.3
Cycle Queue Clearance Time (g_c), s				3.4	0.2	0.2	3.7	2.8	17.0	1.9	22.3
Green Ratio (g/C)				0.05	0.20	0.25	0.05	0.20	0.28	0.05	0.46
Capacity (c), veh/h				165	372	700	170	374	798	87	1696
Volume-to-Capacity Ratio (X)				0.565	0.012	0.008	0.607	0.144	0.578	0.329	0.555
Back of Queue (Q), ft/in (95 th percentile)				71.9	4.8	2.8	77.5	60.9	248.9	38.7	350.7
Back of Queue (Q), veh/in (95 th percentile)				2.7	0.2	0.1	3.0	2.4	9.8	1.5	13.7
Queue Storage Ratio (RQ) (95 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d_1), s/veh				56.1	39.2	34.5	56.2	40.2	37.0	55.5	23.9
Incremental Delay (d_2), s/veh				5.1	0.0	0.0	4.7	0.3	1.1	0.7	0.3
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				61.3	39.2	34.5	60.9	40.4	38.1	56.2	24.2
Level of Service (LOS)				E	D	C	E	D	D	E	C
Approach Delay, s/veh / LOS				58.9		E	42.1		D	25.0	C
Intersection Delay, s/veh / LOS							29.1			C	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				2.60		C	3.28		C	2.57	C
Bicycle LOS Score / LOS				0.66		A	1.51		B	1.33	A

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250				
Analyst	DBZ	Analysis Date	Jun 2, 2021			Area Type	Other				
Jurisdiction		Time Period	AM Peak			PHF	0.91				
Urban Street	Preston Highway	Analysis Year	2032 No Build			Analysis Period	1> 7:15				
Intersection	Cooper Chapel Road	File Name	AM 32 NB Preston.xus								
Project Description	Stern										
Demand Information			EB			WB			NB		SB
Approach Movement			L	T	R	L	T	R	L	T	R
Demand (v), veh/h			89	4	5	82	52	441	29	1338	68
Signal Information											
Cycle, s	130.8	Reference Phase	2								
Offset, s	0	Reference Point	End	Green	6.0	5.7	61.1	6.0	0.5	26.2	
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	0.0	4.3	3.5	0.0	3.6	
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	0.0	1.9	3.0	0.0	2.4	
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase			7	4	3	8	5	2	1	6	
Case Number			2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0	
Phase Duration, s			13.0	32.8	12.5	32.2	12.5	67.3	18.2	73.0	
Change Period, (Y+R c), s			6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2	
Max Allow Headway (MAH), s			5.6	5.8	5.1	5.8	3.0	4.9	4.0	4.9	
Queue Clearance Time (g s), s			5.8	2.2	5.5	21.4	4.2	27.9	10.8	16.5	
Green Extension Time (g e), s			0.7	4.5	0.4	4.8	0.0	33.2	0.8	35.8	
Phase Call Probability			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Max Out Probability			0.00	0.03	0.00	0.01	0.00	0.34	0.00	0.28	
Movement Group Results			EB			WB			NB		SB
Approach Movement			L	T	R	L	T	R	L	T	R
Assigned Movement			7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h			98	4	5	90	57	485	31	1004	489
Adjusted Saturation Flow Rate (s), veh/h/ln			1847	1900	1425	1675	1900	1403	1753	1856	1807
Queue Service Time (g s), s			3.8	0.2	0.2	3.5	3.2	19.4	2.2	25.9	25.9
Cycle Queue Clearance Time (g c), s			3.8	0.2	0.2	3.5	3.2	19.4	2.2	25.9	25.9
Green Ratio (g/C)			0.05	0.20	0.25	0.05	0.20	0.29	0.05	0.47	0.47
Capacity (c), veh/h			165	389	714	154	381	814	80	1734	845
Volume-to-Capacity Ratio (X)			0.593	0.011	0.008	0.587	0.150	0.595	0.383	0.579	0.579
Back of Queue (Q), ft/ln (95 th percentile)			82.1	5.2	3	73.8	70.2	278.3	45.9	403	387.8
Back of Queue (Q), veh/ln (95 th percentile)			3.1	0.2	0.1	2.8	2.8	11.0	1.8	15.7	15.5
Queue Storage Ratio (RQ) (95 th percentile)			0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.33
Uniform Delay (d 1), s/veh			60.9	41.5	36.8	61.2	43.1	39.9	60.6	25.5	25.5
Incremental Delay (d 2), s/veh			5.7	0.0	0.0	4.8	0.3	1.2	0.9	0.4	0.7
Initial Queue Delay (d 3), s/veh			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh			66.6	41.5	36.8	66.0	43.4	41.0	61.6	25.8	26.2
Level of Service (LOS)			E	D	D	E	D	D	E	C	C
Approach Delay, s/veh / LOS			64.1	E		44.8	D		26.7	C	26.1
Intersection Delay, s/veh / LOS						30.7				C	
Multimodal Results			EB			WB			NB		SB
Pedestrian LOS Score / LOS			2.60	C		3.28	C		2.57	C	2.42
Bicycle LOS Score / LOS			0.67	A		1.53	B		1.35	A	1.23

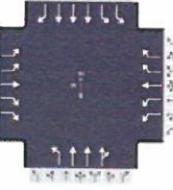
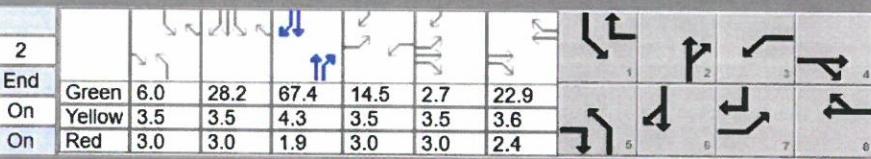
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary															
General Information								Intersection Information							
Agency		Diane B. Zimmerman Traffic Engineering								Duration, h	0.250				
Analyst		DBZ		Analysis Date		Jun 2, 2021		Area Type		Other					
Jurisdiction				Time Period		AM Peak		PHF		0.91					
Urban Street		Preston Highway		Analysis Year		2032 Build		Analysis Period		1 > 7:15					
Intersection		Cooper Chapel Road		File Name		AM 32 B Preston.xus									
Project Description															
Demand Information				EB			WB			NB					
Approach Movement				L	T	R	L	T	R	L	T				
Demand (v), veh/h				89	4	5	98	52	441	29	1353				
										83					
										212	853				
										479					
Signal Information															
Cycle, s	129.2	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase				7	4	3	8	5	2	1	6				
Case Number				2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0				
Phase Duration, s				13.0	31.9	13.1	32.0	12.5	66.1	18.1	71.7				
Change Period, (Y+R _c), s				6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2				
Max Allow Headway (MAH), s				5.6	5.8	5.1	5.8	3.0	4.9	4.0	4.9				
Queue Clearance Time (g _s), s				5.8	2.2	6.1	21.1	4.1	26.9	10.7	16.8				
Green Extension Time (g _e), s				0.7	4.5	0.5	4.8	0.0	33.0	0.8	35.2				
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Max Out Probability				0.00	0.03	0.00	0.01	0.00	0.33	0.00	0.27				
Movement Group Results				EB			WB			NB					
Approach Movement				L	T	R	L	T	R	L	T				
Assigned Movement				7	4	14	3	8	18	5	2				
Adjusted Flow Rate (v), veh/h				98	4	5	108	57	485	29	979				
Adjusted Saturation Flow Rate (s), veh/h/in				1647	1900	1425	1675	1900	1403	1753	1856				
Queue Service Time (g _s), s				3.8	0.2	0.2	4.1	3.2	19.1	2.1	24.9				
Cycle Queue Clearance Time (g _c), s				3.8	0.2	0.2	4.1	3.2	19.1	2.1	24.9				
Green Ratio (g/C)				0.05	0.20	0.25	0.05	0.20	0.29	0.05	0.46				
Capacity (c), veh/h				166	381	704	171	383	817	81	1720				
Volume-to-Capacity Ratio (X)				0.590	0.012	0.008	0.631	0.149	0.593	0.361	0.569				
Back of Queue (Q), ft/in (95 th percentile)				81.1	5.2	3	87.1	69.3	275.1	43.1	388.4				
Back of Queue (Q), veh/in (95 th percentile)				3.0	0.2	0.1	3.3	2.8	10.8	1.7	15.2				
Queue Storage Ratio (RQ) (95 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Uniform Delay (d ₁), s/veh				60.1	41.4	36.7	60.2	42.5	39.3	59.8	25.3				
Incremental Delay (d ₂), s/veh				5.6	0.0	0.0	5.2	0.3	1.1	0.8	0.4				
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.7	0.1				
Control Delay (d), s/veh				65.7	41.4	36.7	65.4	42.8	40.4	60.7	25.6				
Level of Service (LOS)				E	D	D	E	D	D	E	B				
Approach Delay, s/veh / LOS				63.3			44.8			26.4					
Intersection Delay, s/veh / LOS							30.7			C					
Multimodal Results				EB			WB			NB					
Pedestrian LOS Score / LOS				2.60			3.28			2.57					
Bicycle LOS Score / LOS				0.67			A			1.37					

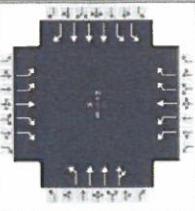
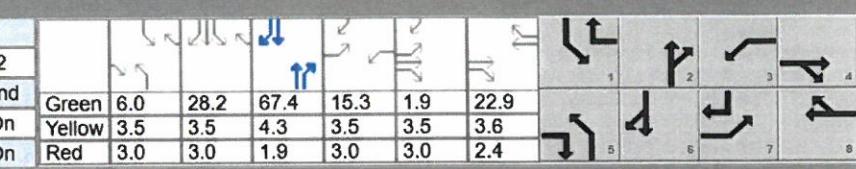
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering		Duration, h	0.250							
Analyst	DBZ	Analysis Date	Jun 2, 2021		Area Type	Other					
Jurisdiction		Time Period	PM Peak		PHF	0.98					
Urban Street	Preston Highway	Analysis Year	2021		Analysis Period	1 > 4:45					
Intersection	Cooper Chapel Rd	File Name	PM 21 Preston.xus								
Project Description	Stern										
Demand Information			EB			WB			NB		
Approach Movement			L	T	R	L	T	R	L	T	R
Demand (v), veh/h			354	61	46	210	15	358	12	1178	214
Signal Information			EB			WB			NB		
Cycle, s	180.0	Reference Phase	2								
Offset, s	0	Reference Point	End	Green	6.0	28.1	67.8	14.5	2.6	22.8	
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	3.5	4.3	3.5	3.5	3.6	
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.9	3.0	3.0	2.4	
Timer Results			EBL			WBL			NBL		
Assigned Phase			7	4		3	8		5	2	
Case Number			2.0	3.0		2.0	3.0		2.0	4.0	
Phase Duration, s			30.1	38.0		21.0	28.8		12.5	74.0	
Change Period, (Y+R_c), s			6.5	6.0		6.5	6.0		6.5	6.2	
Max Allow Headway (MAH), s			5.6	5.7		5.1	5.7		3.0	0.0	
Queue Clearance Time (g_s), s			20.9	7.0		13.1	19.3		3.3		38.1
Green Extension Time (g_e), s			2.7	4.2		1.3	3.5		0.0	0.0	
Phase Call Probability			1.00	1.00		1.00	1.00		1.00		1.00
Max Out Probability			0.00	0.00		0.00	0.04		0.00		0.09
Movement Group Results			EB			WB			NB		
Approach Movement			L	T	R	L	T	R	L	T	R
Assigned Movement			7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h			361	62	47	214	15	365	12	947	435
Adjusted Saturation Flow Rate (s), veh/h/in			1675	1900	1425	1702	1900	1414	1697	1885	1733
Queue Service Time (g_s), s			18.9	5.0	2.4	11.1	1.3	17.3	1.3	38.6	38.2
Cycle Queue Clearance Time (g_c), s			18.9	5.0	2.4	11.1	1.3	17.3	1.3	38.6	38.2
Green Ratio (g/C)			0.13	0.18	0.21	0.08	0.13	0.35	0.03	0.38	0.38
Capacity (c), veh/h			439	337	601	274	241	996	57	1420	653
Volume-to-Capacity Ratio (X)			0.823	0.184	0.078	0.783	0.063	0.367	0.211	0.667	0.667
Back of Queue (Q), ft/in (95 th percentile)			350.2	111.7	39.3	227.6	28.7	257.3	26.9	635.5	592.1
Back of Queue (Q), veh/in (95 th percentile)			13.4	4.5	1.6	8.8	1.1	10.2	1.0	25.2	23.7
Queue Storage Ratio (RQ) (95 th percentile)			0.78	0.25	0.31	0.57	0.05	0.74	0.06	0.53	0.50
Uniform Delay (d_1), s/veh			76.2	62.9	57.0	81.2	69.2	43.4	87.5	49.7	48.5
Incremental Delay (d_2), s/veh			6.5	0.4	0.1	6.7	0.2	0.4	0.6	2.2	4.8
Initial Queue Delay (d_3), s/veh			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh			82.7	63.3	57.1	87.9	69.3	43.7	88.1	51.9	53.3
Level of Service (LOS)			F	E	E	F	E	D	F	D	D
Approach Delay, s/veh / LOS			77.6			60.3			52.7		
Intersection Delay, s/veh / LOS						44.5				D	
Multimodal Results			EB			WB			NB		
Pedestrian LOS Score / LOS			2.62			2.88			2.59		
Bicycle LOS Score / LOS			1.26			1.47			1.28		

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																						
General Information						Intersection Information																
Agency	Diane B. Zimmerman Traffic Engineering			Duration, h			0.250															
Analyst	DBZ		Analysis Date		Jun 2, 2021		Area Type															
Jurisdiction				Time Period		PM Peak		PHF														
Urban Street	Preston Highway		Analysis Year		2022 No Build		Analysis Period		1>4:45													
Intersection	Cooper Chapel			File Name		PM 22 NB Preston.xus																
Project Description	Stern																					
Demand Information				EB		WB		NB		SB												
Approach Movement				L	T	R	L	T	R	L	T	R										
Demand (<i>v</i>), veh/h				356	61	46	211	15	360	12	1184	215										
				711			1777			111												
Signal Information																						
Cycle, s	180.0	Reference Phase	2																			
Offset, s	0	Reference Point	End																			
Uncoordinated	No	Simult. Gap E/W	On	Green	6.0	28.2	67.4	14.5	2.7	22.9												
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	3.5	3.5	3.6												
				Red	3.0	3.0	1.9	3.0	3.0	2.4												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT											
Assigned Phase				7	4	3	8	5	2	1	6											
Case Number				2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0											
Phase Duration, s				30.2	38.1	21.0	28.9	12.5	73.6	47.2	108.4											
Change Period, (<i>Y+R</i> c), s				6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2											
Max Allow Headway (MAH), s				5.6	5.7	5.1	5.7	3.0	0.0	4.0	0.0											
Queue Clearance Time (g s), s				21.0	7.0	13.2	19.4	3.3		38.3												
Green Extension Time (g e), s				2.7	4.2	1.3	3.5	0.0	0.0	2.4	0.0											
Phase Call Probability				1.00	1.00	1.00	1.00	1.00		1.00												
Max Out Probability				0.00	0.00	0.00	0.04	0.00		0.10												
Movement Group Results				EB		WB		NB		SB												
Approach Movement				L	T	R	L	T	R	L	T	R										
Assigned Movement				7	4	14	3	8	18	5	2	12										
Adjusted Flow Rate (<i>v</i>), veh/h				363	62	47	215	15	367	12	951	437										
Adjusted Saturation Flow Rate (s), veh/h/in				1675	1900	1425	1702	1900	1414	1697	1885	1733										
Queue Service Time (g s), s				19.0	5.0	2.4	11.2	1.3	17.4	1.3	39.0	38.6										
Cycle Queue Clearance Time (g c), s				19.0	5.0	2.4	11.2	1.3	17.4	1.3	39.0	38.6										
Green Ratio (g/C)				0.13	0.18	0.21	0.08	0.13	0.35	0.03	0.37	0.37										
Capacity (c), veh/h				441	339	603	275	242	1000	57	1413	649										
Volume-to-Capacity Ratio (X)				0.824	0.184	0.078	0.784	0.063	0.367	0.211	0.674	0.674										
Back of Queue (Q), ft/in (95 th percentile)				351.6	111.6	39.2	228.4	28.7	258	26.9	642.8	599										
Back of Queue (Q), veh/in (95 th percentile)				13.4	4.5	1.6	8.9	1.1	10.2	1.0	25.5	24.0										
Queue Storage Ratio (RQ) (95 th percentile)				0.78	0.25	0.31	0.57	0.05	0.74	0.06	0.54	0.50										
Uniform Delay (d 1), s/veh				76.1	62.8	56.9	81.2	69.1	43.2	87.5	50.3	49.2										
Incremental Delay (d 2), s/veh				6.5	0.4	0.1	6.6	0.2	0.4	0.6	2.3	5.0										
Initial Queue Delay (d 3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0										
Control Delay (d), s/veh				82.6	63.2	56.9	87.8	69.3	43.6	88.1	52.6	54.1										
Level of Service (LOS)				F	E	E	F	E	D	F	D	D										
Approach Delay, s/veh / LOS				77.5			60.2			53.4												
Intersection Delay, s/veh / LOS							44.7				D											
Multimodal Results				EB		WB		NB		SB												
Pedestrian LOS Score / LOS				2.62			2.88			2.59												
Bicycle LOS Score / LOS				1.27			A			1.28												

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary												
General Information						Intersection Information						
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250					
Analyst	DBZ		Analysis Date	Jun 2, 2021			Area Type	Other				
Jurisdiction			Time Period	PM Peak			PHF	0.98				
Urban Street	Preston Highway		Analysis Year	2022 Build			Analysis Period	1> 4:45				
Intersection	Cooper Chapel		File Name	PM 22 B Preston.xus								
Project Description	Stern											
Demand Information			EB			WB			NB		SB	
Approach Movement			L	T	R	L	T	R	L	T	R	
Demand (v), veh/h			356	61	46	225	15	360	12	1197	228	
Signal Information												
Cycle, s	180.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT		
Assigned Phase			7	4	3	8	5	2	1	6		
Case Number			2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0		
Phase Duration, s			30.2	37.3	21.8	28.9	12.5	73.6	47.2	108.4		
Change Period, (Y+R c), s			6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2		
Max Allow Headway (MAH), s			5.6	5.7	5.1	5.7	3.0	0.0	4.0	0.0		
Queue Clearance Time (g s), s			21.0	7.0	13.9	19.4	3.2		38.3			
Green Extension Time (g e), s			2.7	4.2	1.4	3.5	0.0	0.0	2.4	0.0		
Phase Call Probability			1.00	1.00	1.00	1.00	1.00		1.00			
Max Out Probability			0.00	0.00	0.00	0.04	0.00		0.10			
Movement Group Results			EB			WB			NB		SB	
Approach Movement			L	T	R	L	T	R	L	T	R	
Assigned Movement			7	4	14	3	8	18	5	2	12	
Adjusted Flow Rate (v), veh/h			363	62	47	230	15	367	12	950	435	
Adjusted Saturation Flow Rate (s), veh/h/ln			1675	1900	1425	1702	1900	1414	1697	1885	1727	
Queue Service Time (g s), s			19.0	5.0	2.4	11.9	1.3	17.4	1.2	28.9	27.1	
Cycle Queue Clearance Time (g c), s			19.0	5.0	2.4	11.9	1.3	17.4	1.2	28.9	27.1	
Green Ratio (g/C)			0.13	0.17	0.21	0.09	0.13	0.35	0.03	0.37	0.37	
Capacity (c), veh/h			441	330	590	290	242	1000	57	1413	647	
Volume-to-Capacity Ratio (X)			0.824	0.189	0.080	0.791	0.063	0.367	0.206	0.673	0.673	
Back of Queue (Q), ft/ln (95 th percentile)			351.6	112.2	39.5	240.1	28.7	258	26.2	362.8	302.3	
Back of Queue (Q), veh/ln (95 th percentile)			13.4	4.5	1.6	9.3	1.1	10.2	1.0	14.4	12.1	
Queue Storage Ratio (RQ) (95 th percentile)			0.78	0.25	0.32	0.60	0.05	0.74	0.06	0.30	0.25	
Uniform Delay (d 1), s/veh			76.1	63.5	57.5	80.8	69.1	43.2	87.4	25.8	22.8	
Incremental Delay (d 2), s/veh			6.5	0.4	0.1	6.5	0.2	0.4	0.4	1.7	3.7	
Initial Queue Delay (d 3), s/veh			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh			82.6	63.9	57.6	87.3	69.3	43.6	87.9	27.5	26.5	
Level of Service (LOS)			F	E	E	F	E	D	F	C	C	
Approach Delay, s/veh / LOS			77.7	E		60.6	E		27.7	C	30.8	
Intersection Delay, s/veh / LOS						37.8				D		
Multimodal Results			EB			WB			NB		SB	
Pedestrian LOS Score / LOS			2.62	C		2.88	C		2.59	C	2.42	
Bicycle LOS Score / LOS			1.27	A		1.50	A		1.29	A	1.95	

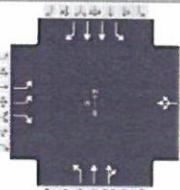
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250				
Analyst	DBZ	Analysis Date	Jun 2, 2021			Area Type	Other				
Jurisdiction		Time Period	PM Peak			PHF	0.98				
Urban Street	Preston Highway	Analysis Year	2032 No Build			Analysis Period	1 > 4:45				
Intersection	Cooper Chapel	File Name	PM 32 NB Preston.xus								
Project Description	Stern										
Demand Information				EB		WB		NB		SB	
Approach Movement	L	T	R	L	T	R	L	T	R	L	T
Demand (v), veh/h	374	64	48	222	16	378	13	1245	226	747	1868
Signal Information											
Cycle, s	180.0	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	No	Simult. Gap E/W	On	Green	6.0	29.8	64.1	15.2	3.1	23.6	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	3.5	3.5	3.6	
				Red	3.0	3.0	1.9	3.0	3.0	2.4	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				7	4	3	8	5	2	1	6
Case Number				2.0	3.0	2.0	3.0	2.0	4.0	2.0	3.0
Phase Duration, s				31.3	39.2	21.7	29.6	12.5	70.3	48.8	106.6
Change Period, (Y+R_c), s				6.5	6.0	6.5	6.0	6.5	6.2	6.5	6.2
Max Allow Headway (MAH), s				5.6	5.7	5.1	5.7	3.0	0.0	4.0	0.0
Queue Clearance Time (g_s), s				22.0	7.2	13.7	20.0	3.4		40.2	
Green Extension Time (g_e), s				2.8	4.5	1.4	3.6	0.0	0.0	2.0	0.0
Phase Call Probability				1.00	1.00	1.00	1.00	1.00		1.00	
Max Out Probability				0.01	0.00	0.00	0.07	0.00		0.36	
Movement Group Results				EB		WB		NB		SB	
Approach Movement	L	T	R	L	T	R	L	T	R	L	T
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6
Adjusted Flow Rate (v), veh/h	382	65	49	227	16	386	13	997	458	762	1906
Adjusted Saturation Flow Rate (s), veh/h/in	1675	1900	1425	1702	1900	1414	1697	1885	1733	1743	1698
Queue Service Time (g_s), s	20.0	5.2	2.5	11.7	1.4	18.0	1.4	42.4	42.1	38.2	34.4
Cycle Queue Clearance Time (g_c), s	20.0	5.2	2.5	11.7	1.4	18.0	1.4	42.4	42.1	38.2	34.4
Green Ratio (g/C)	0.14	0.18	0.22	0.08	0.13	0.37	0.03	0.36	0.36	0.24	0.56
Capacity (c), veh/h	461	351	621	287	249	1036	57	1343	617	839	2842
Volume-to-Capacity Ratio (X)	0.828	0.186	0.079	0.789	0.065	0.372	0.227	0.742	0.742	0.909	0.671
Back of Queue (Q), ft/in (95 th percentile)	365.7	116.2	40.6	237.6	30.4	265.3	29.1	693.9	651.2	630	390.9
Back of Queue (Q), veh/in (95 th percentile)	14.0	4.6	1.6	9.2	1.2	10.5	1.1	27.5	26.0	25.0	15.4
Queue Storage Ratio (RQ) (95 th percentile)	0.81	0.26	0.32	0.59	0.06	0.76	0.07	0.58	0.55	1.15	0.39
Uniform Delay (d_1), s/veh	75.5	62.0	56.0	80.8	68.5	41.9	87.6	53.5	52.4	66.4	14.6
Incremental Delay (d_2), s/veh	6.4	0.4	0.1	6.6	0.2	0.4	0.7	3.3	7.0	11.7	1.3
Initial Queue Delay (d_3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	82.0	62.3	56.1	87.4	68.7	42.2	88.3	56.8	59.4	78.1	15.9
Level of Service (LOS)	F	E	E	F	E	D	F	E	E	E	B
Approach Delay, s/veh / LOS	76.8			59.2			57.9			32.5	
Intersection Delay, s/veh / LOS				46.6						D	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.62	C		2.88	C		2.60	C		2.43	B
Bicycle LOS Score / LOS	1.31	A		1.52	B		1.32	A		2.02	B

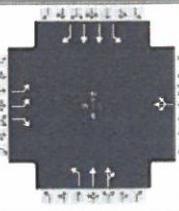
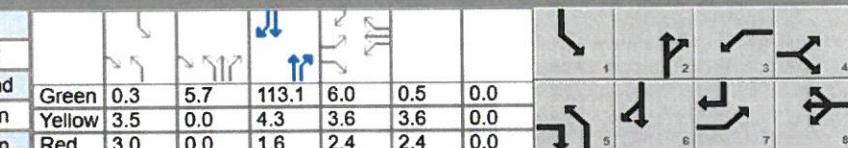
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																	
General Information						Intersection Information											
Agency						Duration, h											
Analyst	DBZ	Analysis Date		Jun 2, 2021		Area Type		Other									
Jurisdiction		Time Period		PM Peak		PHF		0.98									
Urban Street	Preston Highway	Analysis Year		2032 Build		Analysis Period		1 > 4:45									
Intersection	Cooper Chapel	File Name		PM 32 B Preston.xus													
Project Description	Stern																
Demand Information				EB			WB			NB							
Approach Movement				L	T	R	L	T	R	L	T						
Demand (v), veh/h				374	64	48	236	16	378	13	1258						
										239	239						
Signal Information																	
Cycle, s	180.0	Reference Phase	2														
Offset, s	0	Reference Point	End	Green	6.0	29.8	64.1	16.0	2.3	23.6							
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	3.5	4.3	3.5	3.5	3.6							
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.9	3.0	3.0	2.4							
Timer Results				EBL	EBT		WBL	WBT		NBL	NBT						
Assigned Phase				7	4		3	8		5	2						
Case Number				2.0	3.0		2.0	3.0		2.0	4.0						
Phase Duration, s				31.3	38.4		22.5	29.6		12.5	70.3						
Change Period, (Y+R c), s				6.5	6.0		6.5	6.0		6.5	6.2						
Max Allow Headway (MAH), s				5.6	5.7		5.1	5.7		3.0	0.0						
Queue Clearance Time (g s), s				22.0	7.3		14.5	20.0		3.3							
Green Extension Time (g e), s				2.8	4.4		1.5	3.6		0.0	0.0						
Phase Call Probability				1.00	1.00		1.00	1.00		1.00							
Max Out Probability				0.01	0.00		0.00	0.07		0.00							
											0.36						
Movement Group Results				EB			WB			NB							
Approach Movement				L	T	R	L	T	R	L	T						
Assigned Movement				7	4	14	3	8	18	5	2						
Adjusted Flow Rate (v), veh/h				382	65	49	241	16	386	13	991						
Adjusted Saturation Flow Rate (s), veh/h/in				1675	1900	1425	1702	1900	1414	1697	1885						
Queue Service Time (g s), s				20.0	5.3	2.5	12.5	1.4	18.0	1.3	33.0						
Cycle Queue Clearance Time (g c), s				20.0	5.3	2.5	12.5	1.4	18.0	1.3	33.0						
Green Ratio (g/C)				0.14	0.18	0.21	0.09	0.13	0.37	0.03	0.36						
Capacity (c), veh/h				461	342	608	303	249	1036	57	1343						
Volume-to-Capacity Ratio (X)				0.828	0.191	0.081	0.796	0.065	0.372	0.222	0.738						
Back of Queue (Q), ft/in (95 th percentile)				365.7	117	40.9	249.1	30.4	265.3	28.1	401.1						
Back of Queue (Q), veh/in (95 th percentile)				14.0	4.7	1.6	9.7	1.2	10.5	1.1	15.9						
Queue Storage Ratio (RQ) (95 th percentile)				0.81	0.26	0.33	0.62	0.06	0.76	0.07	0.33						
Uniform Delay (d 1), s/veh				75.5	62.7	56.7	80.4	68.5	41.9	87.5	28.1						
Incremental Delay (d 2), s/veh				6.4	0.4	0.1	6.5	0.2	0.4	0.4	2.2						
Initial Queue Delay (d 3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0						
Control Delay (d), s/veh				82.0	63.1	56.8	86.9	68.7	42.2	87.9	30.3						
Level of Service (LOS)				F	E	E	F	E	D	F	C						
Approach Delay, s/veh / LOS				77.0		E	59.6		E	30.6	C						
Intersection Delay, s/veh / LOS							39.3				D						
Multimodal Results				EB			WB			NB							
Pedestrian LOS Score / LOS				2.62		C	2.88		C	2.60	C						
Bicycle LOS Score / LOS				1.31		A	1.55		B	1.34	A						

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																				
General Information						Intersection Information														
Agency	Diane B. Zimmerman Traffic Engineering			Duration, h			0.250													
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type														
Jurisdiction			Time Period	AM Peak		PHF														
Urban Street	Preston Highway		Analysis Year	2021		Analysis Period														
Intersection	Interchange Drive		File Name	AM 21 Preston.xus																
Project Description	Stern																			
Demand Information				EB		WB		NB		SB										
Approach Movement				L	T	R	L	T	R	L	T	R								
Demand (v), veh/h				67		15	1	0	1	10	1315	3								
Signal Information																				
Cycle, s	150.0	Reference Phase	2																	
Offset, s	0	Reference Point	End																	
Uncoordinated	No	Simult. Gap E/W	Off	Green	0.3	5.7	113.1	6.0	0.5	0.0										
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	4.3	3.6	3.6	0.0										
Timer Results				EBL		EBT		WBL		WBT										
Assigned Phase						4		8		5										
Case Number						9.0		12.0		2.0										
Phase Duration, s						12.0		6.5		12.5										
Change Period, (Y+R_c), s						6.0		6.0		6.5										
Max Allow Headway (MAH), s						4.0		3.1		4.0										
Queue Clearance Time (g_s), s								2.2		3.0										
Green Extension Time (g_e), s						0.0		0.0		0.0										
Phase Call Probability								0.09		1.00										
Max Out Probability								0.00		0.04										
Movement Group Results				EB		WB		NB		SB										
Approach Movement				L	T	R	L	T	R	L	T	R								
Assigned Movement				7		14	3	8	18	5	2	12								
Adjusted Flow Rate (v), veh/h				74		17		2		11	700	700								
Adjusted Saturation Flow Rate (s), veh/h/in				1661		1359		1704		1527	1856	1854								
Queue Service Time (g_s), s				3.3		1.7		0.2		1.0	12.5	12.5								
Cycle Queue Clearance Time (g_c), s				3.3		1.7		0.2		1.0	12.5	12.5								
Green Ratio (g/C)				0.04		0.08		0.00		0.60	0.79	0.79								
Capacity (c), veh/h				133		109		6		61	1470	1468								
Volume-to-Capacity Ratio (X)				0.560		0.153		0.368		0.174	0.477	0.477								
Back of Queue (Q), ft/in (95 th percentile)				68.7		31.4		4.8		21.6	138.8	135.5								
Back of Queue (Q), veh/in (95 th percentile)				2.6		1.1		0.2		0.7	5.4	5.4								
Queue Storage Ratio (RQ) (95 th percentile)				0.17		0.52		0.00		0.09	0.00	0.00								
Uniform Delay (d_1), s/veh				70.7		64.3		74.6		71.7	2.8	2.8								
Incremental Delay (d_2), s/veh				3.7		0.6		13.3		0.9	0.8	0.8								
Initial Queue Delay (d_3), s/veh				0.0		0.0		0.0		0.0	0.0	0.0								
Control Delay (d), s/veh				74.4		64.9		87.9		72.6	3.6	3.6								
Level of Service (LOS)				E		E		F		E	A	A								
Approach Delay, s/veh / LOS				72.6		E		87.9		F	A	A								
Intersection Delay, s/veh / LOS						7.4				A										
Multimodal Results				EB		WB		NB		SB										
Pedestrian LOS Score / LOS				2.33		B		2.49		B		2.05								
Bicycle LOS Score / LOS						F		0.49		A		1.29								

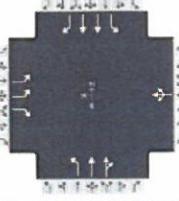
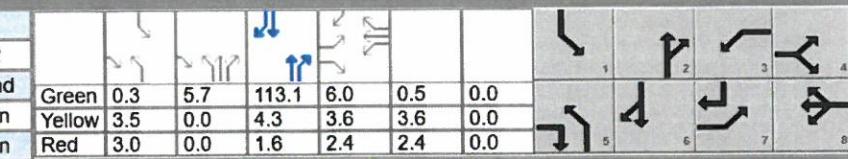
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																			
General Information						Intersection Information													
Agency	Diane B. Zimmerman Traffic Engineering		Duration, h	0.250															
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type	Other												
Jurisdiction			Time Period	AM Peak		PHF	0.90												
Urban Street	Preston Highway		Analysis Year	2022 No Build		Analysis Period	1 > 7:15												
Intersection	Interchange Drive		File Name	AM 22 NB Preston.xus															
Project Description	Stern																		
Demand Information			EB		WB		NB		SB										
Approach Movement	L	T	R	L	T	R	L	T	R	L	T								
Demand (v), veh/h	67		15	1	0	1	10	1322	3	1	725	153							
Signal Information																			
Cycle, s	150.0	Reference Phase	2																
Offset, s	0	Reference Point	End	Green	0.3	5.7	113.1	6.0	0.5	0.0									
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	0.0	4.3	3.6	3.6	0.0									
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	0.0	1.6	2.4	2.4	0.0									
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT									
Assigned Phase				4			8	5	2	1	6								
Case Number				9.0			12.0	2.0	4.0	2.0	3.0								
Phase Duration, s				12.0			6.5	12.5	124.7	6.8	119.0								
Change Period, (Y+R c), s				6.0			6.0	6.5	5.9	6.5	5.9								
Max Allow Headway (MAH), s				4.0			3.1	4.0	0.0	4.0	0.0								
Queue Clearance Time (g s), s							2.2	3.0		2.1									
Green Extension Time (g e), s				0.0			0.0	0.0	0.0	0.0	0.0								
Phase Call Probability							0.09	1.00		0.04									
Max Out Probability							0.00	0.00		0.00									
Movement Group Results			EB		WB		NB		SB										
Approach Movement	L	T	R	L	T	R	L	T	R	L	T								
Assigned Movement	7		14	3	8	18	5	2	12	1	6	16							
Adjusted Flow Rate (v), veh/h	74		17		2		11	703	703	1	785	166							
Adjusted Saturation Flow Rate (s), veh/h/in	1661		1359		1704		1527	1856	1854	1810	1724	1585							
Queue Service Time (g s), s	3.3		1.7		0.2		1.0	12.6	12.6	0.1	10.9	3.6							
Cycle Queue Clearance Time (g c), s	3.3		1.7		0.2		1.0	12.6	12.6	0.1	10.9	3.6							
Green Ratio (g/C)	0.04		0.08		0.00		0.60	0.79	0.79	0.00	0.75	0.79							
Capacity (c), veh/h	133		109		6		61	1470	1468	3	2599	1258							
Volume-to-Capacity Ratio (X)	0.560		0.153		0.368		0.174	0.478	0.479	0.338	0.302	0.132							
Back of Queue (Q), ft/in (95 th percentile)	68.7		31.4		4.8		21.6	140	136.7	3.8	159.6	43.9							
Back of Queue (Q), veh/in (95 th percentile)	2.6		1.1		0.2		0.7	5.5	5.5	0.2	6.1	1.7							
Queue Storage Ratio (RQ) (95 th percentile)	0.17		0.52		0.00		0.09	0.00	0.00	0.04	0.00	0.15							
Uniform Delay (d 1), s/veh	70.7		64.3		74.6		71.7	2.8	2.8	74.8	5.9	3.6							
Incremental Delay (d 2), s/veh	3.7		0.6		13.3		0.9	0.7	0.7	49.8	0.3	0.2							
Initial Queue Delay (d 3), s/veh	0.0		0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0							
Control Delay (d), s/veh	74.4		64.9		87.9		72.6	3.6	3.6	124.5	6.2	3.8							
Level of Service (LOS)	E		E		F		E	A	A	F	A	A							
Approach Delay, s/veh / LOS	72.6	E		87.9	F		4.1	A		5.9	A								
Intersection Delay, s/veh / LOS				7.4					A										
Multimodal Results			EB		WB		NB		SB										
Pedestrian LOS Score / LOS	2.33	B		2.49	B		1.62	B		2.05	B								
Bicycle LOS Score / LOS		F		0.49	A		1.71	B		1.29	A								

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																					
General Information						Intersection Information			Diagram												
Agency		Diane B. Zimmerman Traffic Engineering						Duration, h	0.250												
Analyst		DBZ		Analysis Date		Jun 2, 2021		Area Type	Other												
Jurisdiction		Time Period		AM Peak		PHF		0.90													
Urban Street		Preston Highway		Analysis Year		2022 Build		Analysis Period	1 > 7:15												
Intersection		Interchange Drive		File Name		AM 22 B Preston.xus															
Project Description																					
Demand Information						EB			WB												
Approach Movement			L T R			L T R			L T R												
Demand (v), veh/h			59 24 15			187 16 1			10 1298 132												
Signal Information																					
Cycle, s	150.0	Reference Phase	2																		
Offset, s	0	Reference Point	End																		
Uncoordinated	No	Simult. Gap E/W	On																		
Force Mode	Fixed	Simult. Gap N/S	On																		
Timer Results						EBL		EBT		WBL		WBT		NBL		NBT		SBL		SBT	
Assigned Phase								4				8		5		2		1		6	
Case Number								9.0				10.0		2.0		4.0		2.0		3.0	
Phase Duration, s								16.0				25.6		12.5		89.8		18.7		95.9	
Change Period, (Y+R _c), s								6.0		6.0		6.5		5.9		6.5		5.9			
Max Allow Headway (MAH), s								4.0		4.0		4.0		0.0		4.0		0.0			
Queue Clearance Time (g _s), s								7.6		18.9		3.0				11.9					
Green Extension Time (g _e), s								0.2		0.6		0.0		0.0		0.3		0.0			
Phase Call Probability								1.00		1.00		1.00				0.99					
Max Out Probability								0.00		0.00		0.00		0.00		0.00					
Movement Group Results						EB			WB			NB			SB						
Approach Movement						L	T	R	L	T	R	L	T	R	L	T	R				
Assigned Movement						7	4	14	3	8	18	5	2	12	1	6	16				
Adjusted Flow Rate (v), veh/h						66	27	17	208	19		11	761	744	121	699	166				
Adjusted Saturation Flow Rate (s), veh/h/in						1711	1900	1359	1810	1880		1527	1856	1795	1810	1724	1585				
Queue Service Time (g _s), s						5.6	2.0	1.7	16.9	1.3		1.0	38.7	38.8	9.9	15.2	5.8				
Cycle Queue Clearance Time (g _c), s						5.6	2.0	1.7	16.9	1.3		1.0	38.7	38.8	9.9	15.2	5.8				
Green Ratio (g/C)						0.07	0.07	0.11	0.13	0.13		0.04	0.56	0.56	0.08	0.60	0.67				
Capacity (c), veh/h						114	127	145	236	245		61	1037	1004	147	2070	1057				
Volume-to-Capacity Ratio (X)						0.575	0.211	0.115	0.880	0.077		0.172	0.734	0.741	0.827	0.338	0.157				
Back of Queue (Q), ft/in (95 th percentile)						120.4	44	30.1	325	28.3		21.5	464	431.7	210.4	249.9	89.2				
Back of Queue (Q), veh/in (95 th percentile)						4.6	1.8	1.0	13.0	1.1		0.7	18.1	17.3	8.4	9.5	3.5				
Queue Storage Ratio (RQ) (95 th percentile)						0.30	0.00	0.50	0.00	0.00		0.09	0.00	0.00	2.10	0.00	0.30				
Uniform Delay (d ₁), s/veh						67.9	66.3	60.6	64.1	57.3		72.4	16.3	15.8	67.9	15.0	9.3				
Incremental Delay (d ₂), s/veh						4.5	0.8	0.3	10.2	0.1		0.8	2.7	2.9	10.4	0.4	0.3				
Initial Queue Delay (d ₃), s/veh						0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (d ₄), s/veh						72.4	67.1	60.9	74.2	57.4		73.2	19.0	18.6	78.3	15.4	9.6				
Level of Service (LOS)						E	E	E	E	E		E	B	B	E	B	A				
Approach Delay, s/veh / LOS						69.4		E	72.8		E	19.2		B	22.2		C				
Intersection Delay, s/veh / LOS									26.4												
Multimodal Results						EB			WB			NB			SB						
Pedestrian LOS Score / LOS						2.32		B	2.49		B	1.90		B	2.09		B				
Bicycle LOS Score / LOS						0.67		A	0.86		A	1.81		B	1.32		A				

Preston Highway at Interchange Drive
Traffic Impact Study

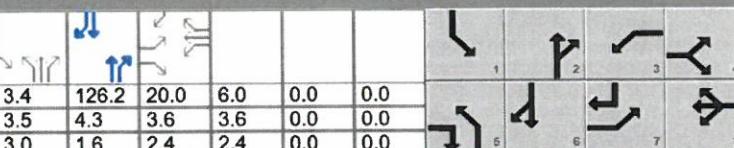
HCS7 Signalized Intersection Results Summary																		
General Information						Intersection Information												
Agency	Diane B. Zimmerman Traffic Engineering			Duration, h			0.250											
Analyst	DBZ	Analysis Date		Jun 2, 2021		Area Type												
Jurisdiction	Time Period		AM Peak		PHF		0.90											
Urban Street	Preston Highway	Analysis Year		2032 No Build		Analysis Period			1> 7:15									
Intersection	Interchange Drive	File Name		AM 32 NB Preston.xus														
Project Description	Stern																	
Demand Information			EB		WB		NB		SB									
Approach Movement			L	T	R	L	T	R	L	T	R							
Demand (v), veh/h			70		16	1	0	1	11	1390	3							
									1	762	161							
Signal Information																		
Cycle, s	150.0	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	No	Simult. Gap E/W	On															
Force Mode	Fixed	Simult. Gap N/S	On															
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT								
Assigned Phase				4			8	5	2	1	6							
Case Number					9.0		12.0	2.0	4.0	2.0	3.0							
Phase Duration, s						12.0		6.5	12.5	124.7	6.8 119.0							
Change Period, (Y+R_c), s						6.0		6.0	6.5	5.9	6.5 5.9							
Max Allow Headway (MAH), s						4.0		3.1	4.0	0.0	3.0 0.0							
Queue Clearance Time (g_s), s							2.2	3.1		2.1								
Green Extension Time (g_e), s						0.0		0.0	0.0	0.0	0.0							
Phase Call Probability								0.09	1.00		0.04							
Max Out Probability								0.00	0.00		0.00							
Movement Group Results			EB		WB		NB		SB									
Approach Movement			L	T	R	L	T	R	L	T	R							
Assigned Movement			7		14	3	8	18	5	2	12							
Adjusted Flow Rate (v), veh/h			78		18		2		12	731	731							
Adjusted Saturation Flow Rate (s), veh/h/in			1661		1359		1704		1527	1856	1854							
Queue Service Time (g_s), s			3.5		1.8		0.2		1.1	13.6	13.6							
Cycle Queue Clearance Time (g_c), s			3.5		1.8		0.2		1.1	13.6	13.6							
Green Ratio (g/C)			0.04		0.08		0.00		0.60	0.79	0.79							
Capacity (c), veh/h			133		109		6		61	1470	1468							
Volume-to-Capacity Ratio (X)			0.585		0.163		0.368		0.189	0.498	0.498							
Back of Queue (Q), ft/in (95 th percentile)			72		33.6		4.8		23.5	144	140.6							
Back of Queue (Q), veh/in (95 th percentile)			2.7		1.2		0.2		0.8	5.6	5.6							
Queue Storage Ratio (RQ) (95 th percentile)			0.18		0.56		0.00		0.09	0.00	0.00							
Uniform Delay (d_1), s/veh			70.8		64.3		74.6		71.8	2.9	2.9							
Incremental Delay (d_2), s/veh			4.0		0.7		13.3		0.9	0.7	0.7							
Initial Queue Delay (d_3), s/veh			0.0		0.0		0.0		0.0	0.0	0.0							
Control Delay (d), s/veh			74.8		65.0		87.9		72.7	3.7	3.7							
Level of Service (LOS)			E		E		F		E	A	A							
Approach Delay, s/veh / LOS			73.0	E		87.9	F		4.2	A	5.9							
Intersection Delay, s/veh / LOS						7.5				A								
Multimodal Results			EB		WB		NB		SB									
Pedestrian LOS Score / LOS			2.33	B		2.49	B		1.62	B	2.05							
Bicycle LOS Score / LOS				F		0.49	A		1.77	B	1.33							

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary													
General Information						Intersection Information							
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250						
Analyst	DBZ			Analysis Date	Jun 2, 2021	Area Type	Other						
Jurisdiction				Time Period	AM Peak	PHF	0.90						
Urban Street	Preston Highway			Analysis Year	2032 Build	Analysis Period	1 > 7:15						
Intersection	Interchange Drive			File Name	AM 32 B Preston.xus								
Project Description	Stern												
Demand Information				EB		WB		NB		SB			
Approach Movement		L	T	R		L	T	R	L	T	R		
Demand (v), veh/h		62	24	16	186	16	1	11	1366	132	112	683	161
Signal Information													
Cycle, s	150.0	Reference Phase	2										
Offset, s	0	Reference Point	End										
Uncoordinated	No	Simult. Gap E/W	On	Green	6.0	6.1	84.0	10.0	19.5	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	4.3	3.6	3.6	0.0			
				Red	3.0	0.0	1.6	2.4	2.4	0.0			
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT		
Assigned Phase					4			5	2	1	6		
Case Number					9.0		10.0	2.0	4.0	2.0	3.0		
Phase Duration, s					16.0		25.5	12.5	89.9	18.6	96.0		
Change Period, ($Y+R_c$), s					6.0		6.0	6.5	5.9	6.5	5.9		
Max Allow Headway (MAH), s					4.0		4.0	4.0	0.0	4.0	0.0		
Queue Clearance Time (g_s), s					7.9		18.8	3.1		11.9			
Green Extension Time (g_e), s					0.3		0.6	0.0	0.0	0.3	0.0		
Phase Call Probability					1.00		1.00	1.00		0.99			
Max Out Probability					0.00		0.00	0.00		0.00			
Movement Group Results				EB		WB		NB		SB			
Approach Movement		L	T	R		L	T	R	L	T	R		
Assigned Movement		7	4	14		3	8	18	5	2	12		
Adjusted Flow Rate (v), veh/h		69	27	18	207	19		11	788	773	121	738	174
Adjusted Saturation Flow Rate (s), veh/h/ln		1711	1900	1359	1810	1880		1527	1856	1798	1810	1724	1585
Queue Service Time (g_s), s		5.9	2.0	1.8	16.8	1.3		1.1	43.3	43.8	9.9	16.3	6.2
Cycle Queue Clearance Time (g_c), s		5.9	2.0	1.8	16.8	1.3		1.1	43.3	43.8	9.9	16.3	6.2
Green Ratio (g/C)		0.07	0.07	0.11	0.13	0.13		0.04	0.56	0.56	0.08	0.60	0.67
Capacity (c), veh/h		114	127	145	235	244		61	1039	1007	146	2072	1058
Volume-to-Capacity Ratio (X)		0.604	0.211	0.123	0.880	0.077		0.188	0.759	0.768	0.827	0.356	0.165
Back of Queue (Q), ft/ln (95 th percentile)		127.4	44	32.2	323.8	28.3		23.4	529	502.4	209.6	264.2	93.9
Back of Queue (Q), veh/ln (95 th percentile)		4.8	1.8	1.1	13.0	1.1		0.8	20.7	20.1	8.4	10.1	3.7
Queue Storage Ratio (RQ) (95 th percentile)		0.32	0.00	0.54	0.00	0.00		0.09	0.00	0.00	2.10	0.00	0.31
Uniform Delay (d_1), s/veh		68.1	66.3	60.6	64.1	57.4		72.5	18.5	18.1	67.9	15.2	9.3
Incremental Delay (d_2), s/veh		5.1	0.8	0.4	10.2	0.1		0.7	2.7	2.9	10.3	0.4	0.3
Initial Queue Delay (d_3), s/veh		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh		73.1	67.1	61.0	74.3	57.5		73.2	21.2	21.0	78.2	15.7	9.6
Level of Service (LOS)		E	E	E	E	E		E	C	C	E	B	A
Approach Delay, s/veh / LOS		69.8		E	72.9		E	21.5		C	22.0		C
Intersection Delay, s/veh / LOS					27.4					C			
Multimodal Results				EB		WB		NB		SB			
Pedestrian LOS Score / LOS		2.32		B	2.49		B	1.90		B	2.09		B
Bicycle LOS Score / LOS		0.67		A	0.86		A	1.87		B	1.36		A

Preston Highway at Interchange Drive

Traffic Impact Study

HCS7 Signalized Intersection Results Summary														
General Information						Intersection Information								
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h		0.250						
Analyst	DBZ		Analysis Date		Jun 2, 2021		Area Type		Other					
Jurisdiction			Time Period		PM Peak		PHF		0.98					
Urban Street	Preston Highway		Analysis Year		2021		Analysis Period		1> 4:45					
Intersection	Interchange Dr		File Name		PM 21 Preston.xus									
Project Description	Stern													
Demand Information			EB		WB		NB		SB					
Approach Movement			L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	184		48		3	0	4	16	1222	2	0	1867	155	
Signal Information														
Cycle, s	180.0	Reference Phase	2											
Offset, s	0	Reference Point	End	Green	3.4	126.2	20.0	6.0	0.0	0.0				
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	4.3	3.6	3.6	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	1.6	2.4	2.4	0.0	0.0				
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase			4		8		5	2	1	6				
Case Number			9.0		12.0		2.0	4.0	2.0	3.0				
Phase Duration, s			26.0		12.0		9.9	142.0	0.0	132.1				
Change Period, (Y+R_c), s			6.0		6.0		6.5	5.9	6.5	5.9				
Max Allow Headway (MAH), s			4.0		3.1		4.0	0.0	0.0	0.0				
Queue Clearance Time (g_s), s					2.7		3.6							
Green Extension Time (g_e), s			0.0		0.0		0.0	0.0	0.0	0.0				
Phase Call Probability					1.00		0.56							
Max Out Probability					0.00		0.00							
Movement Group Results			EB		WB		NB		SB					
Approach Movement			L	T	R	L	T	R	L	T	R			
Assigned Movement	7		14		3	8	18	5	2	12	1	6	16	
Adjusted Flow Rate (v), veh/h	188		49		7		16	630	629	0	1846	153		
Adjusted Saturation Flow Rate (s), veh/h/in	1757		1610		1690		1810	1885	1884	1810	1781	1598		
Queue Service Time (g_s), s	9.0		4.9		0.7		1.6	19.9	19.9	0.0	61.1	3.8		
Cycle Queue Clearance Time (g_c), s	9.0		4.9		0.7		1.6	19.9	19.9	0.0	61.1	3.8		
Green Ratio (g/C)	0.11		0.13		0.03		0.65	0.76	0.76			0.70	0.81	
Capacity (c), veh/h	390		209		56		34	1425	1425	1	2497	1298		
Volume-to-Capacity Ratio (X)	0.481		0.234		0.127		0.486	0.442	0.442	0.000	0.739	0.118		
Back of Queue (Q), ft/in (95 th percentile)	183.7		91.7		14.6		37.6	268.5	266.1	0	788.8	48.1		
Back of Queue (Q), veh/in (95 th percentile)	7.3		3.7		0.6		1.5	10.7	10.6	0.0	31.1	1.9		
Queue Storage Ratio (RQ) (95 th percentile)	0.46		1.53		0.00		0.15	0.00	0.00	0.00	0.00	0.16		
Uniform Delay (d_1), s/veh	75.1		70.3		84.5		88.8	6.9	6.9	0.0	19.0	3.7		
Incremental Delay (d_2), s/veh	0.9		0.6		0.4		7.5	0.7	0.7	0.0	1.5	0.1		
Initial Queue Delay (d_3), s/veh	0.0		0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay (d), s/veh	76.0		70.9		84.8		96.3	7.6	7.6	0.0	20.5	3.8		
Level of Service (LOS)	E		E		F		F	A	A			C	A	
Approach Delay, s/veh / LOS	75.0		E		84.8		F	8.8	A	19.2			B	
Intersection Delay, s/veh / LOS	19.3						B							
Multimodal Results			EB		WB		NB		SB					
Pedestrian LOS Score / LOS	2.33		B		2.49		1.64	B	2.07	B				
Bicycle LOS Score / LOS			F		0.50		A	1.53	B	2.19	B			

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250				
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type	Other				
Jurisdiction			Time Period	PM Peak		PHF	0.98				
Urban Street	Preston Highway		Analysis Year	2022 No Build		Analysis Period	1> 4:45				
Intersection	Interchange Dr		File Name	PM 22 NB Preston.xus							
Project Description	Stern										
Demand Information				EB		WB		NB		SB	
Approach Movement		L	T	R		L	T	R	L	T	R
Demand (v), veh/h		185		48		3	0	4	16	1228	2
Signal Information				EB		WB		NB		SB	
Cycle, s	180.0	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	No	Simult. Gap E/W	On	Green	3.4	126.2	20.0	6.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	4.3	3.6	3.6	0.0	0.0	
				Red	3.0	1.6	2.4	2.4	0.0	0.0	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase					4			5	2	1	6
Case Number						9.0		12.0	2.0	4.0	2.0
Phase Duration, s							26.0		12.0	9.9	142.0
Change Period, ($Y+R_c$), s							6.0		6.0	6.5	5.9
Max Allow Headway (MAH), s							4.0		3.3	4.0	0.0
Queue Clearance Time (g_s), s									2.7	3.6	
Green Extension Time (g_e), s							0.0		0.0	0.0	
Phase Call Probability										1.00	0.56
Max Out Probability										0.00	0.00
Movement Group Results				EB		WB		NB		SB	
Approach Movement		L	T	R		L	T	R	L	T	R
Assigned Movement		7		14		3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h		189		49			7		16	633	632
Adjusted Saturation Flow Rate (s), veh/h/in		1757		1610			1690		1810	1885	1884
Queue Service Time (g_s), s		9.1		4.9			0.7		1.6	19.9	19.9
Cycle Queue Clearance Time (g_c), s		9.1		4.9			0.7		1.6	19.9	19.9
Green Ratio (g/C)		0.11		0.13			0.03		0.65	0.76	0.76
Capacity (c), veh/h		390		209			56		34	1425	1425
Volume-to-Capacity Ratio (X)		0.483		0.234			0.127		0.486	0.444	0.444
Back of Queue (Q), ft/in (95 th percentile)		184.7		91.7			14.9		37.5	266.6	264.3
Back of Queue (Q), veh/in (95 th percentile)		7.4		3.7			0.6		1.5	10.6	10.6
Queue Storage Ratio (RQ) (95 th percentile)		0.46		1.53			0.00		0.15	0.00	0.00
Uniform Delay (d_1), s/veh		75.1		70.3			84.5		88.8	6.9	6.9
Incremental Delay (d_2), s/veh		0.9		0.6			0.4		7.5	0.7	0.7
Initial Queue Delay (d_3), s/veh		0.0		0.0			0.0		0.0	0.0	0.0
Control Delay (d), s/veh		76.1		70.9			84.8		96.3	7.6	7.6
Level of Service (LOS)		E		E			F		F	A	A
Approach Delay, s/veh / LOS		75.0		E			84.8		8.7	A	
Intersection Delay, s/veh / LOS							19.4			B	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS		2.33		B			2.49		B		2.07
Bicycle LOS Score / LOS				F			0.50		A		2.20

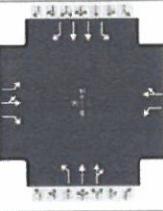
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency			Diane B. Zimmerman Traffic Engineering			Duration, h			0.250		
Analyst			DBZ			Analysis Date			Jun 2, 2021		
Jurisdiction			Time Period			PM Peak			PHF		
Urban Street			Preston Highway			Analysis Year			2022 Build		
Intersection			Interchange Dr			File Name			PM 22 B Preston.xus		
Project Description						Stern					
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Demand (v), veh/h				174	25	48	185	14	4	16	1233
										91	
										121	1783
										156	
Signal Information											
Cycle, s	180.0	Reference Phase	2								
Offset, s	0	Reference Point	End	Green	3.4	4.1	96.6	24.0	21.0	0.0	
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	3.5	4.3	3.6	3.6	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.6	2.4	2.4	0.0	
Timer Results				EBL	EBT		WBL	WBT	NBL	NBT	SBL
Assigned Phase						4			5	2	1
Case Number								10.0	2.0	4.0	2.0
Phase Duration, s								27.0	9.9	102.5	20.5
Change Period, (Y+R c), s									113.1		
Max Allow Headway (MAH), s										6.5	5.9
Queue Clearance Time (g s), s										5.9	6.5
Green Extension Time (g e), s										0.0	0.0
Phase Call Probability										1.00	
Max Out Probability										0.02	0.00
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Assigned Movement				7	4	14	3	8	18	5	2
Adjusted Flow Rate (v), veh/h				178	26	49	189	18		16	689
Adjusted Saturation Flow Rate (s), veh/h/in				1810	1900	1610	1810	1827		1810	1885
Queue Service Time (g s), s				17.0	2.1	4.8	18.5	1.6		1.6	44.9
Cycle Queue Clearance Time (g c), s				17.0	2.1	4.8	18.5	1.6		1.6	43.9
Green Ratio (g/C)				0.13	0.13	0.15	0.12	0.12		0.02	0.54
Capacity (c), veh/h				241	253	245	211	213		34	1012
Volume-to-Capacity Ratio (X)				0.736	0.101	0.200	0.894	0.086		0.487	0.681
Back of Queue (Q), ft/in (95 th percentile)				325.3	46.5	88.9	351.7	33.9		37.1	634.7
Back of Queue (Q), veh/in (95 th percentile)				13.0	1.9	3.6	14.1	1.4		1.5	25.2
Queue Storage Ratio (RQ) (95 th percentile)				0.81	0.00	1.48	0.00	0.00		0.15	0.00
Uniform Delay (d 1), s/veh				75.0	68.5	66.7	78.4	70.9		88.2	26.1
Incremental Delay (d 2), s/veh				7.6	0.2	0.4	10.4	0.1		6.9	2.4
Initial Queue Delay (d 3), s/veh				0.0	0.0	0.0	0.0	0.0		0.0	0.0
Control Delay (d), s/veh				82.5	68.7	67.1	88.8	71.0		95.0	28.6
Level of Service (LOS)				F	E	E	F	E		F	C
Approach Delay, s/veh / LOS				78.1		E	87.3			28.6	C
Intersection Delay, s/veh / LOS							37.0			D	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				2.33		B	2.49			1.92	B
Bicycle LOS Score / LOS				0.90		A	0.83			1.62	B

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary												
General Information						Intersection Information						
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250					
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type	Other					
Jurisdiction			Time Period	PM Peak		PHF	0.98					
Urban Street	Preston Highway		Analysis Year	2032 No Build		Analysis Period	1 > 4:45					
Intersection	Interchange Dr		File Name	PM 32 NB Preston.xus								
Project Description	Stern											
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Demand (v), veh/h				194		50	3	0	4	17	1291	2
Signal Information												
Cycle, s	180.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase					4			5	2	1	6	
Case Number					9.0		12.0	2.0	4.0	2.0	3.0	
Phase Duration, s					26.0		10.0	10.0	144.0	0.0	134.0	
Change Period, (Y+R_c), s					6.0		4.0	6.5	5.9	3.5	5.9	
Max Allow Headway (MAH), s					4.0		3.3	4.0	0.0	0.0	0.0	
Queue Clearance Time (g_s), s							2.7	3.7				
Green Extension Time (g_e), s					0.0		0.0	0.0	0.0	0.0	0.0	
Phase Call Probability							1.00	0.58				
Max Out Probability							0.00	0.00				
Movement Group Results												
Approach Movement				EB	WB	NB		SB				
Assigned Movement				L	T	R	L	T	R	L	T	R
				7		14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h				198		51		7		17	665	665
Adjusted Saturation Flow Rate (s), veh/h/in				1757		1610		1690		1810	1885	1884
Queue Service Time (g_s), s				9.6		5.1		0.7		1.7	19.7	19.7
Cycle Queue Clearance Time (g_c), s				9.6		5.1		0.7		1.7	19.7	19.7
Green Ratio (g/C)				0.11		0.13		0.03		0.65	0.77	0.77
Capacity (c), veh/h				390		210		56		35	1446	1446
Volume-to-Capacity Ratio (X)				0.507		0.243		0.127		0.497	0.460	0.460
Back of Queue (Q), ft/in (95 th percentile)				193.2		95.6		14.9		39.6	250.6	248.4
Back of Queue (Q), veh/in (95 th percentile)				7.7		3.8		0.6		1.6	9.9	9.9
Queue Storage Ratio (RQ) (95 th percentile)				0.48		1.59		0.00		0.16	0.00	0.00
Uniform Delay (d_1), s/veh				75.4		70.3		84.5		88.6	6.0	6.0
Incremental Delay (d_2), s/veh				1.0		0.6		0.4		7.0	0.7	0.7
Initial Queue Delay (d_3), s/veh				0.0		0.0		0.0		0.0	0.0	0.0
Control Delay (d), s/veh				76.4		70.9		84.8		95.7	6.7	6.7
Level of Service (LOS)				E		E		F		F	A	A
Approach Delay, s/veh / LOS				75.2		E		84.8		7.9	A	
Intersection Delay, s/veh / LOS								18.9			B	
Multimodal Results												
Pedestrian LOS Score / LOS				2.34		B		2.49		B		2.07
Bicycle LOS Score / LOS						F		0.50		A		2.29

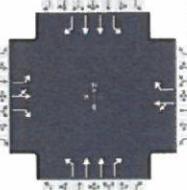
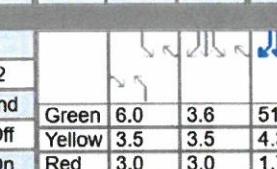
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																
General Information						Intersection Information										
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250									
Analyst	DBZ		Analysis Date	Jun 2, 2021			Area Type	Other								
Jurisdiction			Time Period	PM Peak			PHF	0.98								
Urban Street	Preston Highway		Analysis Year	2032 Build			Analysis Period	1 > 4:45								
Intersection	Interchange Dr		File Name	PM 32 B Preston.xus												
Project Description	Stern															
Demand Information			EB			WB			NB							
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R				
Demand (v), veh/h	183	25	50	168	14	4	17	1296	93	144	1877	164				
Signal Information																
Cycle, s	180.0	Reference Phase	2													
Offset, s	0	Reference Point	End	Green	3.5	6.2	92.5	25.0	22.0	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	3.5	4.3	3.6	3.6	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.6	2.4	2.4	0.0						
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT						
Assigned Phase					4		8		5		2					
Case Number					9.0		10.0		2.0		4.0					
Phase Duration, s					31.0		28.0		10.0		98.4					
Change Period, (Y+R_c), s					6.0		6.0		6.5		5.9					
Max Allow Headway (MAH), s					4.0		3.0		4.0		0.0					
Queue Clearance Time (g_s), s					19.8		18.5		3.7		16.0					
Green Extension Time (g_e), s					0.6		0.2		0.0		0.2					
Phase Call Probability					1.00		1.00		0.58		1.00					
Max Out Probability					0.04		0.00		0.00		0.00					
Movement Group Results			EB			WB			NB			SB				
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R				
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16				
Adjusted Flow Rate (v), veh/h	187	26	51	171	18			17	718	704	141	1836	160			
Adjusted Saturation Flow Rate (s), veh/h/in	1810	1900	1610	1810	1827			1810	1885	1840	1810	1781	1598			
Queue Service Time (g_s), s	17.8	2.1	5.0	16.5	1.6			1.7	51.0	50.2	14.0	78.5	4.6			
Cycle Queue Clearance Time (g_c), s	17.8	2.1	5.0	16.5	1.6			1.7	51.0	50.2	14.0	78.5	4.6			
Green Ratio (g/C)	0.14	0.14	0.16	0.12	0.12			0.02	0.51	0.51	0.09	0.58	0.72			
Capacity (c), veh/h	251	264	255	221	223			35	968	945	162	2080	1155			
Volume-to-Capacity Ratio (X)	0.743	0.097	0.200	0.775	0.082			0.496	0.741	0.745	0.868	0.883	0.139			
Back of Queue (Q), ft/in (95 th percentile)	340.3	46.2	92	308.7	33.6			38.9	714	670.6	269.4	1011.6	64.8			
Back of Queue (Q), veh/in (95 th percentile)	13.6	1.8	3.7	12.3	1.3			1.6	28.3	26.8	10.8	39.8	2.6			
Queue Storage Ratio (RQ) (95 th percentile)	0.85	0.00	1.53	0.00	0.00			0.16	0.00	0.00	2.69	0.00	0.22			
Uniform Delay (d_1), s/veh	74.4	67.6	65.9	76.6	70.0			88.1	29.8	28.0	88.2	30.0	6.1			
Incremental Delay (d_2), s/veh	8.4	0.2	0.4	3.0	0.1			6.4	3.1	3.3	3.7	4.1	0.2			
Initial Queue Delay (d_3), s/veh	0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	82.8	67.8	66.2	79.6	70.1			94.5	32.9	31.3	91.9	34.1	6.3			
Level of Service (LOS)	F	E	E	E	E			F	C	C	F	C	A			
Approach Delay, s/veh / LOS	78.2	E		78.7	E		32.8		C	35.8		D				
Intersection Delay, s/veh / LOS	39.6					D										
Multimodal Results			EB			WB			NB			SB				
Pedestrian LOS Score / LOS	2.33	B			2.49	B			1.92	B						
Bicycle LOS Score / LOS	0.92	A			0.80	A			1.67	B						

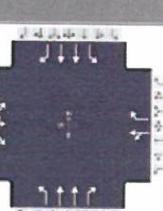
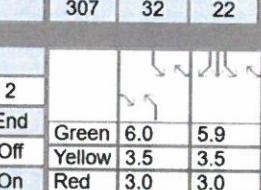
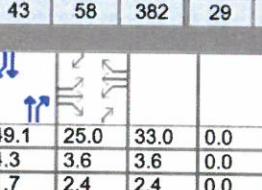
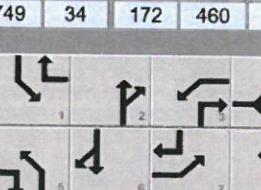
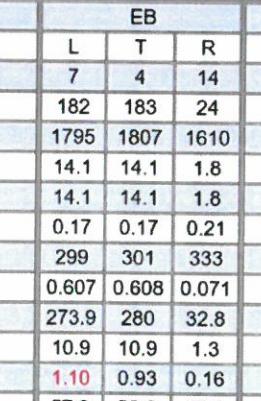
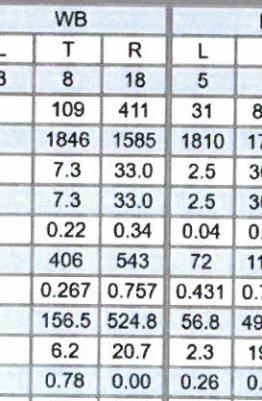
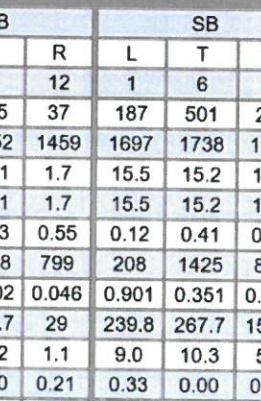
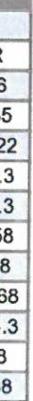
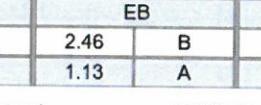
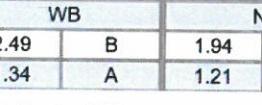
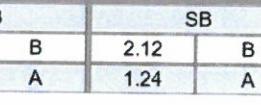
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250				
Analyst	DBZ	Analysis Date	Jun 2, 2021			Area Type	Other				
Jurisdiction		Time Period	AM Peak			PHF	0.93				
Urban Street	Preston Highway	Analysis Year	2021			Analysis Period	1> 7:15				
Intersection	Mt Washington Rd	File Name	AM 21 Preston.xus								
Project Description	Stern										
Demand Information				EB			WB			NB	
Approach Movement	L	T	R	L	T	R	L	T	R	L	T
Demand (v), veh/h	290	32	22	43	58	349	29	685	34	141	395
Signal Information				EB			WB			NB	
Cycle, s	150.0	Reference Phase	2	L	T	R	L	T	R	L	T
Offset, s	0	Reference Point	End	Green	6.0	3.1	52.0	25.0	33.0	0.0	
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	3.5	3.5	4.3	3.6	3.6	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.7	2.4	2.4	0.0	
Timer Results				EBL		EBT		WBL		WBT	
Assigned Phase				4				8		5	
Case Number										2	
Phase Duration, s								11.0		2.0	
Change Period, (Y+R_c), s										3.0	
Max Allow Headway (MAH), s								39.0		12.5	
Queue Clearance Time (g_s), s										58.0	
Green Extension Time (g_e), s										22.1	
Phase Call Probability										67.5	
Max Out Probability											6.0
Movement Group Results				EB			WB			NB	
Approach Movement				L	T	R	L	T	R	L	T
Assigned Movement				7	4	14	3	8	18	5	2
Adjusted Flow Rate (v), veh/h				172	175	24		109	375	31	737
Adjusted Saturation Flow Rate (s), veh/h/in				1795	1807	1610		1846	1585	1810	1752
Queue Service Time (g_s), s				13.2	13.4	1.8		7.3	31.5	2.5	26.1
Cycle Queue Clearance Time (g_c), s				13.2	13.4	1.8		7.3	31.5	2.5	26.1
Green Ratio (g/C)				0.17	0.17	0.21		0.22	0.32	0.04	0.35
Capacity (c), veh/h				299	301	333		406	513	72	1214
Volume-to-Capacity Ratio (X)				0.573	0.580	0.071		0.267	0.732	0.431	0.607
Back of Queue (Q), ft/in (95 th percentile)				259.1	267.4	32.8		154.6	479.8	55.3	434
Back of Queue (Q), veh/in (95 th percentile)				10.3	10.4	1.3		6.1	18.9	2.2	16.8
Queue Storage Ratio (RQ) (95 th percentile)				1.04	0.89	0.16		0.77	0.00	0.26	0.00
Uniform Delay (d_1), s/veh				57.6	57.7	47.9		48.5	45.0	70.3	40.6
Incremental Delay (d_2), s/veh				2.5	2.5	0.1		0.1	4.7	4.0	2.3
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0		0.0	0.0	0.0	0.0
Control Delay (d), s/veh				60.0	60.2	48.0		48.6	49.6	74.3	42.8
Level of Service (LOS)				E	E	D		D	D	E	D
Approach Delay, s/veh / LOS				59.3	E			49.4	D	42.8	D
Intersection Delay, s/veh / LOS								43.9			D
Multimodal Results				EB			WB			NB	
Pedestrian LOS Score / LOS				2.46	B		2.49	B		1.94	B
Bicycle LOS Score / LOS				1.10	A		1.29	A		1.15	A

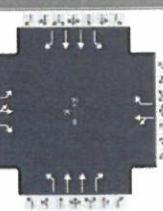
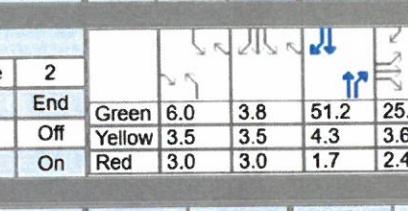
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency			Diane B. Zimmerman Traffic Engineering			Duration, h		0.250			
Analyst			DBZ			Analysis Date		Jun 2, 2021		Area Type	
Jurisdiction			Time Period			AM Peak		PHF		0.93	
Urban Street			Preston Highway			Analysis Year		2022 No Build		Analysis Period	
Intersection			Mt Washington Rd			File Name		AM 22 NB Preston.xus			
Project Description Stern											
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Demand (v), veh/h				291	32	22	43	58	351	29	688
				34			142	397	200	34	
Signal Information											
Cycle, s	150.0	Reference Phase	2								
Offset, s	0	Reference Point	End	Green	6.0	3.6	51.4	25.0	33.0	0.0	
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	3.5	3.5	4.3	3.6	3.6	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.7	2.4	2.4	0.0	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase					4			8	5	2	1
Case Number						9.0		11.0	2.0	3.0	2.0
Phase Duration, s							31.0		39.0	12.5	57.4
Change Period, (Y+R c), s							6.0		6.0	6.5	6.0
Max Allow Headway (MAH), s							5.1		5.3	5.0	0.0
Queue Clearance Time (g s), s								33.5	4.5		15.4
Green Extension Time (g e), s							0.0		0.0	0.0	0.7
Phase Call Probability								1.00	1.00		1.00
Max Out Probability								1.00	0.01		0.00
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	T
Assigned Movement				7	4	14	3	8	18	5	2
Adjusted Flow Rate (v), veh/h				172	175	24	109	377	31	740	37
Adjusted Saturation Flow Rate (s), veh/h/in				1795	1807	1610	1846	1585	1810	1752	1459
Queue Service Time (g s), s				13.3	13.4	1.8	7.3	31.5	2.5	26.4	1.7
Cycle Queue Clearance Time (g c), s				13.3	13.4	1.8	7.3	31.5	2.5	26.4	1.7
Green Ratio (g/C)				0.17	0.17	0.21	0.22	0.33	0.04	0.34	0.56
Capacity (c), veh/h				299	301	333	406	519	72	1202	821
Volume-to-Capacity Ratio (X)				0.575	0.582	0.071	0.267	0.728	0.431	0.616	0.045
Back of Queue (Q), ft/in (95 th percentile)				259.8	268.2	32.8	156.5	484.8	56.8	438.9	27.8
Back of Queue (Q), veh/in (95 th percentile)				10.3	10.5	1.3	6.2	19.1	2.3	17.0	1.0
Queue Storage Ratio (RQ) (95 th percentile)				1.04	0.89	0.16	0.78	0.00	0.26	0.00	0.20
Uniform Delay (d 1), s/veh				57.6	57.7	47.9	48.5	44.6	70.3	41.1	14.7
Incremental Delay (d 2), s/veh				2.5	2.5	0.1	0.5	5.5	5.7	2.4	0.1
Initial Queue Delay (d 3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				60.1	60.2	48.0	49.0	50.1	76.0	43.4	14.8
Level of Service (LOS)				E	E	D	D	D	E	D	B
Approach Delay, s/veh / LOS				59.4	E		49.9	D	43.4	D	36.3
Intersection Delay, s/veh / LOS							44.8			D	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				2.46	B	2.49	B	1.94	B	2.12	B
Bicycle LOS Score / LOS				1.10	A	1.29	A	1.15	A	1.14	A

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary												
General Information								Intersection Information				
Agency								Duration, h	0.250			
Analyst								Analysis Date	Jun 2, 2021	Area Type		
Jurisdiction								Time Period	AM Peak	PHF		
Urban Street								Analysis Year	2022 Build	Analysis Period		
Intersection								File Name	AM 22 B Preston.xus			
Project Description												
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Demand (v), veh/h				307	32	22	43	58	382	29	749	34
Signal Information												
Cycle, s	150.0	Reference Phase	2	Green	6.0	5.9	49.1	25.0	33.0	0.0		
Offset, s	0	Reference Point	End	Yellow	3.5	3.5	4.3	3.6	3.6	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Red	3.0	3.0	1.7	2.4	2.4	0.0		
Force Mode	Fixed	Simult. Gap N/S	On									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase					4			5	2	1	6	
Case Number					9.0			11.0	2.0	3.0	2.0	3.0
Phase Duration, s					31.0			39.0	12.5	55.1	24.9	67.5
Change Period, ($Y+R_c$), s					6.0			6.0	6.5	6.0	6.5	6.0
Max Allow Headway (MAH), s					5.1			5.3	5.0	0.0	5.0	0.0
Queue Clearance Time (g_s), s								35.0	4.5		17.5	
Green Extension Time (g_e), s					0.0			0.0	0.0	0.0	0.9	0.0
Phase Call Probability								1.00	1.00		1.00	
Max Out Probability								1.00	0.01		0.00	
Movement Group Results												
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h				182	183	24	109	411	31	805	37	187
Adjusted Saturation Flow Rate (s), veh/h/in				1795	1807	1610	1846	1585	1810	1752	1459	1697
Queue Service Time (g_s), s				14.1	14.1	1.8	7.3	33.0	2.5	30.1	1.7	15.5
Cycle Queue Clearance Time (g_c), s				14.1	14.1	1.8	7.3	33.0	2.5	30.1	1.7	15.5
Green Ratio (g/C)				0.17	0.17	0.21	0.22	0.34	0.04	0.33	0.55	0.12
Capacity (c), veh/h				299	301	333	406	543	72	1148	799	208
Volume-to-Capacity Ratio (X)				0.607	0.608	0.071	0.267	0.757	0.431	0.702	0.046	0.901
Back of Queue (Q), ft/in (95 th percentile)				273.9	280	32.8	156.5	524.8	56.8	495.7	29	239.8
Back of Queue (Q), veh/in (95 th percentile)				10.9	10.9	1.3	6.2	20.7	2.3	19.2	1.1	9.0
Queue Storage Ratio (RQ) (95 th percentile)				1.10	0.93	0.16	0.78	0.00	0.26	0.00	0.21	0.33
Uniform Delay (d_1), s/veh				57.9	58.0	47.9	48.5	43.8	70.3	44.0	15.7	43.8
Incremental Delay (d_2), s/veh				3.1	3.1	0.1	0.5	6.5	5.7	3.6	0.1	15.9
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.7
Control Delay (d), s/veh				61.0	61.1	48.0	49.0	50.2	76.0	47.6	15.9	59.7
Level of Service (LOS)				E	E	D	D	D	E	D	B	E
Approach Delay, s/veh / LOS				60.3	E		50.0	D	47.3	D		33.0
Intersection Delay, s/veh / LOS							44.8			D		
Multimodal Results												
Pedestrian LOS Score / LOS				2.46	B		2.49	B	1.94	B		2.12
Bcycle LOS Score / LOS				1.13	A		1.34	A	1.21	A		1.24

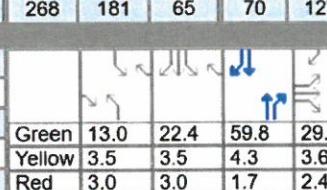
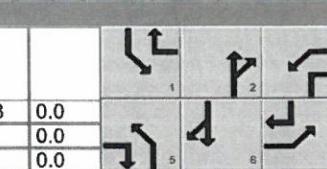
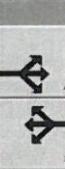
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																				
General Information						Intersection Information														
Agency	Diane B. Zimmerman Traffic Engineering			Duration, h			0.250													
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type														
Jurisdiction			Time Period	AM Peak		PHF														
Urban Street	Preston Highway		Analysis Year	2032 No Build		Analysis Period														
Intersection	Mt Washington Rd		File Name	AM 32 NB Preston.xus																
Project Description	Stern																			
Demand Information				EB		WB		NB		SB										
Approach Movement	L	T	R	L	T	R	L	T	R	L	T									
Demand (v), veh/h	306	34	23	45	61	369	30	723	36	149	417									
Signal Information																				
Cycle, s	150.0	Reference Phase	2																	
Offset, s	0	Reference Point	End	Green	6.0	3.8	51.2	25.0	33.0	0.0										
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	3.5	3.5	4.3	3.6	3.6	0.0										
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.7	2.4	2.4	0.0										
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT									
Assigned Phase					4			8	5	2	1									
Case Number						9.0			2.0	3.0	2.0									
Phase Duration, s							31.0		12.5	57.2	22.8									
Change Period, (Y+R_c), s								6.0	6.5	6.0	6.0									
Max Allow Headway (MAH), s								5.1	3.3	0.0	3.0									
Queue Clearance Time (g_s), s									4.6		16.1									
Green Extension Time (g_e), s								0.0	0.0	0.0	0.0									
Phase Call Probability									1.00	1.00										
Max Out Probability									0.00	0.00										
Movement Group Results				EB		WB		NB		SB										
Approach Movement	L	T	R	L	T	R	L	T	R	L	T									
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6									
Adjusted Flow Rate (v), veh/h	181	185	25		114	397	32	777	39	162	453									
Adjusted Saturation Flow Rate (s), veh/h/in	1795	1807	1610		1846	1585	1810	1752	1459	1697	1738									
Queue Service Time (g_s), s	14.0	14.2	1.9		7.7	33.0	2.6	28.2	1.8	14.1	13.5									
Cycle Queue Clearance Time (g_c), s	14.0	14.2	1.9		7.7	33.0	2.6	28.2	1.8	14.1	11.2									
Green Ratio (g/C)	0.17	0.17	0.21		0.22	0.33	0.04	0.34	0.56	0.11	0.41									
Capacity (c), veh/h	299	301	333		406	521	72	1196	819	185	1425									
Volume-to-Capacity Ratio (X)	0.605	0.613	0.074		0.281	0.761	0.446	0.650	0.047	0.877	0.318									
Back of Queue (Q), ft/in (95 th percentile)	272.9	282.5	34.3		162.7	511.3	57.3	485.1	29.6	272.3	247.1									
Back of Queue (Q), veh/in (95 th percentile)	10.8	11.0	1.4		6.5	20.1	2.3	18.0	1.1	10.2	9.5									
Queue Storage Ratio (RQ) (95 th percentile)	1.09	0.94	0.17		0.81	0.00	0.27	0.00	0.21	0.38	0.00									
Uniform Delay (d_1), s/veh	57.9	58.0	47.9		48.6	45.1	70.4	41.8	14.8	65.9	30.8									
Incremental Delay (d_2), s/veh	3.1	3.2	0.1		0.1	5.9	4.2	2.7	0.1	4.9	0.6									
Initial Queue Delay (d_3), s/veh	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0									
Control Delay (d), s/veh	61.0	61.2	48.1		48.8	50.9	74.6	44.6	14.9	70.9	31.4									
Level of Service (LOS)	E	E	D		D	D	E	D	B	E	C									
Approach Delay, s/veh / LOS	60.3	E			50.5	D		44.4	D	34.9	C									
Intersection Delay, s/veh / LOS					44.9				D											
Multimodal Results				EB		WB		NB		SB										
Pedestrian LOS Score / LOS	2.46	B		2.49	B		1.94	B		2.12	B									
Bicycle LOS Score / LOS	1.13	A		1.33	A		1.19	A		1.18	A									

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary											
General Information						Intersection Information					
Agency						Duration, h					
Analyst						Area Type					
Jurisdiction						Time Period					
Urban Street						PHF					
Intersection						Analysis Year					
Project Description						2032 Build					
Intersection Diagram						Analysis Period					
File Name						1 > 7:15					
AM 32 B Preston.xus											
Demand Information											
Approach Movement			EB			WB			NB		
Demand (v), veh/h			L	T	R	L	T	R	L	T	R
			322	34	23	45	61	400	30	784	36
Signal Information											
Cycle, s	150.0	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	No	Simult. Gap E/W	Off								
Force Mode	Fixed	Simult. Gap N/S	On								
Signal Phases						Phase Sequence					
Green	6.0	6.5	48.5	25.0	33.0	0.0	1	2	3	4	
Yellow	3.5	3.5	4.3	3.6	3.6	0.0	5	6	7	8	
Red	3.0	3.0	1.7	2.4	2.4	0.0					
Timer Results											
Assigned Phase			EBL			EBT			WBL		
			4			8			5		
Case Number											
			9.0								
Phase Duration, s											
			31.0								
Change Period, ($Y+R_c$), s											
			6.0								
Max Allow Headway (MAH), s											
			5.1								
Queue Clearance Time (g_s), s											
Green Extension Time (g_e), s											
			0.0								
Phase Call Probability											
Max Out Probability											
Movement Group Results											
Approach Movement			EB			WB			NB		
			L	T	R	L	T	R	L	T	R
			7	4	14	3	8	18	5	2	12
Assigned Movement											
			1	6	16						
Adjusted Flow Rate (v), veh/h											
			190	192	25						
Adjusted Saturation Flow Rate (s), veh/h/in											
			1795	1807	1610						
Queue Service Time (g_s), s											
			14.8	14.9	1.9						
Cycle Queue Clearance Time (g_c), s											
			14.8	14.9	1.9						
Green Ratio (g/C)											
			0.17	0.17	0.21						
Capacity (c), veh/h											
			299	301	333						
Volume-to-Capacity Ratio (X)											
			0.636	0.639	0.074						
Back of Queue (Q), ft/in (95 th percentile)											
			287.5	294.6	34.3						
Back of Queue (Q), veh/in (95 th percentile)											
			11.4	11.5	1.4						
Queue Storage Ratio (RQ) (95 th percentile)											
			1.15	0.98	0.17						
Uniform Delay (d_1), s/veh											
			58.3	58.3	47.9						
Incremental Delay (d_2), s/veh											
			3.9	3.9	0.1						
Initial Queue Delay (d_3), s/veh											
			0.0	0.0	0.0						
Control Delay (d), s/veh											
			62.1	62.2	48.1						
Level of Service (LOS)			E	E	D	D	D	B	E	C	B
Approach Delay, s/veh / LOS			61.3	E		51.0	D		49.2	D	32.4
Intersection Delay, s/veh / LOS						45.6			D		
Multimodal Results											
Pedestrian LOS Score / LOS			2.46	B		2.49	B		1.94	B	2.12
Bicycle LOS Score / LOS			1.16	A		1.39	A		1.24	A	1.27
Intersection Diagram											

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary												
General Information								Intersection Information				
Agency								Duration, h				
Analyst								Jun 2, 2021				
Jurisdiction								Area Type				
Urban Street								PM Peak				
Intersection								PHF				
Urban Street								0.97				
Intersection								Analysis Year				
Intersection								2021				
Intersection								Analysis Period				
Intersection								1 > 4:45				
Project Description								File Name				
Project Description								PM 21 Preston.xus				
Project Description								Stern				
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Demand (v), veh/h				268	181	65	70	127	251	48	718	98
Demand (v), veh/h				411	1007	489						
Signal Information												
Cycle, s	180.0	Reference Phase	2									
Offset, s	0	Reference Point	End	Green	13.0	22.4	59.8	29.0	24.8	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	3.5	3.5	4.3	3.6	3.6	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.7	2.4	2.4	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase					4			8	5	2	1	6
Case Number						9.0			11.0	2.0	3.0	2.0
Phase Duration, s							35.0		30.8	19.5	65.8	48.4
Change Period, (Y+R_c), s							6.0		6.0	6.5	6.0	6.0
Max Allow Headway (MAH), s							5.1		4.2	4.0	0.0	4.0
Queue Clearance Time (g_s), s								23.9	6.7		41.7	
Green Extension Time (g_e), s							0.0		0.9	0.1	0.0	0.2
Phase Call Probability								1.00	1.00		1.00	
Max Out Probability								0.71	0.00		1.00	
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h				221	242	67	203	259	49	740	101	409
Adjusted Saturation Flow Rate (s), veh/h/in				1795	1853	1610	1823	1598	1810	1795	1598	1795
Queue Service Time (g_s), s				21.1	22.5	6.0	19.3	21.9	4.7	31.2	6.4	39.7
Cycle Queue Clearance Time (g_c), s				21.1	22.5	6.0	19.3	21.9	4.7	31.2	6.4	39.7
Green Ratio (g/C)				0.17	0.17	0.24	0.14	0.38	0.07	0.33	0.47	0.24
Capacity (c), veh/h				299	309	376	261	592	131	1193	751	428
Volume-to-Capacity Ratio (X)				0.739	0.783	0.178	0.778	0.437	0.379	0.620	0.135	0.956
Back of Queue (Q), ft/in (95 th percentile)				399.9	444	112.2	385.4	345.7	100.4	513.6	114.5	587.9
Back of Queue (Q), veh/in (95 th percentile)				15.9	17.5	4.5	15.1	13.7	4.0	20.4	4.5	23.3
Queue Storage Ratio (RQ) (95 th percentile)				1.60	1.48	0.56	1.93	0.00	0.47	0.00	0.82	0.81
Uniform Delay (d_1), s/veh				71.3	71.9	55.2	74.4	42.6	79.6	50.5	27.0	50.5
Incremental Delay (d_2), s/veh				10.0	13.0	0.3	11.2	0.5	1.8	2.4	0.4	23.5
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				81.3	84.9	55.5	85.6	43.1	81.4	52.9	27.3	74.0
Level of Service (LOS)				F	F	E	F	D	F	D	C	E
Approach Delay, s/veh / LOS				79.7		E	61.8	E	51.6		D	28.7
Intersection Delay, s/veh / LOS							45.3				D	
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				2.48		B	2.49		B		2.11	B
Bicycle LOS Score / LOS				1.36		A	1.25		A		2.11	B

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary														
General Information						Intersection Information			Intersection Diagram					
Agency		Diane B. Zimmerman Traffic Engineering						Duration, h	0.250					
Analyst		DBZ		Analysis Date		Jun 2, 2021		Area Type	Other					
Jurisdiction				Time Period		PM Peak		PHF	0.97					
Urban Street		Preston Highway		Analysis Year		2022 No Build		Analysis Period	1> 4:45					
Intersection		Mt Washington Rd		File Name		PM 22 NB Preston.xus								
Project Description														
Demand Information				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T	R		
Demand (v), veh/h				269	182	65	70	128	252	48	722	98		
				269	182	65	70	128	252	48	722	98		
				413	1012	491								
Signal Information				EB		WB		NB		SB				
Cycle, s	180.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	Off	Green	13.0	22.6	59.6	29.0	24.8	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	3.6	3.6	0.0				
				Red	3.0	3.0	1.7	2.4	2.4	0.0				
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT			
Assigned Phase						4				1	6			
Case Number						9.0				2.0	3.0			
Phase Duration, s						35.0				48.6	94.7			
Change Period, (Y+R _c), s						6.0				6.5	6.0			
Max Allow Headway (MAH), s						5.1				4.0	0.0			
Queue Clearance Time (g _s), s							24.0			41.9				
Green Extension Time (g _e), s						0.0				0.1	0.0			
Phase Call Probability							1.00			1.00				
Max Out Probability							0.73			1.00				
Movement Group Results				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T	R		
Assigned Movement				7	4	14	3	8	18	5	2	12		
Adjusted Flow Rate (v), veh/h				222	243	67	204	260	49	744	101	411		
Adjusted Saturation Flow Rate (s), veh/h/in				1795	1853	1610	1823	1598	1810	1795	1598	1795		
Queue Service Time (g _s), s				21.1	22.6	6.0	19.4	22.0	4.7	31.5	6.5	39.9		
Cycle Queue Clearance Time (g _c), s				21.1	22.6	6.0	19.4	22.0	4.7	31.5	6.5	39.9		
Green Ratio (g/C)				0.17	0.17	0.24	0.14	0.38	0.07	0.33	0.47	0.24		
Capacity (c), veh/h				299	309	376	262	594	131	1189	750	429		
Volume-to-Capacity Ratio (X)				0.741	0.787	0.178	0.780	0.438	0.379	0.626	0.135	0.957		
Back of Queue (Q), ft/in (95 th percentile)				401.7	446.8	112.2	387.4	346.3	100.4	517.8	114.8	591		
Back of Queue (Q), veh/in (95 th percentile)				15.9	17.6	4.5	15.1	13.7	4.0	20.5	4.6	23.5		
Queue Storage Ratio (RQ) (95 th percentile)				1.61	1.49	0.56	1.94	0.00	0.47	0.00	0.82	0.82		
Uniform Delay (d ₁), s/veh				71.3	71.9	55.2	74.3	42.4	79.6	50.8	27.1	50.5		
Incremental Delay (d ₂), s/veh				10.2	13.3	0.3	11.4	0.5	1.8	2.5	0.4	23.6		
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay (d ₄), s/veh				81.5	85.3	55.5	85.8	43.0	81.4	53.3	27.4	74.1		
Level of Service (LOS)				F	F	E	F	D	F	D	C	E		
Approach Delay, s/veh / LOS				79.9		E	61.8		51.9		D	28.7		
Intersection Delay, s/veh / LOS							45.4				D			
Multimodal Results				EB		WB		NB		SB				
Pedestrian LOS Score / LOS				2.48		B	2.49		B		B	2.11		
Bicycle LOS Score / LOS				1.37		A	1.25		A		A	2.12		

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary														
General Information						Intersection Information								
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h	0.250							
Analyst	DBZ		Analysis Date	Jun 2, 2021			Area Type	Other						
Jurisdiction			Time Period	PM Peak			PHF	0.97						
Urban Street	Preston Highway		Analysis Year	2022 Build			Analysis Period	1 > 4:45						
Intersection	Mt Washington Rd		File Name	PM 22 B Preston.xus										
Project Description	Stern													
Demand Information			EB			WB			NB		SB			
Approach Movement			L	T	R	L	T	R	L	T	R			
Demand (v), veh/h			283	182	65	70	128	280	48	776	98	440 1067 504		
Signal Information														
Cycle, s	180.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	Off											
Force Mode	Fixed	Simult. Gap N/S	On											
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase				4			8	5	2	1	6			
Case Number					9.0			11.0	2.0	3.0	2.0	3.0		
Phase Duration, s						35.0			19.5	65.0	46.7	92.2		
Change Period, (Y+R c), s						6.0			6.5	6.0	6.5	6.0		
Max Allow Headway (MAH), s						5.1			4.0	0.0	4.0	0.0		
Queue Clearance Time (g e), s							26.8		6.7		43.2			
Green Extension Time (g e), s						0.0		0.5	0.0	0.0	0.0	0.0		
Phase Call Probability								1.00	1.00		1.00			
Max Out Probability									1.00	0.00		1.00		
Movement Group Results			EB			WB			NB		SB			
Approach Movement			L	T	R	L	T	R	L	T	R			
Assigned Movement			7	4	14	3	8	18	5	2	12	1 6 16		
Adjusted Flow Rate (v), veh/h			233	246	67	204	289	49	800	101	438	1061 501		
Adjusted Saturation Flow Rate (s), veh/h/ln			1795	1853	1610	1823	1598	1810	1795	1598	1795	1781 1572		
Queue Service Time (g s), s			22.4	23.0	6.0	19.1	24.8	4.7	34.7	6.3	41.2	27.4 16.8		
Cycle Queue Clearance Time (g c), s			22.4	23.0	6.0	19.1	24.8	4.7	34.7	6.3	41.2	27.4 16.8		
Green Ratio (g/C)			0.17	0.17	0.24	0.16	0.38	0.07	0.33	0.48	0.23	0.48 0.64		
Capacity (c), veh/h			299	309	376	287	599	131	1177	766	411	1706 1006		
Volume-to-Capacity Ratio (X)			0.780	0.797	0.178	0.712	0.482	0.379	0.680	0.132	1.065	0.622 0.498		
Back of Queue (Q), ft/ln (95 th percentile)			427.8	453.8	112.2	375.4	383.1	100.4	564.7	112.3	773.1	293.9 123.8		
Back of Queue (Q), veh/ln (95 th percentile)			17.0	17.9	4.5	14.7	15.2	4.0	22.4	4.5	30.7	11.6 4.8		
Queue Storage Ratio (RQ) (95 th percentile)			1.71	1.51	0.56	1.88	0.00	0.47	0.00	0.80	1.07	0.00 0.71		
Uniform Delay (d 1), s/veh			71.8	72.1	55.2	72.0	42.9	79.6	52.3	26.0	68.5	17.2 5.6		
Incremental Delay (d 2), s/veh			13.1	14.2	0.3	7.7	0.6	1.8	3.2	0.4	49.0	0.8 0.8		
Initial Queue Delay (d 3), s/veh			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 0.0		
Control Delay (d), s/veh			84.9	86.3	55.5	79.7	43.5	81.4	55.5	26.4	117.5	18.0 6.4		
Level of Service (LOS)			F	F	E	E	D	F	E	C	F	B A		
Approach Delay, s/veh / LOS			81.9	F		58.5	E	53.8	D	36.9	D			
Intersection Delay, s/veh / LOS					49.7				D					
Multimodal Results			EB			WB			NB		SB			
Pedestrian LOS Score / LOS			2.48	B		2.49	B	1.95	B	2.12	B			
Bicycle LOS Score / LOS			1.39	A		1.30	A	1.27	A	2.20	B			

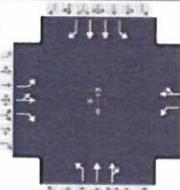
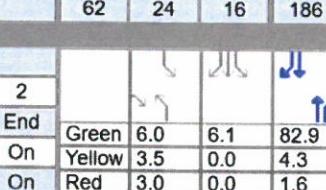
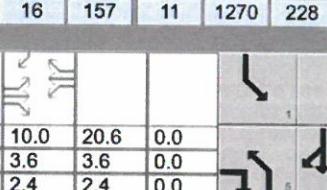
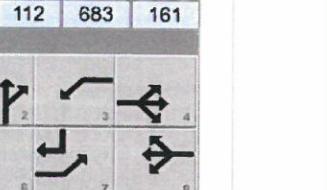
Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																	
General Information								Intersection Information									
Agency	Diane B. Zimmerman Traffic Engineering						Duration, h		0.250								
Analyst	DBZ		Analysis Date		Jun 2, 2021		Area Type		Other								
Jurisdiction							Time Period		PM Peak								
Urban Street	Preston Highway						Analysis Year		2032 No Build								
Intersection	Mt Washington Rd		File Name		PM 32 NB Preston.xus												
Project Description	Stern																
Demand Information				EB		WB		NB		SB							
Approach Movement				L	T	R	L	T	R	L	T	R					
Demand (v), veh/h				283	191	68	74	135	265	50	759	103					
Signal Information				EB		WB		NB		SB							
Cycle, s	180.0	Reference Phase	2														
Offset, s	0	Reference Point	End														
Uncoordinated	No	Simult. Gap E/W	Off	Green	13.0	22.0	59.0	29.0	26.0	0.0							
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	3.6	3.6	0.0							
				Red	3.0	3.0	1.7	2.4	2.4	0.0							
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT						
Assigned Phase					4			5	2	1	6						
Case Number					9.0		11.0	2.0	3.0	2.0	3.0						
Phase Duration, s					35.0		32.0	19.5	65.0	48.0	93.5						
Change Period, (Y+R_c), s					6.0		6.0	6.5	6.0	6.5	6.0						
Max Allow Headway (MAH), s					5.1		4.2	4.0	0.0	4.0	0.0						
Queue Clearance Time (g_s), s							25.2	6.9		44.5							
Green Extension Time (g_e), s					0.0		0.7	0.1	0.0	0.0	0.0						
Phase Call Probability							1.00	1.00		1.00							
Max Out Probability							1.00	0.00		1.00							
Movement Group Results				EB		WB		NB		SB							
Approach Movement				L	T	R	L	T	R	L	T	R					
Assigned Movement				7	4	14	3	8	18	5	2	12					
Adjusted Flow Rate (v), veh/h				233	255	70	215	273	52	782	106	431					
Adjusted Saturation Flow Rate (s), veh/h/ln				1795	1853	1610	1823	1598	1810	1795	1598	1795					
Queue Service Time (g_s), s				22.4	24.0	6.3	20.5	23.2	4.9	33.7	6.8	42.5					
Cycle Queue Clearance Time (g_c), s				22.4	24.0	6.3	20.5	23.2	4.9	33.7	6.8	42.5					
Green Ratio (g/C)				0.17	0.17	0.24	0.15	0.38	0.07	0.33	0.47	0.24					
Capacity (c), veh/h				299	309	376	273	599	131	1178	755	424					
Volume-to-Capacity Ratio (X)				0.780	0.826	0.187	0.789	0.456	0.394	0.664	0.141	1.018					
Back of Queue (Q), ft/in (95 th percentile)				427.8	477	117.7	407.7	362.6	104.8	550.5	120.1	679.4					
Back of Queue (Q), veh/in (95 th percentile)				17.0	18.8	4.7	15.9	14.4	4.2	21.8	4.8	27.0					
Queue Storage Ratio (RQ) (95 th percentile)				1.71	1.59	0.59	2.04	0.00	0.49	0.00	0.86	0.94					
Uniform Delay (d_1), s/veh				71.8	72.5	55.3	73.8	42.5	79.7	52.0	26.8	53.1					
Incremental Delay (d_2), s/veh				13.1	17.3	0.3	12.6	0.5	1.9	3.0	0.4	38.0					
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					
Control Delay (d), s/veh				84.9	89.7	55.6	86.4	43.0	81.7	54.9	27.2	91.1					
Level of Service (LOS)				F	F	E	F	D	F	D	C	F					
Approach Delay, s/veh / LOS				83.5	F		62.1	E	53.3	D		33.7					
Intersection Delay, s/veh / LOS							48.8			D							
Multimodal Results				EB		WB		NB		SB							
Pedestrian LOS Score / LOS				2.48	B		2.49	B	1.95	B	2.12	B					
Bicycle LOS Score / LOS				1.41	A		1.29	A	1.26	A	2.20	B					

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary												
General Information						Intersection Information						
Agency	Diane B. Zimmerman Traffic Engineering					Duration, h						
Analyst	DBZ			Analysis Date	Jun 2, 2021		Area Type					
Jurisdiction				Time Period	PM Peak		PHF					
Urban Street	Preston Highway			Analysis Year	2032 Build		Analysis Period					
Intersection	Mt Washington Rd			File Name	PM 32 B Preston.xus							
Project Description	Stern											
Demand Information			EB			WB			NB			SB
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	297	191	68	74	135	293	50	813	103	461	1119	529
Signal Information												
Cycle, s	180.0	Reference Phase	2									
Offset, s	0	Reference Point	End	Green	13.0	19.6	59.0	29.0	28.4	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	3.5	3.5	4.3	3.6	3.6	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.7	2.4	2.4	0.0		
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT		
Assigned Phase				4			8	5	2	1	6	
Case Number					9.0			11.0	2.0	3.0	2.0	3.0
Phase Duration, s					35.0			34.4	19.5	65.0	45.6	91.1
Change Period, (Y+R c), s					6.0			6.0	6.5	6.0	6.5	6.0
Max Allow Headway (MAH), s					5.1			4.2	4.0	0.0	4.0	0.0
Queue Clearance Time (g s), s							28.2	6.9		42.1		
Green Extension Time (g e), s					0.0			0.2	0.0	0.0	0.0	0.0
Phase Call Probability								1.00	1.00		1.00	
Max Out Probability								1.00	0.02		1.00	
Movement Group Results			EB			WB			NB			SB
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h	245	258	70		215	302	52	838	106	450	1092	516
Adjusted Saturation Flow Rate (s), veh/h/ln	1795	1853	1610		1823	1598	1810	1795	1598	1795	1781	1572
Queue Service Time (g s), s	23.7	24.3	6.3		20.2	26.2	4.9	36.9	6.6	40.1	28.0	17.5
Cycle Queue Clearance Time (g c), s	23.7	24.3	6.3		20.2	26.2	4.9	36.9	6.6	40.1	28.0	17.5
Green Ratio (g/C)	0.17	0.17	0.24		0.16	0.38	0.07	0.33	0.49	0.22	0.47	0.63
Capacity (c), veh/h	299	309	376		298	599	131	1177	776	400	1683	997
Volume-to-Capacity Ratio (X)	0.819	0.836	0.187		0.723	0.504	0.394	0.712	0.137	1.126	0.649	0.518
Back of Queue (Q), ft/in (95 th percentile)	456	484.7	117.7		394.1	401.3	104.8	596.1	116.6	846.1	273.4	117.6
Back of Queue (Q), veh/in (95 th percentile)	18.1	19.1	4.7		15.4	15.9	4.2	23.7	4.6	33.6	10.8	4.6
Queue Storage Ratio (RQ) (95 th percentile)	1.82	1.62	0.59		1.97	0.00	0.49	0.00	0.83	1.17	0.00	0.67
Uniform Delay (d 1), s/veh	72.4	72.6	55.3		71.4	43.4	79.7	53.1	25.5	71.2	16.3	5.4
Incremental Delay (d 2), s/veh	16.8	18.4	0.3		8.6	0.7	1.9	3.7	0.4	69.6	0.8	0.8
Initial Queue Delay (d 3), s/veh	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	89.2	91.0	55.6		80.0	44.0	81.7	56.7	25.9	140.8	17.1	6.1
Level of Service (LOS)	F	F	E		F	D	F	E	C	F	B	A
Approach Delay, s/veh / LOS	85.9	F		59.0	E		54.7	D		41.4	D	
Intersection Delay, s/veh / LOS				52.9						D		
Multimodal Results			EB			WB			NB			SB
Pedestrian LOS Score / LOS	2.48	B		2.49	B		1.95	B		2.12	B	
Bicycle LOS Score / LOS	1.43	A		1.34	A		1.31	A		2.28	B	

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary																				
General Information						Intersection Information														
Agency	Diane B. Zimmerman Traffic Engineering			Duration, h			0.250													
Analyst	DBZ		Analysis Date	Jun 2, 2021		Area Type														
Jurisdiction			Time Period	AM Peak		PHF														
Urban Street	Preston Highway		Analysis Year	2032 Build		Analysis Period														
Intersection	Interchange Drive		File Name	AM 32 B Preston no riro.xus																
Project Description	Stern No Ri/ro																			
Demand Information				EB		WB		NB		SB										
Approach Movement				L	T	R	L	T	R	L	T	R								
Demand (v), veh/h				62	24	16	186	16	157	11	1270	228								
Signal Information																				
Cycle, s	150.0	Reference Phase	2	Green	6.0	6.1	82.9	10.0	20.6	0.0										
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	4.3	3.6	3.6	0.0										
Uncoordinated	No	Simult. Gap E/W	On	Red	3.0	0.0	1.6	2.4	2.4	0.0										
Force Mode	Fixed	Simult. Gap N/S	On																	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT									
Assigned Phase					4			5	2	1	6									
Case Number					9.0		10.0	2.0	4.0	2.0	3.0									
Phase Duration, s					16.0		26.6	12.5	88.8	18.6	94.9									
Change Period, (Y+R_c), s					6.0		6.0	6.5	5.9	6.5	5.9									
Max Allow Headway (MAH), s					4.0		4.1	4.0	0.0	4.0	0.0									
Queue Clearance Time (g_s), s					7.9		19.3	3.1		11.9										
Green Extension Time (g_e), s					0.3		1.3	0.0	0.0	0.3	0.0									
Phase Call Probability					1.00		1.00	1.00		0.99										
Max Out Probability					0.00		0.00	0.00		0.00										
Movement Group Results				EB		WB		NB		SB										
Approach Movement				L	T	R	L	T	R	L	T	R								
Assigned Movement				7	4	14	3	8	18	5	2	12								
Adjusted Flow Rate (v), veh/h				69	27	18	207	192		11	794	767								
Adjusted Saturation Flow Rate (s), veh/h/in				1711	1900	1359	1810	1633		1527	1856	1758								
Queue Service Time (g_s), s				5.9	2.0	1.8	16.7	17.3		1.1	45.2	46.1								
Cycle Queue Clearance Time (g_c), s				5.9	2.0	1.8	16.7	17.3		1.1	45.2	46.1								
Green Ratio (g/C)				0.07	0.07	0.11	0.14	0.14		0.04	0.55	0.55								
Capacity (c), veh/h				114	127	145	248	224		61	1025	971								
Volume-to-Capacity Ratio (X)				0.604	0.211	0.123	0.833	0.859		0.188	0.774	0.789								
Back of Queue (Q), ft/in (95 th percentile)				127.4	44	32.2	316	302.5		23.4	557.5	516.9								
Back of Queue (Q), veh/in (95 th percentile)				4.8	1.8	1.1	12.6	12.1		0.8	21.8	20.7								
Queue Storage Ratio (RQ) (95 th percentile)				0.32	0.00	0.54	0.00	0.00		0.09	0.00	0.00								
Uniform Delay (d_1), s/veh				68.1	66.3	60.6	63.0	63.3		72.5	19.6	18.9								
Incremental Delay (d_2), s/veh				5.1	0.8	0.4	7.2	9.2		0.7	3.0	3.4								
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0								
Control Delay (d), s/veh				73.1	67.1	61.0	70.2	72.5		73.2	22.5	22.3								
Level of Service (LOS)				E	E	E	E	E		E	C	C								
Approach Delay, s/veh / LOS				69.8		E	71.3		E	22.8		C								
Intersection Delay, s/veh / LOS							30.6			C										
Multimodal Results				EB		WB		NB		SB										
Pedestrian LOS Score / LOS				2.32	B	2.49	B	1.91	B	2.09	B									
Bicycle LOS Score / LOS				0.67	A	1.15	A	1.87	B	1.36	A									

Preston Highway at Interchange Drive
Traffic Impact Study

HCS7 Signalized Intersection Results Summary															
General Information						Intersection Information			Diagram						
Agency		Diane B. Zimmerman Traffic Engineering						Duration, h	0.250						
Analyst		DBZ		Analysis Date		Jun 2, 2021		Area Type	Other						
Jurisdiction		Time Period		PM Peak		PHF		0.98							
Urban Street		Preston Highway		Analysis Year		2032 Build		Analysis Period	1> 4:45						
Intersection		Interchange Dr		File Name		PM 32 B Preston No riro.xus									
Project Description															
Demand Information				EB		WB		NB		SB					
Approach Movement				L	T	R	L	T	R	L	T	R			
Demand (v), veh/h				183	25	50	168	14	95	17	1235	152			
											144	1877	164		
Signal Information															
Cycle, s	180.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					9.0		10.0	2.0	4.0	2.0	3.0				
Phase Duration, s					31.0		28.0	10.0	98.4	22.6	111.0				
Change Period, (Y+R c), s					6.0		6.0	6.5	5.9	6.5	5.9				
Max Allow Headway (MAH), s					4.0		3.1	4.0	0.0	3.0	0.0				
Queue Clearance Time (g s), s					19.8		18.5	3.7		16.0					
Green Extension Time (g e), s					0.6		0.4	0.0	0.0	0.2	0.0				
Phase Call Probability					1.00		1.00	0.58		1.00					
Max Out Probability					0.04		0.00	0.00		0.00					
Movement Group Results				EB		WB		NB		SB					
Approach Movement				L	T	R	L	T	R	L	T	R			
Assigned Movement				7	4	14	3	8	18	5	2	12			
Adjusted Flow Rate (v), veh/h				187	26	51	171	111		17	722	700			
Adjusted Saturation Flow Rate (s), veh/h/ln				1810	1900	1610	1810	1642		1810	1885	1813			
Queue Service Time (g s), s				17.8	2.1	5.0	16.5	11.5		1.7	52.0	50.6			
Cycle Queue Clearance Time (g c), s				17.8	2.1	5.0	16.5	11.5		1.7	52.0	50.6			
Green Ratio (g/C)				0.14	0.14	0.16	0.12	0.12		0.02	0.51	0.51			
Capacity (c), veh/h				251	264	255	221	201		35	968	931			
Volume-to-Capacity Ratio (X)				0.743	0.097	0.200	0.775	0.554		0.497	0.746	0.752			
Back of Queue (Q), ft/ln (95 th percentile)				340.3	46.2	92	308.7	210.9		38.9	730.7	661.5			
Back of Queue (Q), veh/ln (95 th percentile)				13.6	1.8	3.7	12.3	8.4		1.6	29.0	26.5			
Queue Storage Ratio (RQ) (95 th percentile)				0.85	0.00	1.53	0.00	0.00		0.16	0.00	0.00			
Uniform Delay (d 1), s/veh				74.4	67.6	65.9	76.6	74.4		88.1	30.5	27.5			
Incremental Delay (d 2), s/veh				8.4	0.2	0.4	3.0	0.9		6.4	3.2	3.4			
Initial Queue Delay (d 3), s/veh				0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0			
Control Delay (d), s/veh				82.8	67.8	66.2	79.6	75.3		94.5	33.7	30.9			
Level of Service (LOS)				F	E	E	E	E		F	C	C			
Approach Delay, s/veh / LOS				78.2		E	77.9	E		33.1	C				
Intersection Delay, s/veh / LOS							40.5			D					
Multimodal Results				EB		WB		NB		SB					
Pedestrian LOS Score / LOS				2.33	B		2.49	B		1.92	B				
Bicycle LOS Score / LOS				0.92	A		0.95	A		1.67	B				

Date: September 29, 2021

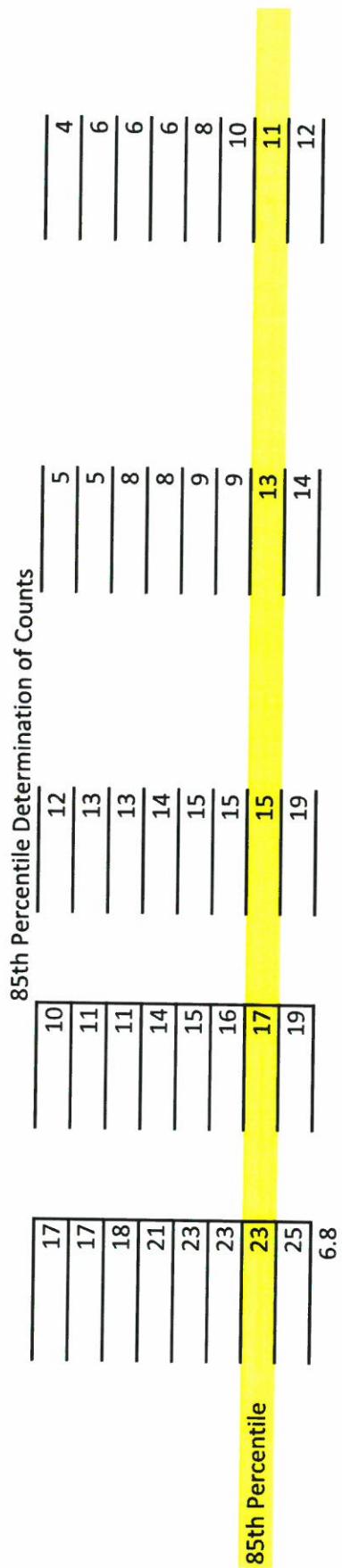
Parking Spaces Site	41	56	44	20	South	4,500	At the Thorntons pumps	Time	Thorntons pumps	At the Thorntons pumps	Time	4,000
Costco	3,400	Kohls	4,000	North	4,500	At the Thorntons pumps	5	3	10:36	12	7	At the Thorntons pumps
Time	Chick Fil A	Time	Chick Fil A	Time	Thorntons	pumps	Time	Thorntons	pumps	Time	Thorntons	pumps
10:18	21	10:41	16	10:14	14		2	10:16	5	3	10:36	12
11:59	17	11:00	14	11:58	13		7	11:57	14	9	11:01	11
12:11	17	12:40	19	12:07	12		7	12:03	13	7	12:37	10
1:59	23	1:00	17	1:55	19		9	1:57	9	10	1:03	8
2:01	23	2:22	11	2:05	15		10	2:09	8	1	2:25	6
3:26	23	3:02	10	3:21	15		9	3:24	9	4	3:00	6
4:05	18	4:24	15	4:00	13		10	4:03	8	3	4:31	4
5:39	25	5:04	11	5:32	15		9	5:36	5	2	5:00	6
Average	20.9	14.1		14.5								7.9
Peak Use	25	19		19								12
85th Percentile	23	17		15								11
Peak per sq ft	7.35	4.75		4.22								3.00

ITE 5th Ed Parking Site from above	934 Fast Food w/ Drive-Thru	Site from above	960 Super Convenience with Gas		
			1	2	3
Average	8.66	29	35	36	32
85th Percentile	13.78	47	55	50	45

Calculations for Stern Site using ITE Parking Manual 85th Percentile for Max

5,240	72	4,500	50
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- 1 Chick Fil A 3420 Bardstown Road in front of Costco
- 2 Chick Fil A 7901 Bardstown Road at Kohl's
- 3 Thornton's 3255 Bardstown Road
- 3 Thornton's 3300 Bardstown Road
- 3 Thornton's 7920 Bardstown Road



Date: September 29, 2021

Parking Spaces Site	41	56	44	24	24	21					
Costco	3,400	Kohls	4,000	North	4,500	South	4,500	Ichobod	4,000	At the pumps	At the pumps
Time	Chick Fil A	Time	Chick Fil A	Time	Thorntons pumps	Time	Thorntons pumps	Time	Thorntons pumps	Time	Thorntons pumps
10:18	21	10:41	16	10:14	14	2	10:16	5	3	10:36	12
11:59	17	11:00	14	11:58	13	7	11:57	14	9	11:01	11
12:11	17	12:40	19	12:07	12	7	12:03	13	7	12:37	10
1:59	23	1:00	17	1:55	19	9	1:57	9	10	1:03	8
2:01	23	2:22	11	2:05	15	10	2:09	8	1	2:25	6
3:26	23	3:02	10	3:21	15	9	3:24	9	4	3:00	6
4:05	18	4:24	15	4:00	13	10	4:03	8	3	4:31	4
5:39	25	5:04	11	5:32	15	9	5:36	5	2	5:00	6
Average	20.9		14.1		14.5			8.9		7.9	
Peak Use	25		19		19			14		12	
85th Percentile	23		17		15			13		11	
Peak per sq ft	7.35		4.75		4.22			3.11		3.00	

ITE 5th Ed Parking 934 Fast Food w/ Drive-Thru

Site from above 1 2

Site from above 1 2 3

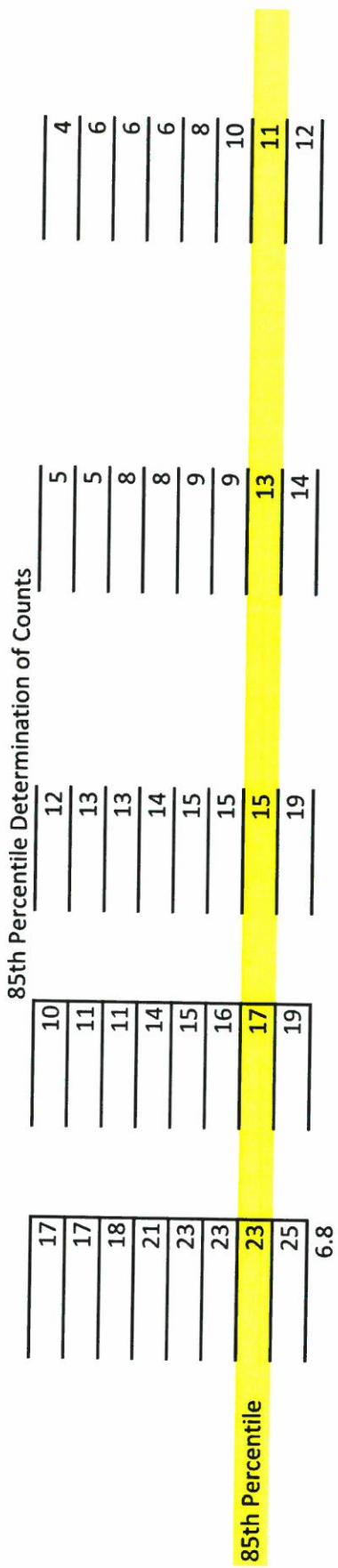
960 Super Convenience with Gas

	1	2	3
Average	8.11	36	36
85th Percentile	11.15	50	50

Calculations for Stern Site using ITE Parking Manual 85th Percentile for Max

5,240	72
4,500	50

1 Chick Fil A 3420 Bardstown Road in front of Costco
2 Chick Fil A 7901 Bardstown Road at Kohl's
3 Thornton's 3225 Bardstown Road
3300 Bardstown Road
7920 Bardstown Road



Parking Study for Chick Fil A

On September 29, 2021 and October 12, 2021, parking studies were conducted at the following locations:

- St. Matthews Chick Fil A, located at 5001 Shelbyville Road from 9:00 am to 8:00 pm.
- Fern Creek Chick Fil A, located at 3420 Bardstown Road from 10:00 am to 5:30 pm.
- Hikes Point Chick Fil A, located at 7901 Bardstown Road from 10:30 am to 5:00 pm.

The results of these parking studies are set forth below, which included the offsite employee parking that was leased by the above-named Chick Fil A.

Date: **October 12, 2021** **September 29, 2021** **September 29, 2021**

Time	CFA - St. Matthews	Time	CFA - Fern Creek	Time	CFA - Hikes Point
9:05	27				
10:02	29	10:18	21	10:41	16
11:00	36	11:59	17	11:00	14
12:11	63	12:11	17	12:40	19
1:10	52	1:59	23	1:00	17
2:01	44	2:01	23	2:22	11
3:05	46	3:26	23	3:02	10
4:05	27	4:05	18	4:24	15
5:05	44	5:39	25	5:04	11
6:00	37				
7:06	46				
8:10	31				
Average	41.36		20.9		14.1

