

final report

February 26, 2016

## Traffic Impact Study

*Sutherland Pointe  
Aiken Road*

Prepared for

Louisville Metro

**DIANE B. ZIMMERMAN TRAFFIC ENGINEERING, LLC.**

12803 High Meadows Pike  
Prospect, KY 40059  
502-648-1858

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## INTRODUCTION

The development plan for the Sutherland Pointe residential subdivision on Aiken Road shows 241 buildable single family lots. **Figure 1** displays a map of the site. The recently approved Stapleton Ridge is also shown in Figure 1. Access to the development will be from Aiken Road at two entrances. Eleven lots will access Aiken Road using the existing Old Aiken Road. The purpose of this study is to examine the traffic impacts of the development upon the adjacent highway system. For this study the impact area was defined as the intersections of Aiken Road at Beckley Station Road, Arnold Palmer Boulevard, Bush Farm Road, and Bush Farm Road at Old Henry Road.

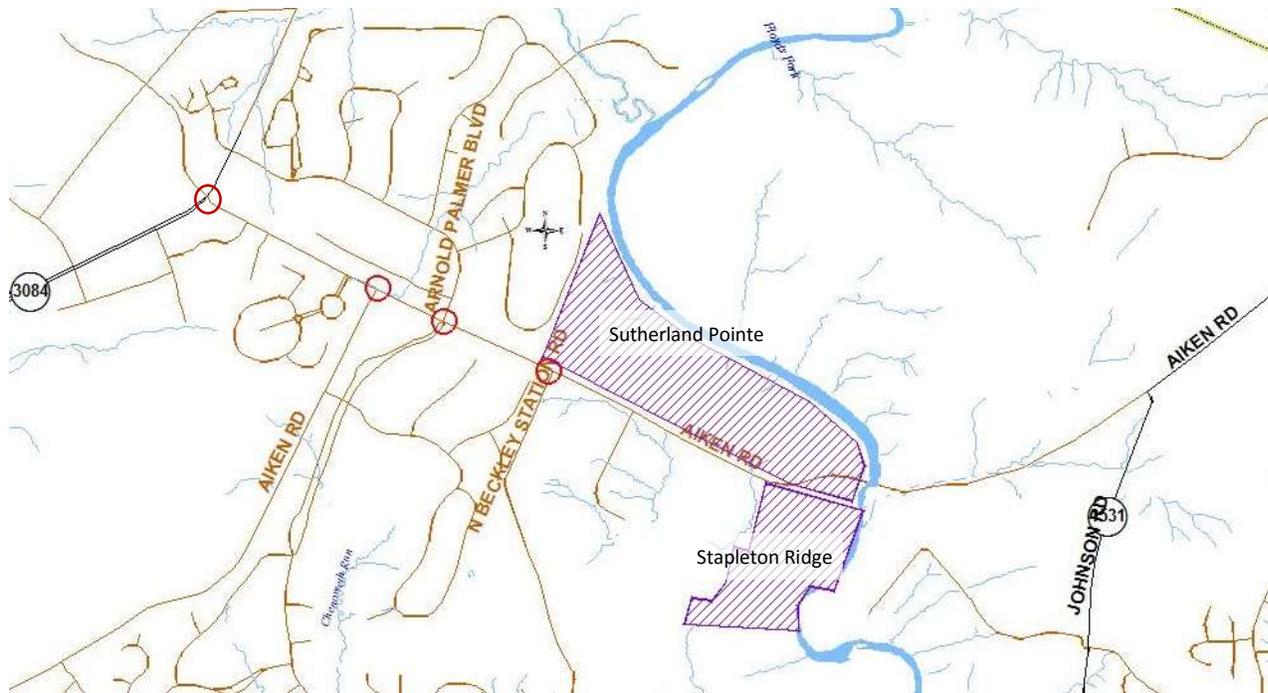


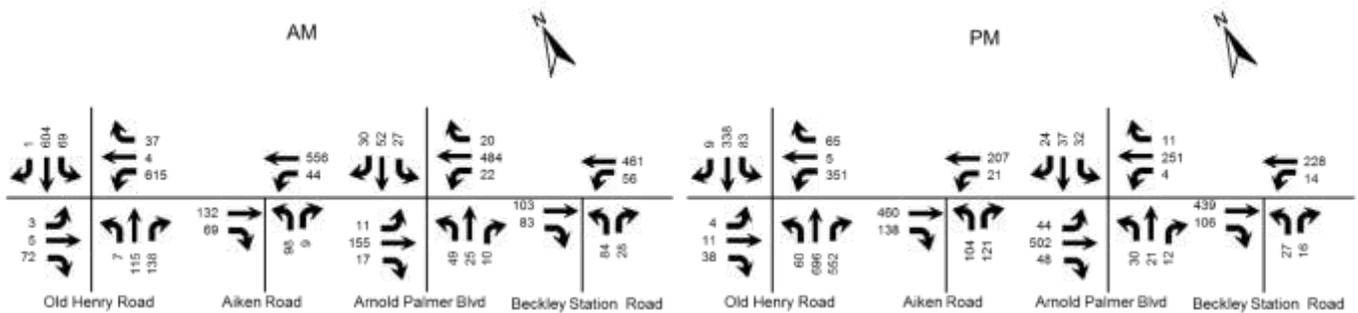
Figure 1. Site Map

## EXISTING CONDITIONS

Aiken Road is a Metro Louisville maintained road with an estimated 2015 Average Annual Daily Traffic (AADT) volume of 7,000 vehicles per day east of Beckley Station, as estimated from the turning movement count. The road is two lanes with ten-foot lanes and a one-foot shoulder. The speed limit is 35 mph. There are no sidewalks. The intersections are controlled with a stop sign.

Bush Farm Road is a Metro Louisville maintained road with an estimated 2015 ADT of 9,700 vehicles per day between Old Henry Road and Promenade Green Way, as estimated from the turning movement count. The road is a two-lane highway with 11 foot lanes. There are exclusive left turn lanes at the intersections with Old Henry Road, Promenade Green Way, and Bush Ridge Drive. The speed limit is 35 mph. There are sidewalks along the south side, east of Promenade Green Way.

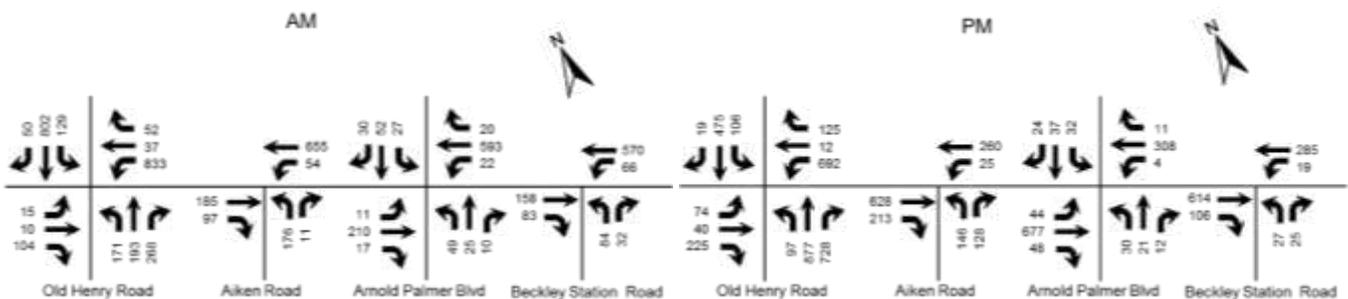
A.m. and p.m. peak hour turning movement counts for the intersection were made on various dates (see Appendix A). The a.m. peak occurred between 7:15 and 8:15 and the p.m. peak hour occurred between 4:45 and 5:45. **Figure 2** illustrates the 2015 peak hour traffic volumes.



**Figure 2. 2015 Peak Hour Volumes**

### FUTURE CONDITIONS

The build-out year for this project is 2021. To determine the traffic volumes in 2021, the traffic generated by the approved plans for Stapleton Ridge, the Bush Farm retirement community, and Old Henry Crossings was added to the 2015 volumes. Additionally, one percent annual growth was added to the through volumes on Old Henry Road. **Figure 3** displays the 2021 No Build volumes.



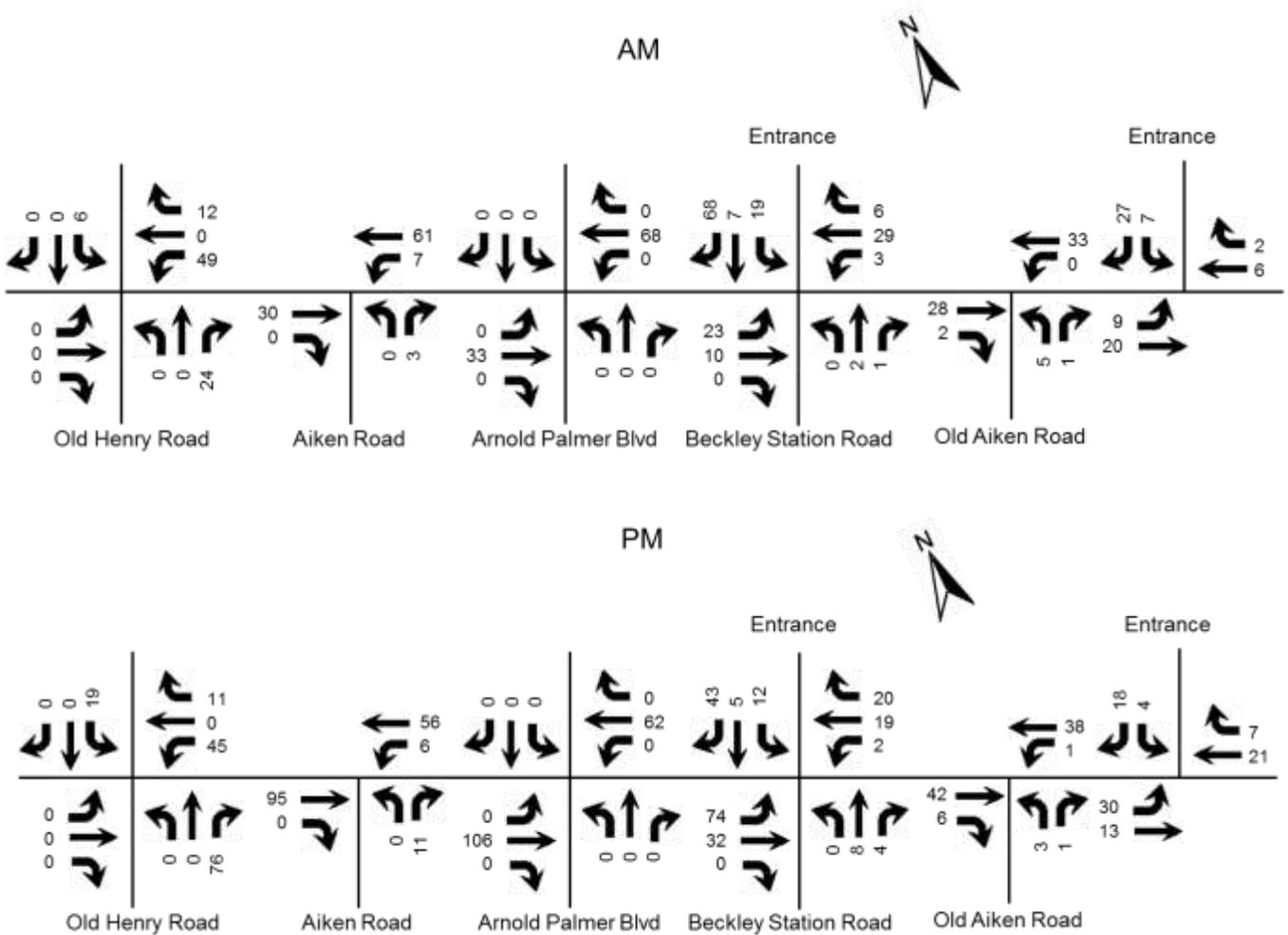
**Figure 3. 2021 Peak Hour No Build**

### TRIP GENERATION

The Institute of Transportation Engineers Trip Generation Manual, 9<sup>th</sup> Edition contains trip generation rates for a wide range of developments. The land use of “Single-Family Detached Housing” was reviewed and determined to be the best match. The trip generation results are listed in **Table 1**. The results of the trip generation analysis are that this development will generate 178 a.m. peak hour trips and 232 p.m. peak hour trips. The trips were assigned to the highway network with 80 percent to the west and 20 percent to the east. **Figure 4** shows the trips generated by this development and distributed throughout the road network for the year 2021 during the peak hours. **Figure 5** displays the individual turning movements for the year 2021 for the peak hours when the development is completed.

**Table 1. Peak Hour Trips Generated by Site**

Land Use	A.M. Peak Hour			P.M. Peak Hour		
	Trips	IN	OUT	Trips	IN	OUT
Single-Family Detached (210)	178	44	134	232	146	86



**Figure 4. Hour Trips Generated by Site**

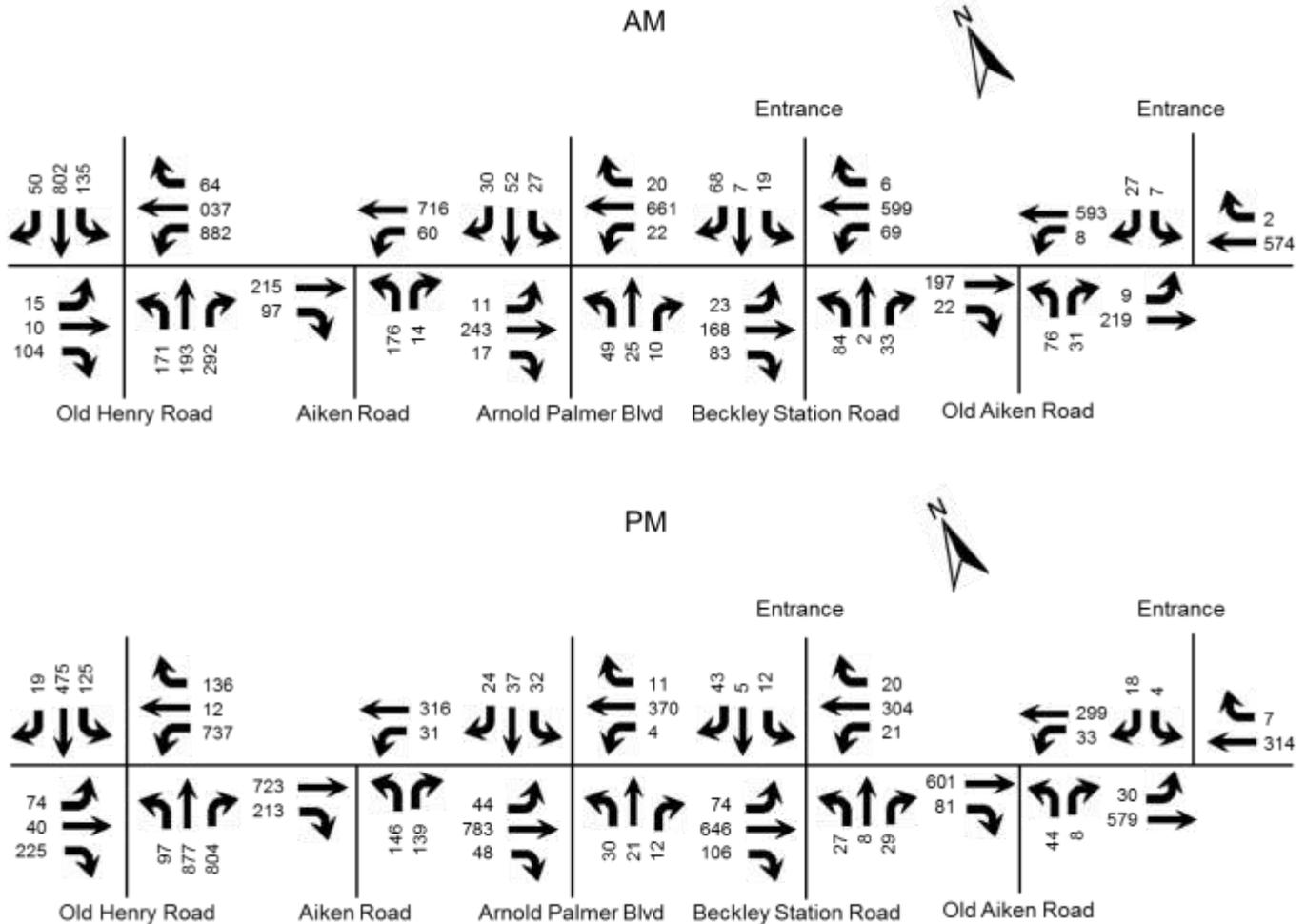


Figure 5. 2021 Peak Hour Build

## ANALYSIS

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a “Level of Service” or LOS. Level of Service is a ranking scale from A through F with each level representing a range. LOS results depend upon the type of facility that is analyzed. In this case, the LOS is based upon the average vehicle delay each movement experiences at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersection were determined using procedures detailed in the Highway Capacity Manual, 2010 edition. Average delay and Level of Service were determined for the intersection using HCS 2010 Streets and TWSC (version 6.7) software. **Table 2** shows the results of the analysis.

**Table 2. Peak Hour Level of Service**

Approach	A.M.			P.M.		
	2015 Existing	2021 No Build	2021 Build	2015 Existing	2021 No Build	2021 Build
<b>Aiken Road at Beckley Station Road</b>						
Aiken Road Eastbound	NA	NA	A 9.2	NA	NA	A 8.1
Aiken Road Westbound	A 7.8	A 8.0	A 8.0	A 8.6	A 9.3	A 9.4
Beckley Station Northbound	C 20.4	D 30.2	E 37.1	B 14.8	C 18.7	C 15.9
Sutherland Pointe Entrance Southbound	NA	NA	C 16.7	NA	NA	B 13.1
<b>Aiken Road at Arnold Palmer Boulevard</b>						
Aiken Road Eastbound	A 8.6	A 9.0	A 9.2	A 7.9	A 8.0	A 8.2
Aiken Road Westbound	A 7.6	A 7.8	A 7.9	A 8.2	A 9.2	A 9.6
Arnold Palmer Boulevard Northbound	D 25.3	E 38.5	F 53.8	C 24.9	E 40.5	F 65.2
Arnold Palmer Boulevard Southbound	C 21.8	D 30.4	E 39.1	C 24.0	E 39.2	F 64.0
<b>Aiken Road at Bush Farm Road</b>						
Aiken Road Westbound	A 7.8	A 8.0	A 8.1	A 8.9	A 9.9	B 10.5
Aiken Road Northbound	C 23.6	F 86.2	F 140.8	D 25.5	F 113.1	F 227.6
<b>Old Henry Road at Bush Farm Road</b>						
Bush Farm Road Eastbound	B 11.5	B 13.4	B 13.4	B 16.9	B 20.0	C 20.1
Bush Farm Road Westbound	D 47.6	F 151.8	F 179.8	C 24.5	F 316.7	F 357.6
Old Henry Road Northbound	B 18.5	F 88.8	F 87.3	B 17.8	F 93.9	F 114.4
Old Henry Road Southbound	C 20.8	D 37.8	D 37.8	B 14.2	F 83.6	F 117.6

Approach	A.M.			P.M.		
	2015 Existing	2021 No Build	2021 Build	2015 Existing	2021 No Build	2021 Build
<b>Aiken Road at Sutherland Pointe Entrance (East)</b>						
Aiken Road Westbound			A 9.0			A 8.0
Sutherland Pointe Entrance Southbound			C 15.1			B 11.8

*Key: Level of Service, Delay in seconds per vehicle*

The western entrance will align with Beckley Station Road. The intersection will include left turn lanes for eastbound and westbound traffic. The eastbound direction will also meet the warrants for right turn lane. Beckley Station Road northbound should be widened to include a left turn lane.

## CONCLUSIONS

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2021, there will be manageable impact to the existing highway network. The delays experienced will increase, but will continue to operate at an acceptable Level of Service.

The intersection of Aiken Road at Bush Farm Road will experience increases in delay in the No Build condition. The intersection should be monitored to determine if a roundabout or traffic signal could provide improvement to the operating condition.

Metro Public Works should continue to monitor the traffic patterns from Old Henry Road Crossing South and the impact on the Bush Farm Road intersections with Old Henry Road. If the projections from the Old Henry Road Crossing traffic study are confirmed, additional improvements are needed at the intersection.

## **APPENDIX**

Traffic Count

**Study Name Aiken Rd & Beckley Station Rd**

**Start Date 10/22/2015**

**Start Time 7:00 AM**

**Site Code**

Start Time	Southbound		Northbound		Eastbound		TOTAL
	Right	Thru	Thru	Left	Right	Left	
7:00 AM	5	13	103	4	0	3	128
7:15 AM	26	14	125	20	1	7	193
7:30 AM	51	17	111	31	11	25	246
7:45 AM	3	28	126	5	13	43	218
8:00 AM	3	44	99	0	3	9	158
8:15 AM	6	35	89	3	1	9	143
8:30 AM	5	31	118	1	0	17	172
8:45 AM	5	37	106	7	1	12	168
4:00 PM	24	101	42	5	10	26	208
4:15 PM	15	72	63	1	5	13	169
4:30 PM	18	84	48	2	3	10	165
4:45 PM	38	97	64	3	5	9	216
5:00 PM	22	104	52	2	7	5	192
5:15 PM	20	122	60	7	2	4	215
5:30 PM	26	116	52	2	2	9	207
5:45 PM	21	102	56	4	2	9	194

Start Time	Southbound		Northbound		Eastbound		TOTAL
	Right	Thru	Thru	Left	Right	Left	
7:15 AM	26	14	125	20	1	7	193
7:30 AM	51	17	111	31	11	25	246
7:45 AM	3	28	126	5	13	43	218
8:00 AM	3	44	99	0	3	9	158
<b>TOTAL</b>	<b>83</b>	<b>103</b>	<b>461</b>	<b>56</b>	<b>28</b>	<b>84</b>	<b>815</b>
4:45 PM	38	97	64	3	5	9	216
5:00 PM	22	104	52	2	7	5	192
5:15 PM	20	122	60	7	2	4	215
5:30 PM	26	116	52	2	2	9	207
<b>TOTAL</b>	<b>106</b>	<b>439</b>	<b>228</b>	<b>14</b>	<b>16</b>	<b>27</b>	<b>830</b>

**Study Name Aiken Rd & Arnold Palmer Dr**

**Start Date 10/22/2015**

**Start Time 7:00 AM**

**Site Code**

Start Time	Southbound Approach Southbound			Westbound Approach Westbound			Northbound Approach Northbound			Eastbound Approach Eastbound			TOTAL
	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	
7:00 AM	3	15	3	6	5	1	1	117	6	2	1	2	162
7:15 AM	1	43	3	8	16	16	4	117	6	6	5	15	240
7:30 AM	5	37	3	7	12	5	10	129	6	1	3	11	229
7:45 AM	4	35	3	3	21	4	6	141	9	2	10	10	248
8:00 AM	7	40	2	12	3	2	0	97	1	1	7	13	185
8:15 AM	8	36	4	8	2	2	2	101	1	2	6	8	180
8:30 AM	3	29	6	8	6	2	6	121	11	4	8	13	217
8:45 AM	4	40	3	6	11	2	1	91	11	5	8	22	204
4:00 PM	7	103	8	4	11	5	6	65	2	5	7	5	228
4:15 PM	6	88	6	3	16	2	5	63	4	2	3	4	202
4:30 PM	18	103	9	4	10	5	3	61	6	3	4	6	232
4:45 PM	14	120	14	16	11	8	1	51	4	5	7	4	255
5:00 PM	9	130	12	10	11	3	1	62	0	1	6	6	251
5:15 PM	15	135	14	11	8	3	2	63	1	1	5	7	265
5:30 PM	7	125	11	7	8	12	4	53	1	3	7	5	243
5:45 PM	17	112	7	4	10	6	4	73	2	7	3	12	257

Start Time	Southbound Approach Southbound			Westbound Approach Westbound			Northbound Approach Northbound			Eastbound Approach Eastbound			TOTAL
	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	
7:15 AM	1	43	3	8	16	16	4	117	6	6	5	15	240
7:30 AM	5	37	3	7	12	5	10	129	6	1	3	11	229
7:45 AM	4	35	3	3	21	4	6	141	9	2	10	10	248
8:00 AM	7	40	2	12	3	2	0	97	1	1	7	13	185
<b>TOTAL</b>	<b>17</b>	<b>155</b>	<b>11</b>	<b>30</b>	<b>52</b>	<b>27</b>	<b>20</b>	<b>484</b>	<b>22</b>	<b>10</b>	<b>25</b>	<b>49</b>	<b>902</b>
5:00 PM	9	130	12	10	11	3	1	62	0	1	6	6	251
5:15 PM	15	135	14	11	8	3	2	63	1	1	5	7	265
5:30 PM	7	125	11	7	8	12	4	53	1	3	7	5	243
5:45 PM	17	112	7	4	10	6	4	73	2	7	3	12	257
<b>TOTAL</b>	<b>48</b>	<b>502</b>	<b>44</b>	<b>32</b>	<b>37</b>	<b>24</b>	<b>11</b>	<b>251</b>	<b>4</b>	<b>12</b>	<b>21</b>	<b>30</b>	<b>1016</b>

**Study Name Bush Farm Rd & Aiken Rd**

**Start Date 10/27/2015**

**Start Time 7:00 AM**

**Site Code**

Start Time	Southbound Approach Southbound		Northbound Approach Northbound		Eastbound Approach Eastbound		TOTAL
	Right	Thru	Thru	Left	Right	Left	
7:00 AM	19	17	146	5	3	20	210
7:15 AM	14	46	108	11	3	20	202
7:30 AM	14	35	159	14	1	31	254
7:45 AM	22	34	143	14	2	27	242
8:00 AM	27	26	105	16	6	24	204
8:15 AM	30	15	119	13	5	17	199
8:30 AM	50	16	114	23	8	24	235
8:45 AM	72	33	82	27	13	59	286
4:00 PM	19	84	42	5	29	55	234
4:15 PM	22	92	45	3	17	28	207
4:30 PM	29	104	51	4	22	24	234
4:45 PM	45	126	47	8	30	27	283
5:00 PM	31	108	46	2	26	28	241
5:15 PM	30	108	49	4	29	32	252
5:30 PM	32	118	65	7	36	17	275
5:45 PM	20	106	64	5	44	24	263

Start Time	Southbound Approach Southbound		Northbound Approach Northbound		Eastbound Approach Eastbound		TOTAL
	Right	Thru	Thru	Left	Right	Left	
7:00 AM	19	17	146	5	3	20	210
7:15 AM	14	46	108	11	3	20	202
7:30 AM	14	35	159	14	1	31	254
7:45 AM	22	34	143	14	2	27	242
<b>TOTAL</b>	<b>69</b>	<b>132</b>	<b>556</b>	<b>44</b>	<b>9</b>	<b>98</b>	<b>908</b>
4:45 PM	45	126	47	8	30	27	283
5:00 PM	31	108	46	2	26	28	241
5:15 PM	30	108	49	4	29	32	252
5:30 PM	32	118	65	7	36	17	275
<b>TOTAL</b>	<b>138</b>	<b>460</b>	<b>207</b>	<b>21</b>	<b>121</b>	<b>104</b>	<b>1051</b>

11940 Highway 42, Suite 1  
Goshen, KY 40026

Counted by: Andy Wolak

File Name : OldHenryBushFarmAM  
Site Code : 00062151  
Start Date : 6/2/2015  
Page No : 1

Groups Printed- Unshifted

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	10	142	0	152	121	0	5	126	2	16	16	34	0	1	10	11	323
07:15 AM	8	169	1	178	119	0	9	128	2	12	36	50	0	0	9	9	365
07:30 AM	8	150	0	158	155	0	12	167	2	33	34	69	0	3	19	22	416
07:45 AM	14	158	0	172	161	1	9	171	1	20	42	63	2	1	14	17	423
<b>Total</b>	<b>40</b>	<b>619</b>	<b>1</b>	<b>660</b>	<b>566</b>	<b>1</b>	<b>35</b>	<b>592</b>	<b>7</b>	<b>81</b>	<b>128</b>	<b>216</b>	<b>2</b>	<b>5</b>	<b>52</b>	<b>69</b>	<b>1527</b>
08:00 AM	24	147	1	172	153	2	11	166	2	34	29	65	1	1	16	18	421
08:15 AM	23	149	0	172	146	1	5	152	2	28	33	63	0	0	23	23	410
08:30 AM	14	139	0	153	127	0	7	134	3	25	62	90	0	2	12	14	391
08:45 AM	46	126	2	174	149	1	13	163	4	31	74	109	1	3	11	15	461
<b>Total</b>	<b>107</b>	<b>561</b>	<b>3</b>	<b>671</b>	<b>575</b>	<b>4</b>	<b>36</b>	<b>615</b>	<b>11</b>	<b>118</b>	<b>198</b>	<b>327</b>	<b>2</b>	<b>6</b>	<b>62</b>	<b>70</b>	<b>1683</b>
<b>Grand Total</b>	<b>147</b>	<b>1180</b>	<b>4</b>	<b>1331</b>	<b>1131</b>	<b>5</b>	<b>71</b>	<b>1207</b>	<b>18</b>	<b>199</b>	<b>326</b>	<b>543</b>	<b>4</b>	<b>11</b>	<b>114</b>	<b>129</b>	<b>3210</b>
Apprch %	11	88.7	0.3		93.7	0.4	5.9		3.3	36.6	60		3.1	8.5	88.4		
Total %	4.6	36.8	0.1	41.5	35.2	0.2	2.2	37.6	0.6	6.2	10.2	16.9	0.1	0.3	3.6	4	

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 08:00 AM																	
08:00 AM	24	147	1	172	153	2	11	166	2	34	29	65	1	1	16	18	421
08:15 AM	23	149	0	172	146	1	5	152	2	28	33	63	0	0	23	23	410
08:30 AM	14	139	0	153	127	0	7	134	3	25	62	90	0	2	12	14	391
08:45 AM	46	126	2	174	149	1	13	163	4	31	74	109	1	3	11	15	461
<b>Total Volume</b>	<b>107</b>	<b>561</b>	<b>3</b>	<b>671</b>	<b>575</b>	<b>4</b>	<b>36</b>	<b>615</b>	<b>11</b>	<b>118</b>	<b>198</b>	<b>327</b>	<b>2</b>	<b>6</b>	<b>62</b>	<b>70</b>	<b>1683</b>

Counted by: Andy Wolak

File Name : OldHenryBushFarmPM  
Site Code : 00062152  
Start Date : 6/2/2015  
Page No : 1

Groups Printed- Unshifted

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	19	57	1	77	97	4	21	122	13	99	105	217	0	1	4	5	421
04:15 PM	12	65	1	78	89	3	11	103	5	93	108	206	0	0	7	7	394
04:30 PM	20	65	1	86	67	0	15	82	15	126	99	240	0	1	7	8	416
04:45 PM	32	83	2	117	78	0	15	93	10	134	123	267	2	0	9	11	488
<b>Total</b>	<b>83</b>	<b>270</b>	<b>5</b>	<b>358</b>	<b>331</b>	<b>7</b>	<b>62</b>	<b>400</b>	<b>43</b>	<b>452</b>	<b>435</b>	<b>930</b>	<b>2</b>	<b>2</b>	<b>27</b>	<b>31</b>	<b>1719</b>
05:00 PM	27	91	2	120	107	1	11	119	25	175	145	345	0	3	6	9	593
05:15 PM	16	86	3	105	81	1	31	113	15	185	141	341	1	3	8	12	571
05:30 PM	18	81	2	101	77	1	12	90	13	175	131	319	2	3	12	17	527
05:45 PM	22	80	2	104	86	2	11	99	7	161	135	303	1	2	12	15	521
<b>Total</b>	<b>83</b>	<b>338</b>	<b>9</b>	<b>430</b>	<b>351</b>	<b>5</b>	<b>65</b>	<b>421</b>	<b>60</b>	<b>696</b>	<b>552</b>	<b>1308</b>	<b>4</b>	<b>11</b>	<b>38</b>	<b>53</b>	<b>2212</b>
<b>Grand Total</b>	<b>166</b>	<b>608</b>	<b>14</b>	<b>788</b>	<b>682</b>	<b>12</b>	<b>127</b>	<b>821</b>	<b>103</b>	<b>1148</b>	<b>987</b>	<b>2238</b>	<b>6</b>	<b>13</b>	<b>65</b>	<b>84</b>	<b>3931</b>
Apprch %	21.1	77.2	1.8		83.1	1.5	15.5		4.6	51.3	44.1		7.1	15.5	77.4		
Total %	4.2	15.5	0.4	20	17.3	0.3	3.2	20.9	2.6	29.2	25.1	56.9	0.2	0.3	1.7	2.1	

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	27	91	2	120	107	1	11	119	25	175	145	345	0	3	6	9	593
05:15 PM	16	86	3	105	81	1	31	113	15	185	141	341	1	3	8	12	571
05:30 PM	18	81	2	101	77	1	12	90	13	175	131	319	2	3	12	17	527
05:45 PM	22	80	2	104	86	2	11	99	7	161	135	303	1	2	12	15	521
<b>Total Volume</b>	<b>83</b>	<b>338</b>	<b>9</b>	<b>430</b>	<b>351</b>	<b>5</b>	<b>65</b>	<b>421</b>	<b>60</b>	<b>696</b>	<b>552</b>	<b>1308</b>	<b>4</b>	<b>11</b>	<b>38</b>	<b>53</b>	<b>2212</b>
% App. Total	19.3	78.6	2.1		83.4	1.2	15.4		4.6	53.2	42.2		7.5	20.8	71.7		
PHF	769	929	750	896	820	625	524	884	600	941	952	948	500	917	792	779	933

HCS Reports

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Beckley							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2015							North/South Street	Beckley Stations							
Time Analyzed	AM Peak							Peak Hour Factor	0.83							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			103	83		56	461			84		28				
Percent Heavy Vehicles						1				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						622					135					
Capacity						1350					367					
v/c Ratio						0.46					0.37					
95% Queue Length						0.2					1.7					
Control Delay (s/veh)						7.8					20.4					
Level of Service (LOS)						A					C					
Approach Delay (s/veh)					1.3				20.4							
Approach LOS					A				C							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Beckley							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2021							North/South Street	Beckley Stations							
Time Analyzed	AM Peak No Build							Peak Hour Factor	0.83							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			158	83		66	570			84		32				
Percent Heavy Vehicles						1				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						767					140					
Capacity						1277					279					
v/c Ratio						0.60					0.50					
95% Queue Length						0.2					2.6					
Control Delay (s/veh)						8.0					30.2					
Level of Service (LOS)						A					D					
Approach Delay (s/veh)					1.6				30.2							

HCS 2010 Two-Way Stop Control Summary Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Aiken at Beckley Station								
Agency/Co.	DBZ							Jurisdiction									
Date Performed	2/28/2016							East/West Street	Aiken Road								
Analysis Year	2021							North/South Street	Beckley Station/Entrance								
Time Analyzed	AM Peak Build							Peak Hour Factor	0.83								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Sutherland Pointe																
Lanes																	
<p>Major Street East-West</p>																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6			7	8	9		10	11	12
Number of Lanes	0	1	1	1	0	1	1	0			1	1	0		0	1	1
Configuration		L	T	R		L		TR			L		TR		LT		R
Volume (veh/h)		23	168	83		69	599	6			84	2	33		19	7	68
Percent Heavy Vehicles		1				1					1	3	1		1	1	1
Proportion Time Blocked																	
Right Turn Channelized	No				No				No				No				
Median Type	Left Only																
Median Storage	1																
Delay, Queue Length, and Level of Service																	
Flow Rate (veh/h)		28				83					101		42		31		82
Capacity		879				1264					178		882		332		426
v/c Ratio		0.03				0.07					0.57		0.05		0.09		0.19
95% Queue Length		0.1				0.2					3.0		0.1		0.3		0.7
Control Delay (s/veh)		9.2				8.0					48.6		9.3		17.0		15.4
Level of Service (LOS)		A				A					E		A		C		C
Approach Delay (s/veh)	0.8				0.8				37.1				16.7				
Approach LOS	A				A				E				C				

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Beckley							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2015							North/South Street	Beckley Stations							
Time Analyzed	PM Peak							Peak Hour Factor	0.96							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR			LT					LR				
Volume (veh/h)			439	106			14	228			27		16			
Percent Heavy Vehicles							1				3		3			
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)							253						45			
Capacity							1010						414			
v/c Ratio							0.25						0.11			
95% Queue Length							0.0						0.4			
Control Delay (s/veh)							8.6						14.8			
Level of Service (LOS)							A						B			
Approach Delay (s/veh)					0.6				14.8							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Beckley							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2021							North/South Street	Beckley Stations							
Time Analyzed	PM Peak No Build							Peak Hour Factor	0.96							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR			LT				LR					
Volume (veh/h)			614	106			19	285			27		25			
Percent Heavy Vehicles							1				3		3			
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)							317						54			
Capacity							864						316			
v/c Ratio							0.37						0.17			
95% Queue Length							0.1						0.6			
Control Delay (s/veh)							9.3						18.7			
Level of Service (LOS)							A						C			
Approach Delay (s/veh)					0.8				18.7							
Approach LOS					A				C							

HCS 2010 Two-Way Stop Control Summary Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Aiken at Beckley Station								
Agency/Co.	DBZ							Jurisdiction									
Date Performed	2/28/2016							East/West Street	Aiken Road								
Analysis Year	2021							North/South Street	Beckley Station/Entrance								
Time Analyzed	PM Peak Build							Peak Hour Factor	0.96								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Sutherland Pointe																
Lanes																	
<p>Major Street: East-West</p>																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6			7	8	9		10	11	12
Number of Lanes	0	1	1	1	0	1	1	0			1	1	0		0	1	1
Configuration		L	T	R		L		TR			L		TR		LT		R
Volume (veh/h)		74	646	106		21	304	20			27	8	29		12	5	43
Percent Heavy Vehicles		1				1					1	3	1		1	1	1
Proportion Time Blocked																	
Right Turn Channelized	No				No				No				No				
Median Type	Left Only																
Median Storage	1																
Delay, Queue Length, and Level of Service																	
Flow Rate (veh/h)		77				22					28		38		17		45
Capacity		1226				839					244		576		321		716
v/c Ratio		0.06				0.03					0.11		0.07		0.05		0.06
95% Queue Length		0.2				0.1					0.4		0.2		0.2		0.2
Control Delay (s/veh)		8.1				9.4					21.7		11.7		16.8		10.4
Level of Service (LOS)		A				A					C		B		C		B
Approach Delay (s/veh)	0.7				0.6				15.9				13.1				
Approach LOS	A				A				C				B				

HCS 2010 Two-Way Stop Control Summary Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	Aiken at Arnold Palmer									
Agency/Co.	DBZ							Jurisdiction										
Date Performed	2/28/2016							East/West Street	Aiken Road									
Analysis Year	2015							North/South Street	Arnold Palmer Boulevard									
Time Analyzed	AM Peak							Peak Hour Factor	0.91									
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25									
Project Description	Sutherland Pointe																	
Lanes																		
<p style="text-align: center;">Major Street East-West</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0		
Configuration			LTR				LTR				LTR				LTR			
Volume (veh/h)		11	155	17		22	484	20		49	25	10		27	52	30		
Percent Heavy Vehicles		1				1				1	1	1		1	1	1		
Proportion Time Blocked																		
Right Turn Channelized	No				No				No				No					
Median Type	Undivided																	
Median Storage																		
Delay, Queue Length, and Level of Service																		
Flow Rate (veh/h)		12				24					92				120			
Capacity		1021				1391					268				333			
v/c Ratio		0.01				0.02					0.34				0.36			
95% Queue Length		0.0				0.1					1.5				1.6			
Control Delay (s/veh)		8.6				7.6					25.3				21.8			
Level of Service (LOS)		A				A					D				C			
Approach Delay (s/veh)		0.6				0.5					25.3				21.8			
Approach LOS		A				A					D				C			

HCS 2010 Two-Way Stop Control Summary Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Aiken at Arnold Palmer								
Agency/Co.	DBZ							Jurisdiction									
Date Performed	2/28/2016							East/West Street	Aiken Road								
Analysis Year	2021							North/South Street	Arnold Palmer Boulevard								
Time Analyzed	AM Peak No Build							Peak Hour Factor	0.91								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Sutherland Pointe																
Lanes																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0	
Configuration			LTR				LTR				LTR				LTR		
Volume (veh/h)		11	210	17		22	593	20		49	25	10		27	52	30	
Percent Heavy Vehicles		1				1				1	1	1		1	1	1	
Proportion Time Blocked																	
Right Turn Channelized	No				No				No				No				
Median Type	Undivided																
Median Storage																	
Delay, Queue Length, and Level of Service																	
Flow Rate (veh/h)		12				24					92					120	
Capacity		922				1321					196					259	
v/c Ratio		0.01				0.02					0.47					0.46	
95% Queue Length		0.0				0.1					2.3					2.3	
Control Delay (s/veh)		9.0				7.8					38.5					30.4	
Level of Service (LOS)		A				A					E					D	
Approach Delay (s/veh)		0.5				0.5				38.5				30.4			
Approach LOS		A				A				E				D			

HCS 2010 Two-Way Stop Control Summary Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	Aiken at Arnold Palmer									
Agency/Co.	DBZ							Jurisdiction										
Date Performed	2/28/2016							East/West Street	Aiken Road									
Analysis Year	2021							North/South Street	Arnold Palmer Boulevard									
Time Analyzed	AM Peak Build							Peak Hour Factor	0.91									
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25									
Project Description	Sutherland Pointe																	
Lanes																		
<p>Major Street: East-West</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority	1U	1	2	3	4U	4	5	6			7	8	9			10	11	12
Number of Lanes	0	0	1	0	0	0	1	0			0	1	0			0	1	0
Configuration			LTR				LTR				LTR				LTR			
Volume (veh/h)		11	243	17		22	661	20			49	25	10			27	52	30
Percent Heavy Vehicles		1				1					1	1	1			1	1	1
Proportion Time Blocked																		
Right Turn Channelized	No				No				No				No					
Median Type	Undivided																	
Median Storage																		
Delay, Queue Length, and Level of Service																		
Flow Rate (veh/h)		12				24					92					120		
Capacity		865				1282					161					221		
v/c Ratio		0.01				0.02					0.57					0.54		
95% Queue Length		0.0				0.1					3.0					2.9		
Control Delay (s/veh)		9.2				7.9					53.8					39.1		
Level of Service (LOS)		A				A					F					E		
Approach Delay (s/veh)	0.5				0.5				53.8				39.1					
Approach LOS	A				A				F				E					

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Arnold Palmer							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2015							North/South Street	Arnold Palmer Boulevard							
Time Analyzed	PM Peak Build							Peak Hour Factor	0.96							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		44	502	48		4	251	11		30	21	12		32	37	24
Percent Heavy Vehicles		1				1				1	1	1		1	1	1
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)		46				4					65				97	
Capacity		1297				1005					245				285	
v/c Ratio		0.04				0.00					0.26				0.34	
95% Queue Length		0.1				0.0					1.0				1.5	
Control Delay (s/veh)		7.9				8.6					24.9				24.0	
Level of Service (LOS)		A				A					C				C	
Approach Delay (s/veh)	1.0				0.2				24.9				24.0			

HCS 2010 Two-Way Stop Control Summary Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	Aiken at Arnold Palmer									
Agency/Co.	DBZ							Jurisdiction										
Date Performed	2/28/2016							East/West Street	Aiken Road									
Analysis Year	2021							North/South Street	Arnold Palmer Boulevard									
Time Analyzed	PM Peak Build No Build							Peak Hour Factor	0.96									
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25									
Project Description	Sutherland Pointe																	
Lanes																		
<p>Major Street East-West</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0		
Configuration			LTR				LTR				LTR				LTR			
Volume (veh/h)		44	677	48		4	308	11		30	21	12		32	37	24		
Percent Heavy Vehicles		1				1				1	1	1		1	1	1		
Proportion Time Blocked																		
Right Turn Channelized	No				No				No				No					
Median Type	Undivided																	
Median Storage																		
Delay, Queue Length, and Level of Service																		
Flow Rate (veh/h)		46				4					65					97		
Capacity		1233				860					165					199		
v/c Ratio		0.04				0.00					0.40					0.49		
95% Queue Length		0.1				0.0					1.7					2.4		
Control Delay (s/veh)		8.0				9.2					40.5					39.2		
Level of Service (LOS)		A				A					E					E		
Approach Delay (s/veh)		1.0				0.2					40.5				39.2			
Approach LOS		A				A					E				E			

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Arnold Palmer							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2021							North/South Street	Arnold Palmer Boulevard							
Time Analyzed	PM Peak Build Build							Peak Hour Factor	0.96							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	1	0	
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		44	783	48		4	370	11		30	21	12		32	37	24
Percent Heavy Vehicles		1				1				1	1	1		1	1	1
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)		46				4				65						97
Capacity		1168				782				121						151
v/c Ratio		0.04				0.01				0.54						0.64
95% Queue Length		0.1				0.0				2.6						3.5
Control Delay (s/veh)		8.2				9.6				65.2						64.0
Level of Service (LOS)		A				A				F						F
Approach Delay (s/veh)	1.0				0.2				65.2				64.0			
Approach LOS	A				A				F				F			

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Bush Farm							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road							
Analysis Year	2015							North/South Street	Aiken Road							
Time Analyzed	AM Peak							Peak Hour Factor	0.89							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p style="text-align: center;">Major Street East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			132	69		44	556			98		9				
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						674					120					
Capacity						1348					312					
v/c Ratio						0.50					0.38					
95% Queue Length						0.1					1.7					
Control Delay (s/veh)						7.8					23.6					
Level of Service (LOS)						A					C					
Approach Delay (s/veh)					1.0				23.6							
Approach LOS					A				C							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Bush Farm							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road							
Analysis Year	2021							North/South Street	Aiken Road							
Time Analyzed	AM Peak No Build							Peak Hour Factor	0.89							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p style="text-align: center;">Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			185	97		54	655			176		11				
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						797						210				
Capacity						1248						228				
v/c Ratio						0.64						0.92				
95% Queue Length						0.2						7.8				
Control Delay (s/veh)						8.0						86.2				
Level of Service (LOS)						A						F				
Approach Delay (s/veh)					1.2				86.2							
Approach LOS					A				F							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Bush Farm							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road							
Analysis Year	2021							North/South Street	Aiken Road							
Time Analyzed	AM Peak Build							Peak Hour Factor	0.89							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p style="text-align: center;">Major Street East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			215	97		60	716			176		14				
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						871						214				
Capacity						1213						196				
v/c Ratio						0.72						1.09				
95% Queue Length						0.2						10.1				
Control Delay (s/veh)						8.1						140.8				
Level of Service (LOS)						A						F				
Approach Delay (s/veh)					1.4				140.8							
Approach LOS					A				F							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Bush Farm							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road							
Analysis Year	2015							North/South Street	Aiken Road							
Time Analyzed	PM Peak							Peak Hour Factor	0.93							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR			LT					LR				
Volume (veh/h)			460	138			21	207			104		121			
Percent Heavy Vehicles							1				1		1			
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)							246						242			
Capacity							946						411			
v/c Ratio							0.26						0.59			
95% Queue Length							0.1						3.7			
Control Delay (s/veh)							8.9						25.5			
Level of Service (LOS)							A						D			
Approach Delay (s/veh)					1.1				25.5							

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Bush Farm							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road							
Analysis Year	2021							North/South Street	Aiken Road							
Time Analyzed	PM Peak No Build							Peak Hour Factor	0.93							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			628	213		25	260			146		128				
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)						307						295				
Capacity						756						277				
v/c Ratio						0.41						1.07				
95% Queue Length						0.1						11.7				
Control Delay (s/veh)						9.9						113.1				
Level of Service (LOS)						A						F				
Approach Delay (s/veh)					1.3				113.1							
Approach LOS					A				F							

HCS 2010 Two-Way Stop Control Summary Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Aiken at Bush Farm								
Agency/Co.	DBZ							Jurisdiction									
Date Performed	2/28/2016							East/West Street	Aiken Road/Bush Farm Road								
Analysis Year	2021							North/South Street	Aiken Road								
Time Analyzed	PM Peak Build							Peak Hour Factor	0.93								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Sutherland Pointe																
Lanes																	
<p>Major Street: East-West</p>																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0	
Configuration				TR		LT					LR						
Volume (veh/h)			723	213		31	316			146		139					
Percent Heavy Vehicles						1				1		1					
Proportion Time Blocked																	
Right Turn Channelized	No				No				No				No				
Median Type	Undivided																
Median Storage																	
Delay, Queue Length, and Level of Service																	
Flow Rate (veh/h)						373						306					
Capacity						693						226					
v/c Ratio						0.54						1.35					
95% Queue Length						0.1						16.8					
Control Delay (s/veh)						10.5						227.6					
Level of Service (LOS)						B						F					
Approach Delay (s/veh)					1.5				227.6								
Approach LOS					A				F								

### HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information																										
Agency	DBZ			Duration, h	0.25																									
Analyst	DBZ			Analysis Date	Feb 28, 2016																									
Jurisdiction				Time Period	AM Peak																									
Urban Street	Old Henry Road			Analysis Year	2015																									
Intersection	Bush Farm Road			File Name	AM 15.xus																									
Project Description	Sutherland Pointe																													
Demand Information				EB			WB				NB																		SB	
Approach Movement				L	T	R	L				T	R	L	T	R	L													T	R
Demand (v), veh/h				3	5	72	615				4	37	7	115	138	69	604	1												
Signal Information																														
Cycle, s	82.2	Reference Phase	2																											
Offset, s	0	Reference Point	End																											
Uncoordinated	Yes	Simult. Gap E/W	Off	Green	30.0	40.0	0.0	0.0	0.0	0.0	0.0	0.0																		
Force Mode	Fixed	Simult. Gap N/S	Off	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0																		
				Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0																		
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT					
Assigned Phase							4						8						2						6					
Case Number							6.0						6.0						5.0						6.0					
Phase Duration, s							46.6						46.6						35.6						35.6					
Change Period, (Y+R <sub>c</sub> ), s							6.6						6.6						5.6						5.6					
Max Allow Headway (MAH), s							4.8						4.7						5.3						5.1					
Queue Clearance Time (g <sub>s</sub> ), s							4.3						42.0						13.7						13.1					
Green Extension Time (g <sub>e</sub> ), s							0.4						0.0						1.7						4.7					
Phase Call Probability							1.00						1.00						1.00						1.00					
Max Out Probability							0.00						1.00						0.00						0.01					
Movement Group Results				EB			WB			NB			SB																	
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R															
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	11															
Adjusted Flow Rate (v), veh/h				3	85		676	45		8	126	152	76	333	33															
Adjusted Saturation Flow Rate (s), veh/h/in				1383	1626		1334	1634		783	1900	1610	1284	1900	181															
Queue Service Time (g <sub>s</sub> ), s				0.1	2.3		37.7	1.2		0.6	3.7	5.4	3.5	11.1	11															
Cycle Queue Clearance Time (g <sub>c</sub> ), s				1.3	2.3		40.0	1.2		11.7	3.7	5.4	7.2	11.1	11															
Green Ratio (g/C)				0.49	0.49		0.49	0.49		0.36	0.36	0.36	0.36	0.36	0.3															
Capacity (c), veh/h				740	791		699	795		268	693	588	498	893	69															
Volume-to-Capacity Ratio (X)				0.004	0.107		0.967	0.057		0.029	0.182	0.258	0.152	0.480	0.4															
Available Capacity (c <sub>a</sub> ), veh/h				740	791		699	795		411	1040	881	733	1040	10															
Back of Queue (Q), veh/in (95 th percentile)				0.1	1.4		24.7	0.7		0.2	2.8	3.5	1.9	8.3	8															
Queue Storage Ratio (RQ) (95 th percentile)				0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.0															
Uniform Delay (d <sub>1</sub> ), s/veh				11.5	11.4		24.1	11.1		24.6	17.8	18.3	20.2	20.1	20															
Incremental Delay (d <sub>2</sub> ), s/veh				0.0	0.1		26.0	0.0		0.1	0.2	0.3	0.2	0.7	0															
Initial Queue Delay (d <sub>3</sub> ), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0															
Control Delay (d), s/veh				11.5	11.5		50.0	11.2		24.7	17.9	18.6	20.4	20.8	20															
Level of Service (LOS)				B	B		D	B		C	B	B	C	C	C															
Approach Delay, s/veh / LOS				11.5		B	47.6		D	18.5		B	20.8		C															
Intersection Delay, s/veh / LOS				30.5						C																				
Multimodal Results				EB			WB			NB			SB																	
Pedestrian LOS Score / LOS				2.8		C	2.4		B	2.3		B	2.3		B															
Bicycle LOS Score / LOS				0.6		A	1.7		A	1.0		A	1.1		A															

### HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency				Duration, h	0.25		
Analyst	DBZ	Analysis Date	Feb 28, 2016	Area Type	Other		
Jurisdiction				Time Period	AM Peak		
Urban Street	Old Henry Road	Analysis Year	2021 No Build	PHF	0.91		
Intersection	Bush Farm Road	File Name	AM 21 NB.xus	Analysis Period	1> 7:30		
Project Description	Sutherland Pointe						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h	15	10	104	833	37	52	171	193	268	129	802	5

Signal Information				Signal Timing (s)													
Cycle, s	120.2	Reference Phase	2	Green	42.0	66.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	Yes	Simult. Gap E/W	Off	Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	Off														

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		6.0		6.0		5.0		6.0
Phase Duration, s		72.6		72.6		47.8		47.6
Change Period, ( Y+R <sub>c</sub> ), s		6.6		6.6		5.6		5.8
Max Allow Headway ( MAH ), s		4.8		4.8		6.1		5.1
Queue Clearance Time ( g <sub>s</sub> ), s		6.5		68.0		44.0		27.9
Green Extension Time ( g <sub>e</sub> ), s		0.7		0.0		0.0		5.7
Phase Call Probability		1.00		1.00		1.00		1.00
Max Out Probability		0.00		1.00		1.00		0.39

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	11
Adjusted Flow Rate ( v ), veh/h	16	125		915	98		188	212	295	142	473	46
Adjusted Saturation Flow Rate ( s ), veh/h/ln	1318	1632		1286	1719		808	1900	1610	1188	1900	184
Queue Service Time ( g <sub>s</sub> ), s	0.7	4.5		61.5	3.3		16.1	9.8	17.5	11.9	25.9	25
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	4.0	4.5		66.0	3.3		42.0	9.8	17.5	21.8	25.9	25
Green Ratio ( g/C )	0.55	0.55		0.55	0.55		0.35	0.35	0.35	0.35	0.35	0.3
Capacity ( c ), veh/h	748	896		718	944		141	664	563	378	664	65
Volume-to-Capacity Ratio ( X )	0.022	0.140		1.276	0.104		1.331	0.319	0.523	0.375	0.713	0.7
Available Capacity ( c <sub>a</sub> ), veh/h	748	896		718	944		141	664	563	378	664	65
Back of Queue ( Q ), veh/ln ( 95 th percentile)	0.4	3.0		71.1	2.3		20.0	8.0	11.2	6.4	18.2	17
Queue Storage Ratio ( RQ ) ( 95 th percentile)	0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.0
Uniform Delay ( d <sub>1</sub> ), s/veh	13.9	13.2		31.9	13.0		55.5	28.6	31.1	36.6	33.9	33
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0	0.1		134.7	0.1		189.4	0.4	1.2	0.9	3.9	4.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh	13.9	13.3		166.6	13.0		244.9	29.0	32.3	37.5	37.8	37
Level of Service (LOS)	B	B		F	B		F	C	C	D	D	D
Approach Delay, s/veh / LOS	13.4		B	151.8		F	88.8		F	37.8		D
Intersection Delay, s/veh / LOS	88.2						F					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.4	B	2.3	B	2.3	B
Bicycle LOS Score / LOS	0.7	A	2.2	B	1.6	A	1.4	A

### HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency				Duration, h	0.25		
Analyst	DBZ	Analysis Date	Feb 28, 2016	Area Type	Other		
Jurisdiction				PHF	0.91		
Urban Street	Old Henry Road	Analysis Year	2021 Build	Analysis Period	1> 7:30		
Intersection	Bush Farm Road	File Name	AM 21 B.xus				
Project Description	Sutherland Pointe						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h	15	10	104	882	37	64	171	193	292	135	802	5

Signal Information				EB				WB				NB				SB			
Cycle, s	120.2	Reference Phase	2	Green	42.0	66.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	Yes	Simult. Gap E/W	Off	Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	Off																

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		6.0		6.0		5.0		6.0
Phase Duration, s		72.6		72.6		47.6		47.6
Change Period, ( Y+R <sub>c</sub> ), s		6.6		6.6		5.6		5.6
Max Allow Headway ( MAH ), s		4.8		4.8		6.1		5.1
Queue Clearance Time ( g <sub>s</sub> ), s		6.5		68.0		44.0		27.9
Green Extension Time ( g <sub>e</sub> ), s		0.7		0.0		0.0		5.7
Phase Call Probability		1.00		1.00		1.00		1.00
Max Out Probability		0.00		1.00		1.00		0.39

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	11
Adjusted Flow Rate ( v ), veh/h	16	125		969	111		188	212	321	148	473	46
Adjusted Saturation Flow Rate ( s ), veh/h/in	1302	1632		1286	1705		608	1900	1610	1188	1900	1800
Queue Service Time ( g <sub>s</sub> ), s	0.7	4.5		61.5	3.8		16.1	9.8	19.5	12.6	25.9	25
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	4.5	4.5		66.0	3.8		42.0	9.8	19.5	22.4	25.9	25
Green Ratio ( g/C )	0.55	0.55		0.55	0.55		0.35	0.35	0.35	0.35	0.35	0.35
Capacity ( c ), veh/h	734	896		718	936		141	664	563	378	664	65
Volume-to-Capacity Ratio ( X )	0.022	0.140		1.351	0.119		1.331	0.319	0.570	0.393	0.713	0.7
Available Capacity ( c <sub>a</sub> ), veh/h	734	896		718	936		141	664	563	378	664	65
Back of Queue ( Q ), veh/in ( 95 th percentile)	0.4	3.0		82.2	2.6		20.0	8.0	12.3	6.7	18.2	17
Queue Storage Ratio ( RQ ) ( 95 th percentile)	0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh	14.2	13.2		31.9	13.1		55.5	28.6	31.8	36.8	33.9	33
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0	0.1		166.9	0.1		189.4	0.4	1.7	0.9	3.9	4.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh	14.2	13.3		198.8	13.1		244.9	29.0	33.5	37.8	37.8	37
Level of Service ( LOS )	B	B		F	B		F	C	C	D	D	D
Approach Delay, s/veh / LOS	13.4		B	179.8		F	87.3		F	37.8		D
Intersection Delay, s/veh / LOS	99.1						F					

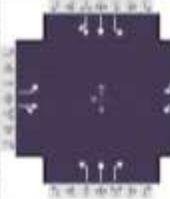
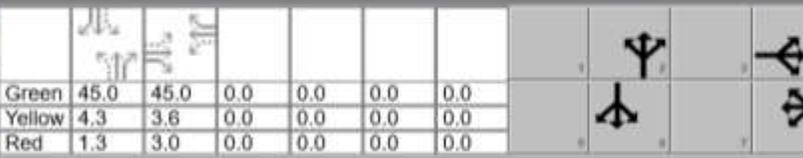
  

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.4	B	2.3	B	2.3	B
Bicycle LOS Score / LOS	0.7	A	2.3	B	1.7	A	1.4	A

### HCS 2010 Signalized Intersection Results Summary

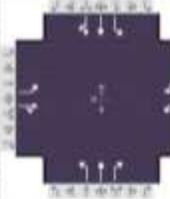
General Information				Intersection Information				Diagram																			
Agency	DBZ			Duration, h	0.25																						
Analyst	DBZ			Analysis Date	Feb 28, 2016																						
Jurisdiction				Time Period	PM Peak																						
Urban Street	Old Henry Road			Analysis Year	2015																						
Intersection	Bush Farm Road			File Name	PM 15.xus																						
Project Description	Sutherland Pointe																										
Demand Information				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h	4	11	38	351	5	65	60	696	552	83	338	6	6	6	6												
Signal Information																											
Cycle, s	73.3	Reference Phase	2																								
Offset, s	0	Reference Point	End																								
Uncoordinated	Yes	Simult. Gap E/W	Off	Green	36.5	24.6	0.0	0.0	0.0	0.0																	
Force Mode	Fixed	Simult. Gap N/S	Off	Yellow	4.3	3.6	0.0	0.0	0.0	0.0																	
				Red	1.3	3.0	0.0	0.0	0.0	0.0																	
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				4			8			2			6														
Case Number				6.0			6.0			5.0			6.0														
Phase Duration, s				31.2			31.2			42.1			42.1														
Change Period, (Y+R <sub>c</sub> ), s				6.6			6.6			5.6			5.6														
Max Allow Headway (MAH), s				4.8			4.7			5.2			5.5														
Queue Clearance Time (g <sub>s</sub> ), s				4.5			22.6			25.9			34.4														
Green Extension Time (g <sub>e</sub> ), s				0.2			2.0			9.7			2.1														
Phase Call Probability				1.00			1.00			1.00			1.00														
Max Out Probability				0.00			0.01			0.45			0.27														
Movement Group Results				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	11	1	6	11												
Adjusted Flow Rate (v), veh/h	4	53		377	75		65	748	594	89	187	18	89	187	18												
Adjusted Saturation Flow Rate (s), veh/h/in	1345	1667		1373	1628		1025	1900	1610	725	1900	1810	725	1900	1810												
Queue Service Time (g <sub>s</sub> ), s	0.2	1.6		19.1	2.4		2.7	23.9	21.4	8.5	4.0	4.0	8.5	4.0	4.0												
Cycle Queue Clearance Time (g <sub>c</sub> ), s	2.5	1.6		20.6	2.4		6.7	23.9	21.4	32.4	4.0	4.0	32.4	4.0	4.0												
Green Ratio (g/C)	0.34	0.34		0.34	0.34		0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50												
Capacity (c), veh/h	506	559		529	546		553	947	802	223	947	930	223	947	930												
Volume-to-Capacity Ratio (X)	0.009	0.094		0.714	0.138		0.117	0.791	0.740	0.400	0.198	0.198	0.400	0.198	0.198												
Available Capacity (c <sub>a</sub> ), veh/h	790	911		819	889		873	1168	990	308	1168	1168	308	1168	1168												
Back of Queue (Q), veh/in (95 th percentile)	0.1	1.0		10.0	1.5		1.1	14.8	11.7	2.7	2.7	2.7	2.7	2.7	2.7												
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00												
Uniform Delay (d <sub>1</sub> ), s/veh	17.8	16.7		23.8	17.0		12.1	15.2	14.6	28.6	10.2	10.2	28.6	10.2	10.2												
Incremental Delay (d <sub>2</sub> ), s/veh	0.0	0.1		2.2	0.1		0.1	3.5	2.8	1.6	0.1	0.1	1.6	0.1	0.1												
Initial Queue Delay (d <sub>3</sub> ), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh	17.9	16.8		26.0	17.1		12.2	18.7	17.4	30.2	10.4	10.4	30.2	10.4	10.4												
Level of Service (LOS)	B	B		C	B		B	B	B	C	B	B	C	B	B												
Approach Delay, s/veh / LOS	16.9		B	24.5		C	17.8		B	14.2		B	14.2		B												
Intersection Delay, s/veh / LOS				18.4						B																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS	2.8		C	2.4		B	2.2		B	2.2		B	2.2		B												
Bicycle LOS Score / LOS	0.6		A	1.2		A	2.8		C	0.9		A	0.9		A												

### HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	DBZ			Duration, h	0.25										
Analyst	DBZ			Analysis Date	Feb 28, 2016										
Jurisdiction				Area Type	Other										
Urban Street	Old Henry Road			Time Period	PM Peak										
Intersection	Bush Farm Road			PHF	0.93										
Project Description	Sutherland Pointe			Analysis Year	2021 No Build										
				Analysis Period	1> 5:00										
				File Name	PM 21 NB.xus										
															
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				74	40	225	692	12	125	97	877	728	106	475	1
Signal Information															
Cycle, s	102.2	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	Off	Green	45.0	45.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	Off	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
				Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8		2		6				
Case Number					6.0		6.0		5.0		6.0				
Phase Duration, s					51.6		51.6		50.6		50.6				
Change Period, (Y+R <sub>c</sub> ), s					6.6		6.6		5.6		5.6				
Max Allow Headway (MAH), s					4.8		5.2		5.2		5.6				
Queue Clearance Time (g <sub>s</sub> ), s					14.0		47.0		47.0		47.0				
Green Extension Time (g <sub>e</sub> ), s					1.9		0.0		0.0		0.0				
Phase Call Probability					1.00		1.00		1.00		1.00				
Max Out Probability					0.00		1.00		1.00		1.00				
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	11
Adjusted Flow Rate (v), veh/h				80	285		744	147		104	943	783	114	267	26
Adjusted Saturation Flow Rate (s), veh/h/ln				1260	1648		1112	1632		886	1900	1610	604	1900	18
Queue Service Time (g <sub>s</sub> ), s				4.2	12.0		33.0	5.7		8.9	45.0	45.0	0.0	9.4	9.0
Cycle Queue Clearance Time (g <sub>c</sub> ), s				9.9	12.0		45.0	5.7		18.3	45.0	45.0	45.0	9.4	9.0
Green Ratio (g/C)				0.44	0.44		0.44	0.44		0.44	0.44	0.44	0.44	0.44	0.4
Capacity (c), veh/h				555	726		430	719		379	837	709	70	837	82
Volume-to-Capacity Ratio (X)				0.143	0.393		1.731	0.205		0.275	1.127	1.104	1.618	0.319	0.3
Available Capacity (c <sub>a</sub> ), veh/h				555	726		430	719		379	837	709	70	837	82
Back of Queue (Q), veh/ln (95 th percentile)				2.2	8.0		83.4	3.8		3.4	49.9	40.8	15.0	7.3	7.0
Queue Storage Ratio (RQ) (95 th percentile)				0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.0
Uniform Delay (d <sub>1</sub> ), s/veh				20.6	19.4		37.3	17.6		24.6	28.6	28.6	51.1	18.6	18
Incremental Delay (d <sub>2</sub> ), s/veh				0.1	0.4		338.6	0.2		0.6	72.3	66.0	333.8	0.3	0.0
Initial Queue Delay (d <sub>3</sub> ), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				20.8	19.8		375.9	17.8		25.1	100.9	94.6	384.9	18.9	18
Level of Service (LOS)				C	B		F	B		C	F	F	F	B	B
Approach Delay, s/veh / LOS				20.0		B	316.7		F	93.9		F	83.6		F
Intersection Delay, s/veh / LOS				138.1						F					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.8		C	2.4		B	2.3		B	2.3		B
Bicycle LOS Score / LOS				1.1		A	2.0		A	3.5		D	1.0		A

### HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	DBZ			Duration, h	0.25		
Analyst	DBZ	Analysis Date	Feb 28, 2016	Area Type	Other		
Jurisdiction		Time Period	PM Peak	PHF	0.93		
Urban Street	Old Henry Road		Analysis Year	2021 Build	Analysis Period	1> 5:00	
Intersection	Bush Farm Road		File Name	PM 21 B.xus			
Project Description	Sutherland Pointe						



Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h				74	40	225	737	12	136	97	877	804	125	475	11

Signal Information				Signal Phases											
Cycle, s	102.2	Reference Phase	2	[Signal Diagram]											
Offset, s	0	Reference Point	End	[Signal Diagram]											
Uncoordinated	Yes	Simult. Gap E/W	Off	Green	45.0	45.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	Off	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
				Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Timer Results		EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase			4		8		2		6
Case Number			6.0		6.0		5.0		6.0
Phase Duration, s			51.6		51.6		50.6		50.6
Change Period, ( Y+R <sub>c</sub> ), s			6.6		6.6		5.6		5.6
Max Allow Headway ( MAH ), s			4.8		5.2		5.2		5.7
Queue Clearance Time ( g <sub>s</sub> ), s			14.0		47.0		47.0		47.0
Green Extension Time ( g <sub>e</sub> ), s			1.9		0.0		0.0		0.0
Phase Call Probability			1.00		1.00		1.00		1.00
Max Out Probability			0.00		1.00		1.00		1.00

Movement Group Results		EB			WB			NB			SB		
Approach Movement		L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement		7	4	14	3	8	18	5	2	12	1	6	11
Adjusted Flow Rate ( v ), veh/h		80	285		792	159		104	943	865	134	267	26
Adjusted Saturation Flow Rate ( s ), veh/h/ln		1247	1648		1112	1630		886	1900	1610	604	1900	18
Queue Service Time ( g <sub>s</sub> ), s		4.3	12.0		33.0	6.2		8.9	45.0	45.0	0.0	9.4	9.0
Cycle Queue Clearance Time ( g <sub>c</sub> ), s		10.5	12.0		45.0	6.2		18.3	45.0	45.0	45.0	9.4	9.0
Green Ratio ( g/C )		0.44	0.44		0.44	0.44		0.44	0.44	0.44	0.44	0.44	0.44
Capacity ( c ), veh/h		544	726		430	718		379	837	709	70	837	82
Volume-to-Capacity Ratio ( X )		0.146	0.393		1.844	0.222		0.275	1.127	1.219	1.908	0.319	0.319
Available Capacity ( c <sub>a</sub> ), veh/h		544	726		430	718		379	837	709	70	837	82
Back of Queue ( Q ), veh/ln ( 95 th percentile)		2.3	8.0		93.7	4.1		3.4	49.9	55.2	19.3	7.3	7.0
Queue Storage Ratio ( RQ ) ( 95 th percentile)		0.00	0.00		0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh		21.0	19.4		37.3	17.7		24.6	28.6	28.6	51.1	18.6	18.0
Incremental Delay ( d <sub>2</sub> ), s/veh		0.1	0.4		388.6	0.2		0.6	72.3	111.2	456.8	0.3	0.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh		0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh		21.1	19.8		425.9	17.9		25.1	100.9	139.8	507.7	18.9	18.0
Level of Service ( LOS )		C	B		F	B		C	F	F	F	B	B
Approach Delay, s/veh / LOS		20.1	C	357.6	F	114.4	F	117.6	F				F
Intersection Delay, s/veh / LOS		165.6						F					

Multimodal Results		EB			WB			NB			SB		
Pedestrian LOS Score / LOS		2.8	C	2.4	B	2.3	B	2.3	B	2.3	B	2.3	B
Bicycle LOS Score / LOS		1.1	A	2.1	B	3.6	D	1.0	A	1.0	A	1.0	A

HCS 2010 Two-Way Stop Control Summary Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	Aiken at Sutherland Pointe									
Agency/Co.	DBZ							Jurisdiction										
Date Performed	2/28/2016							East/West Street	Aiken Road									
Analysis Year	2021							North/South Street	Sutherland Pointe Entranc									
Time Analyzed	AM Peak							Peak Hour Factor	0.83									
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25									
Project Description	Sutherland Pointe																	
Lanes																		
<p>Major Street: East-West</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority	1U	1	2	3	4U	4	5	6			7	8	9			10	11	12
Number of Lanes	0	0	1	0	0	0	1	0			0	0	0			0	0	0
Configuration		LT						TR								LR		
Volume (veh/h)		9	219				574	2							7		27	
Percent Heavy Vehicles		1													1		1	
Proportion Time Blocked																		
Right Turn Channelized	No				No				No				No					
Median Type	Undivided																	
Median Storage																		
Delay, Queue Length, and Level of Service																		
Flow Rate (veh/h)		275															41	
Capacity		906															397	
v/c Ratio		0.30															0.10	
95% Queue Length		0.0															0.3	
Control Delay (s/veh)		9.0															15.1	
Level of Service (LOS)		A															C	
Approach Delay (s/veh)		0.5															15.1	
Approach LOS		A															C	

HCS 2010 Two-Way Stop Control Summary Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Aiken at Sutherland Pointe							
Agency/Co.	DBZ							Jurisdiction								
Date Performed	2/28/2016							East/West Street	Aiken Road							
Analysis Year	2021							North/South Street	Sutherland Pointe Entrance							
Time Analyzed	PM Peak							Peak Hour Factor	0.96							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Sutherland Pointe															
Lanes																
<p>Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	0	0
Configuration		LT						TR							LR	
Volume (veh/h)		30	579				314	7						4		18
Percent Heavy Vehicles		1												1		1
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																
Delay, Queue Length, and Level of Service																
Flow Rate (veh/h)		634														23
Capacity		1231														552
v/c Ratio		0.52														0.04
95% Queue Length		0.1														0.1
Control Delay (s/veh)		8.0														11.8
Level of Service (LOS)		A														B
Approach Delay (s/veh)	0.7												11.8			
Approach LOS	A												B			