

SALES CONTRACT

AD General Contracting LLC

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Angie's List

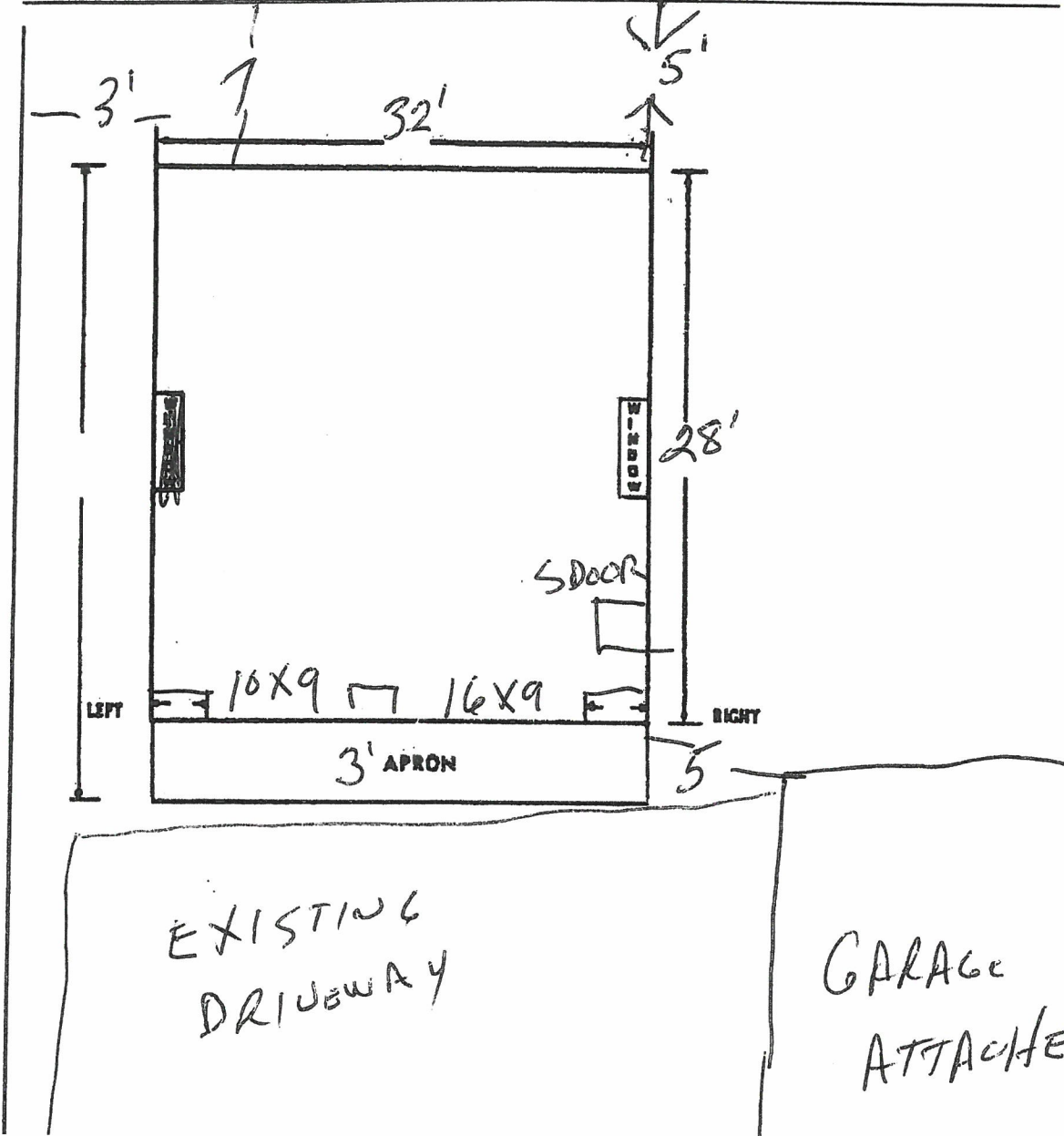
Garages, Porches,
Steps, Driveways
All Types of Concrete Const.
Room Additions

"Recommended by the people we serve"

Cell Phone: 419-0958 PO Box 379
web: www.adgeneralcontracting.com Mt. Washington, KY 40047

EXHIBIT A

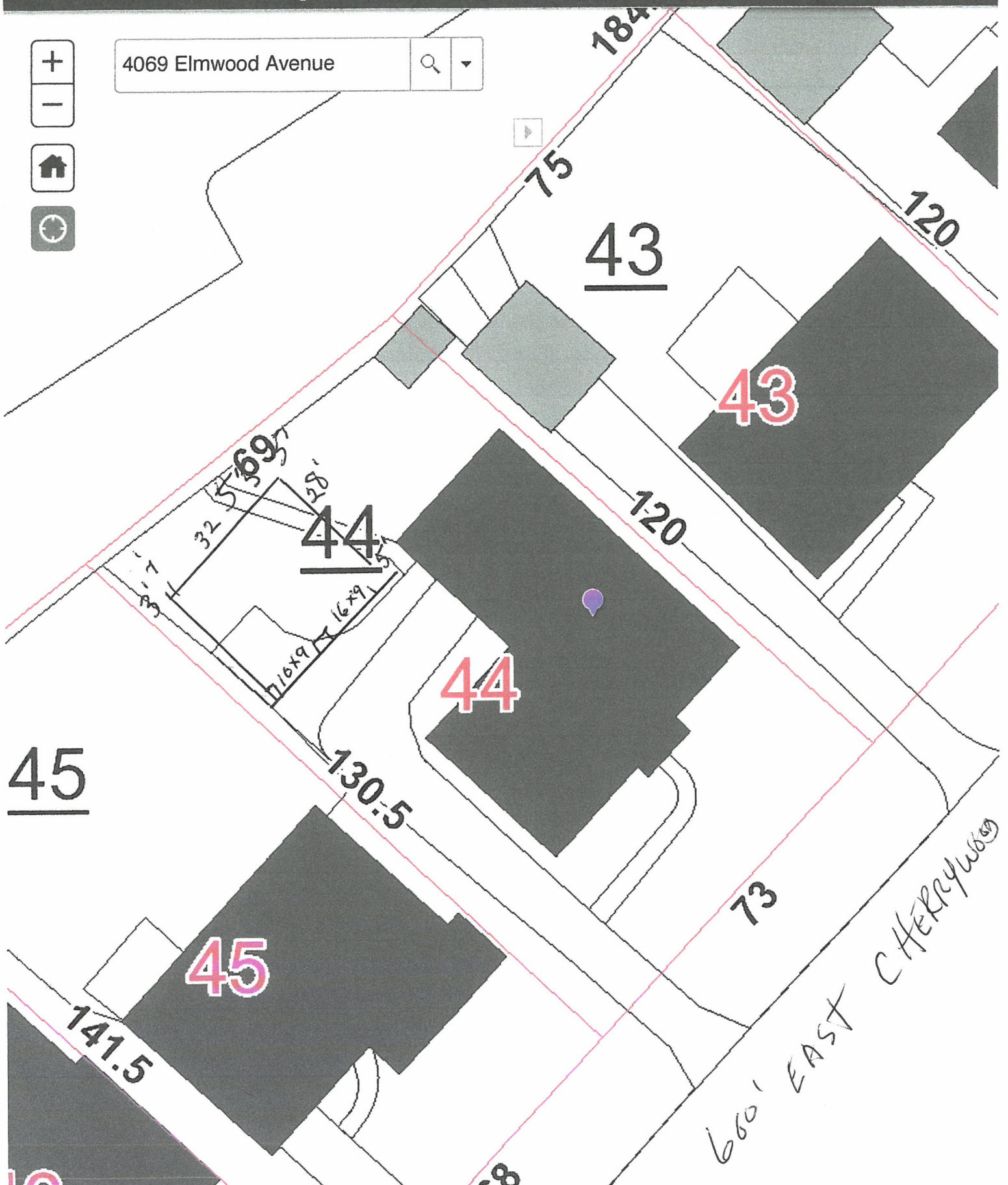
CUSTOMERS NAME MR DAVE GAMBOL COARSED HIGH 12'
 ADDRESS 4069 ELMWOOD AVE 40207 FRONT BRICK FRONT
 SUBDIVISION _____ ROOF R GABLE
 PERMISSION TO USE DRIVE _____ SALESMAN _____
 NEEDED FILL _____ LOT SIZE _____



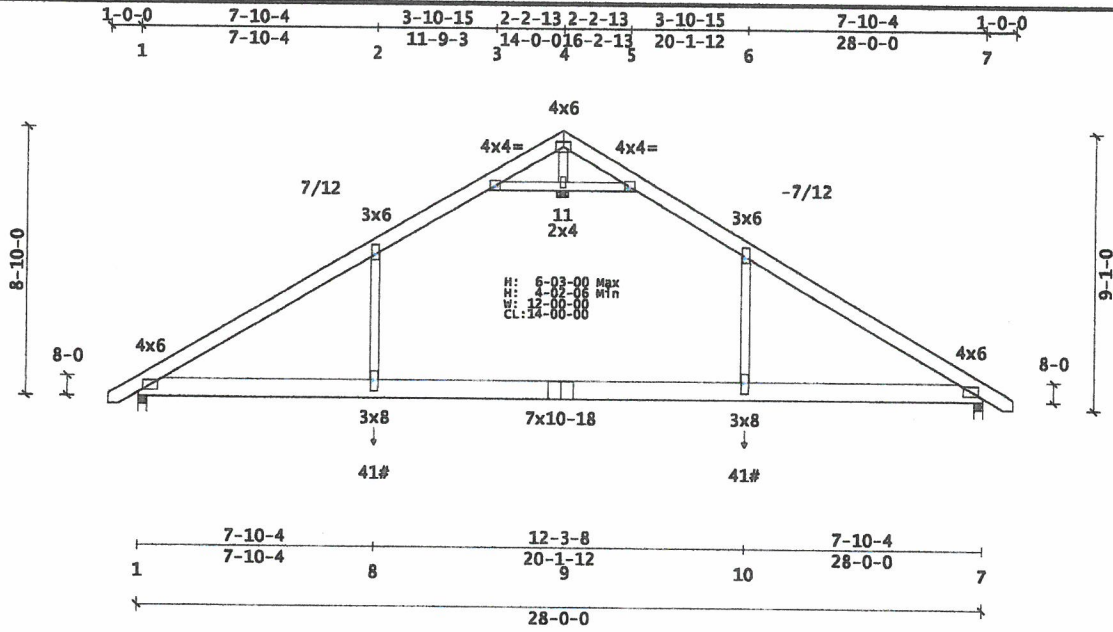




4069 Elmwood Avenue



Truss Mfr. Contact: Levi Yoder



Truss Weight = 198.2 lb

Code/Design: IBC-2018/TPI-2014
 PSF Live Dead Dur Factors
 TC 20.0 10.0 Live Wind Snow
 BC 0.0 10.0 Lum 1.15 1.60 1.15
 Total 40.0 Plt 1.15 1.60 1.15
 Spacing: 2-00-00 o.c. Plies: 1
 Repetitive Member Increase: Yes
 Green Lumber: No Wet Service: No
 Fab Tolerance: 20% Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

Snow Load Specs
 ASCE7-16 Ground Snow(Pg) = 25.0 psf
 Risk Cat: II Terrain Cat: C
 Roof Exposure: Sheltered
 Thermal Condition: All Others(1.0)
 Unobstructed Slippery Roof: No
 Low-Slope Minimums(Pfmin): No
 Unbalanced Snow Loads: Yes
 Rain Surcharge: No Ice Dam Chk: No
 Lu(max) = 20-00-00

Wind Load Specs
 ASCE7-16 Wind Speed(V) = 115 mph
 Risk Cat: II Exposure Cat: C
 Bldg Dims: L = 0.0 ft B = 0.0 ft
 M.R.H(h) = 15.0 ft Kzt = 1.0
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 6.0 BC = 6.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: ASCE7 C&C
 C&C End Zone: 3-00-00

Additional Design Checks
 10 psf Non-Concurrent BCLL: Yes
 20 psf BC Limited Storage: Yes
 200 lb BC Accessible Ceiling: No
 300 lb TC Maintenance Load: No
 2000 lb TC Safe Load: No
 Unbalanced TLL: Yes

Material Summary

TC	2x6	SP	2400/2.0
BC	2x8	SP	2400/2.0
Webs	2x4	SP (ALSC6-2013)	#1

Member Forces Summary

Mem	Ten	Comp	CSI
TC OH-1	32	0	0.01
1-2	0	2473	0.90
2-3	100	1867	0.69
3-4	814	0	0.73
4-5	814	0	0.73
5-6	100	1867	0.69
6-7	0	2473	0.90
7-OH	32	0	0.01
BC 1-8	1946	0	0.26
7-10	1946	0	0.26
8-9	1946	0	0.60
9-10	1946	0	0.60
Web 2-8	987	0	0.19
3-11	69	2657	0.33
4-11	69	0	0.01
5-11	69	2657	0.33
6-10	987	0	0.19

Reaction Summary

Reaction Summary (Lbs)

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat	PSI
1	01-12	1628	43	03-08	02-05	SPF	470
7	27-10-04	1628	43	03-08	02-05	SPF	470

Max Horiz = -159 / +159 at Joint 1

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [14-00-00] using a 1.00 Full and 0.00 Reduced load factor.
 Attic space centered at 14-00-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

See Loadcase Report for loading combinations and additional details.
 Dead Loads may be slope adjusted: > 12.0/12
 Loads based on maximum and minimum reactions from tie-in spans

Domain	Max	Min	Location	Dir	Description
BC	41	24	7-10-04	Vert	SidewallDL
BC	41	24	20-01-12	Vert	SidewallDL

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/627 (-0.53)	8-10
Vert DL	L/120	L/801 (-0.42)	8-10
Vert CR	L/180	L/351 (-0.94)	8-10
Horz LL	0.75in	(0.02)	@Jt 1
Horz CR	1.25in	(0.05)	@Jt 1
Ohng CR	2L/180	2L/999 (-0.00)	1- 1
Ohng CR	2L/180	2L/999 (-0.00)	7- 7

Bracing Data Summary

Chords; continuous except where shown
 Attic tie beam (Tib) & walls; bracing indicated or rigid sheathing.

Purlins

oc	From	To	#Bays	
TB	2-06-00	11-06-03	16-05-13	2

Web Bracing -- CLR

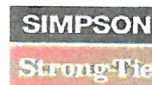
Single: 3-11 11- 5
 Continuous Restraint Bracing Req'd
 See BCST-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt2(0,00-14), Jnt3(0,00-04),
 Jnt5(0,00-04), Jnt6(0,00-14),
 Jnt8(0,-00-11), Jnt10(0,-00-11)



NOTICE A copy of this design shall be furnished to the erection contractor. The design of individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

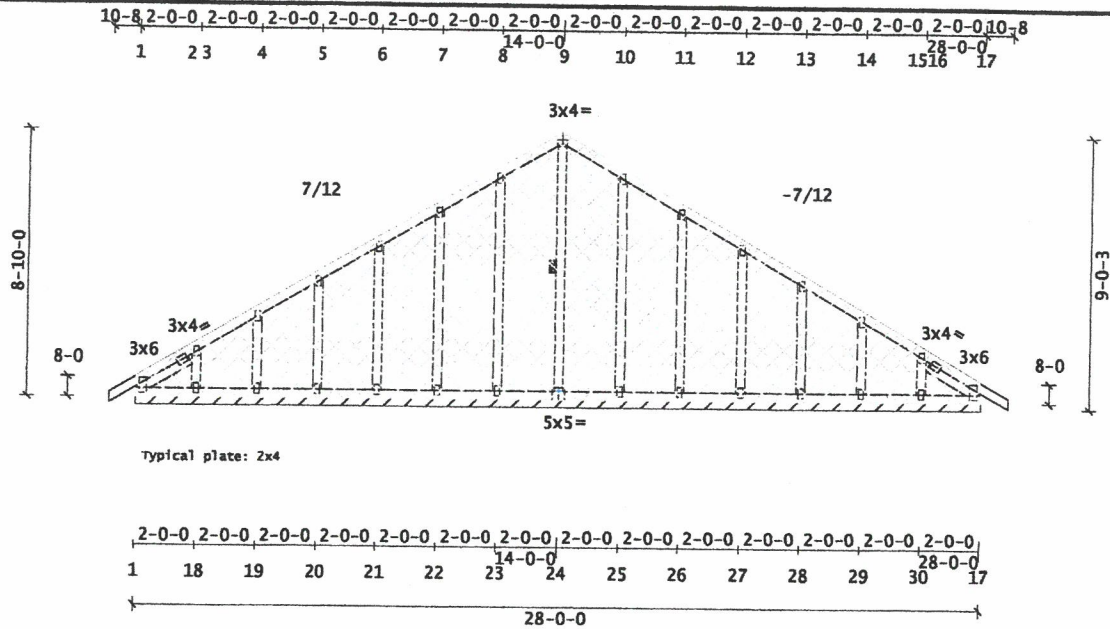


Component Solutions
 Truss Studio V
 2021.3.5.2

Customer: 84-Georgetown

SID: 000
TID: 136
Date: 10/
Page: 1 of

Truss Mfr. Contact: Levi Yoder



Truss Weight = 191.7 lb

<p>Code/Design: IBC-2018/TPI-2014</p> <p>PSF Live Dead Dur Factors</p> <p>TC 20.0 10.0 Live Wind Snow</p> <p>BC 0.0 10.0 Lum 1.15 1.60 1.15</p> <p>Total 40.0 Plt 1.15 1.60 1.15</p> <p>Spacing: 2-00-00 o.c. Plies: 1</p> <p>Repetitive Member Increase: Yes</p> <p>Green Lumber: No Wet Service: No</p> <p>Fab Tolerance: 20% Creep (Kcr) = 2.0</p> <p>OH Soffit Load: 2.0 psf</p>	<p>Snow Load Specs</p> <p>ASCE7-16 Ground Snow(Pg) = 25.0 psf</p> <p>Risk Cat: II Terrain Cat: C</p> <p>Roof Exposure: Sheltered</p> <p>Thermal Condition: All Others(1.0)</p> <p>Unobstructed Slippery Roof: No</p> <p>Low-Slope Minimums(Pfmin): No</p> <p>Unbalanced Snow Loads: Yes</p> <p>Rain Surcharge: No Ice Dam Chk: No</p> <p>Lu(max) = 20-00-00</p>	<p>Wind Load Specs</p> <p>ASCE7-16 Wind Speed(V) = 115 mph</p> <p>Risk Cat: II Exposure Cat: C</p> <p>Bldg Dims: L = 0.0 ft B = 0.0 ft</p> <p>M.R.H(h) = 15.0 ft Kzt = 1.0</p> <p>Bldg Enclosure: Enclosed</p> <p>Wind DL(psf): TC = 6.0 BC = 6.0</p> <p>Wind Vertical Exposed: L = Yes R = Yes</p> <p>Wind Uplift Reporting: ASCE7 C&C</p> <p>C&C End Zone: 3-00-00</p>	<p>Additional Design Checks</p> <p>10 psf Non-Concurrent BCLL:</p> <p>20 psf BC Limited Storage:</p> <p>200 lb BC Accessible Ceiling:</p> <p>300 lb TC Maintenance Load:</p> <p>2000 lb TC Safe Load:</p> <p>Unbalanced TCELL:</p>
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Material Summary

TC	2x4	SP (ALSC6-2013)	#1
BC	2x4	SP (ALSC6-2013)	#1
Webs	2x4	SP (ALSC6-2013)	#1

Reaction Summary

Reactions not shown: down < 400 and up < 150

---- Reaction Summary (plf) ----

Jnt-Jnt	React	-Up-	--Width-
1-17	83	12	28-00-00

Max Horiz = -161 / +161 at Joint 24

Deflection Summary

TrussSpan	Limit	Actual(in)	Locat
Vert LL	L/240	L/999(-0.00)	29-3
Vert DL	L/120	L/999(-0.00)	18-1
Vert CR	L/180	L/999(-0.00)	18-1
Horz LL	0.75in	(0.00)	8Jt1
Horz CR	1.25in	(0.00)	8Jt1
Ohng CR	2L/180	2L/999(-0.00)	1-1
Ohng CR	2L/180	2L/999(-0.00)	17-1

Member Forces Summary

Mem...	Ten	Comp	.CSI.
TC 1-9	201	129	0.06
TC 9-17	201	129	0.06
BC 1-24	115	57	0.03
BC 17-24	115	57	0.03
Web 1-2	6	12	0.01
Web 3-18	82	108	0.01
Web 4-19	68	123	0.01
Web 5-20	68	119	0.03
Web 6-21	68	120	0.04
Web 7-22	72	171	0.10
Web 8-23	62	201	0.17
Web 9-24	37	102	0.04
Web 10-25	62	201	0.17
Web 11-26	72	171	0.10
Web 12-27	68	120	0.04
Web 13-28	68	119	0.03
Web 14-29	68	123	0.01
Web 15-30	82	108	0.01
Web 16-17	6	12	0.01

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [14-00-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for loading combinations and additional details.

Dead Loads may be slope adjusted: > 12.0/12

Notes

Gable webs are attached with min. 1x3 20 ga. plates. The max. rake overhang = 1/2 the truss spacing. If this truss is exposed to wind loads perpendicular to the plane of the truss, it must be braced according to a standard detail matching the wind criteria shown, or according to the Construction Documents and/or BCSI - B3.

Gable requires 7/16" OSB sheathing on front from 0 to 28-00-00; connection details to be provided by building designer.

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees

Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Bracing Data Summary

-----Bracing Data-----

Chords: continuous except where shown

----- Web Bracing ----- CLR

Single: 24-9

Continuous Restraint Bracing Req'd

See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)

Jnt24(0, -01-00)

