

final report

February 25, 2021
Revised March 29, 2021

Traffic Impact Study

Cedar Creek Crossings
7714 Bardstown Road
Louisville, KY

Prepared for

Louisville Metro Planning Commission
Kentucky Transportation Cabinet



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INTRODUCTION

The development plan for 7714 Bardstown Road in Louisville, KY shows six buildings, which are proposed as general retail (28,025 square feet), a furniture store (3,015 square feet), medical office (3,500 square feet), a fast-food restaurant (4,600 square feet), a fast casual restaurant (2,330 square feet) and a coffee shop (2,320 square feet). **Figure 1** displays a map of the site. Access to the center will be from a full access driveway and a proposed right-in/right-out on Bardstown Road, and full access driveway on Cedar Creek Road. The purpose of this study is to examine the traffic impacts of the development upon the adjacent highway system. For this study, the impact area was defined to be the intersection of Bardstown Road with Cedar Creek Road/Brentlinger Lane, and Southpointe Boulevard/Bartley Drive, and the entrances.



Figure 1. Site Map

EXISTING CONDITIONS

Bardstown Road, US 31E, is maintained by the Kentucky Transportation Cabinet (KYTC) with an estimated 2020 ADT of 48,000 vehicles per day between I 265 and Cedar Creek Road, as estimated from the turning movement count. The road is a four-lane highway with twelve-foot lanes, a center turn lane and eight-foot paved shoulders. An additional northbound lane begins near the entrance of Cedar Creek Baptist Church. The speed limit is 50 mph. There are no sidewalks. The intersection with Cedar Creek Road /Brentlinger Lane is controlled with a traffic signal. There are left turn lanes on all approaches. The northbound and westbound approaches have right turn lanes.

Peak hour traffic counts for the intersections were obtained on March 3, 2020. The a.m. peak hour occurred between 7:15 and 8:15 a.m. The p.m. peak occurred between 4:45 and 5:45 p.m. **Figure 2** illustrates the existing a.m. and p.m. peak hour traffic volumes.

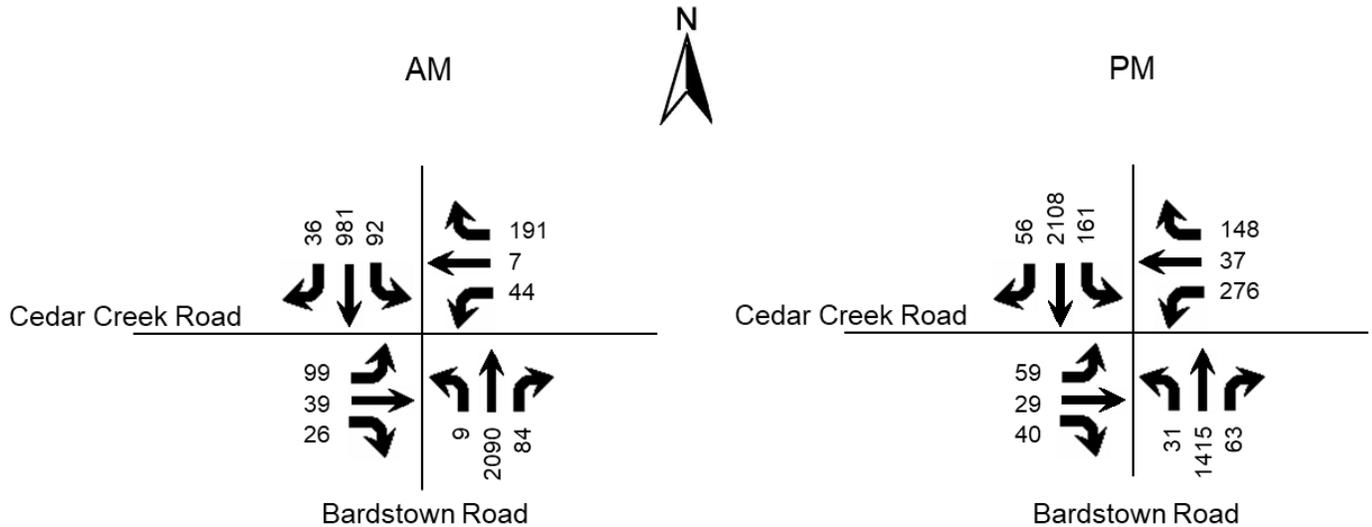


Figure 2. Existing (2020) Peak Hour Volumes

FUTURE CONDITIONS

The project completion date is 2023. An annual growth rate of 1.0 percent was applied to all 2020 volumes except Bardstown Road through traffic; 0.5 percent annual growth was used for Bardstown Road through traffic. This is determined by reviewing 2018 and 2015 counts at the intersection of Cedar Creek Road and Bardstown Road. Additionally, trip generation for 60 additional single-family homes in Cedar Creek Gardens, 88 single family homes on Heights Drive, 116 multifamily units on Brentlinger Lane, Southpointe Commons (approved development plan), Bartley Drive Credit Union, 168 apartments at 7703 Cedar Creek Drive, and 324 apartments at 8000 Cedar Creek Drive are included were included. **Figure 3** displays the 2023 No Build peak hour volumes.

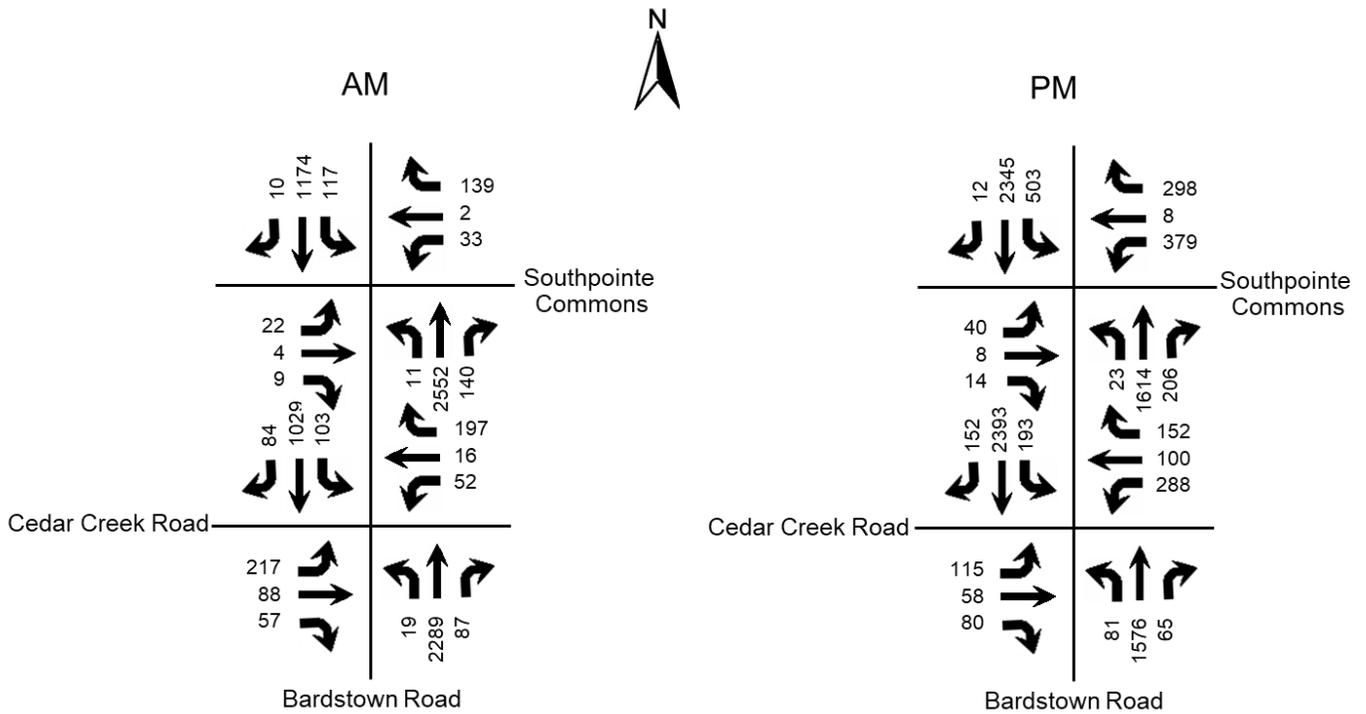


Figure 3. No Build 2023 Peak Hour Volumes

TRIP GENERATION

The Institute of Transportation Engineers Trip Generation Manual, 10th Edition contains trip generation rates for a wide range of land uses. The land uses of “Shopping Center (820)”, “Medical Office (720)”, “Furniture Store (890)”, “Fast-Food Restaurant with drive-thru (934)”, “Fast Casual (930)”, and “Coffee Shop with Drive-Through Window (937)” were reviewed and determined to be the best match. The trip generation results are listed in **Table 1**. The new trips were assigned to the highway network with the percentages shown in **Figure 4**. Pass-by trips were assigned using the peak directional direction of the adjacent streets. **Figure 5** shows the trips generated by this development and distributed throughout the road network during the peak hours. **Figure 6** displays the individual turning movements for the peak hours when the development is completed.

Table 1. Peak Hour Trips Generated by Site

Land Use	A.M. Peak Hour			P.M. Peak Hour		
	Trips	In	Out	Trips	In	Out
Shopping Center (28,025 sq. ft.)	26	16	10	212	102	110
Furniture Store (3,015 sq. ft.)	2	2	0	2	1	1
Medical Office (3,500 sq. ft.)	11	9	2	14	4	10
Fast-Food with drive-thru (4,600 sq. ft.)	185	94	91	150	78	72
Fast Casual (2,330 sq. ft.)	5	3	2	33	18	15
Coffee Shop w/ Drive Thru (2,320 sq. ft.)	206	105	101	101	51	50
Total	435	229	206	512	254	258
Pass-by Trips	192	98	94	212	107	105
TOTAL New Trips	243	131	112	300	147	153

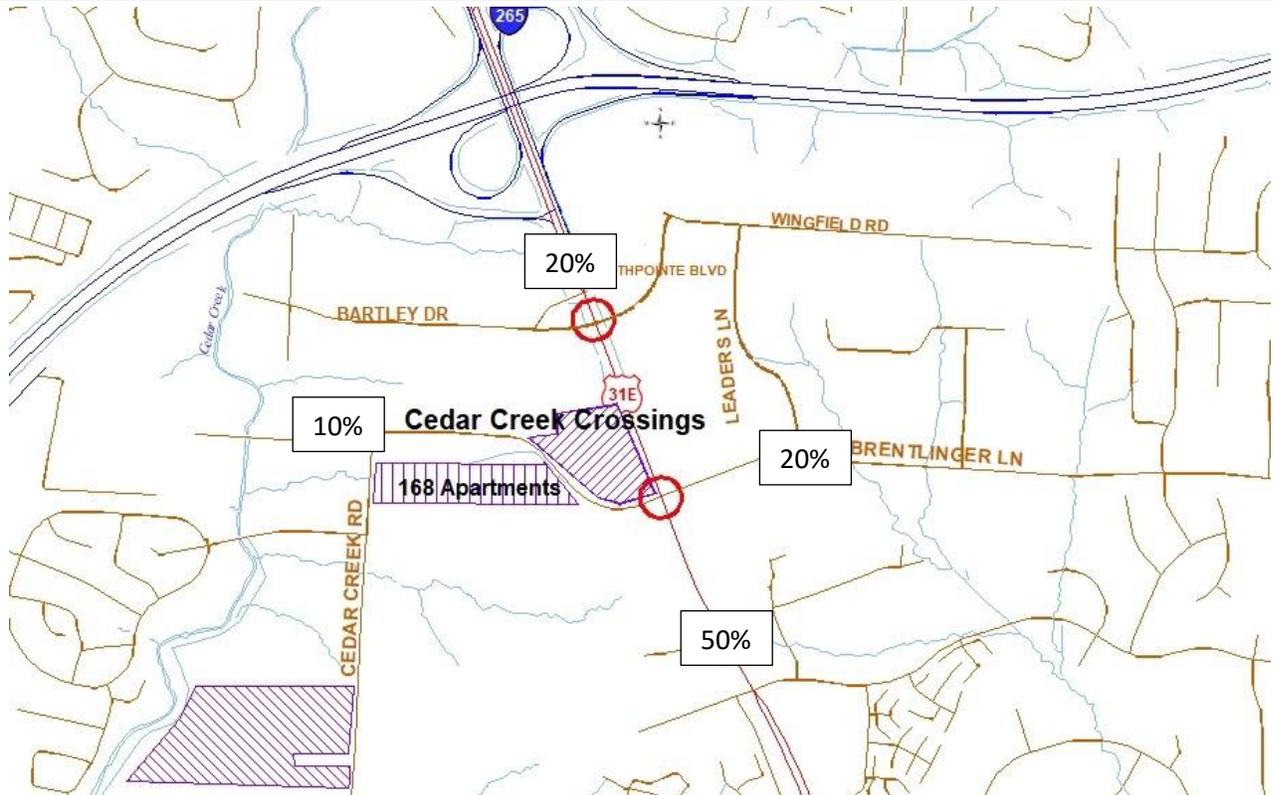


Figure 4. Trip Distribution Percentages

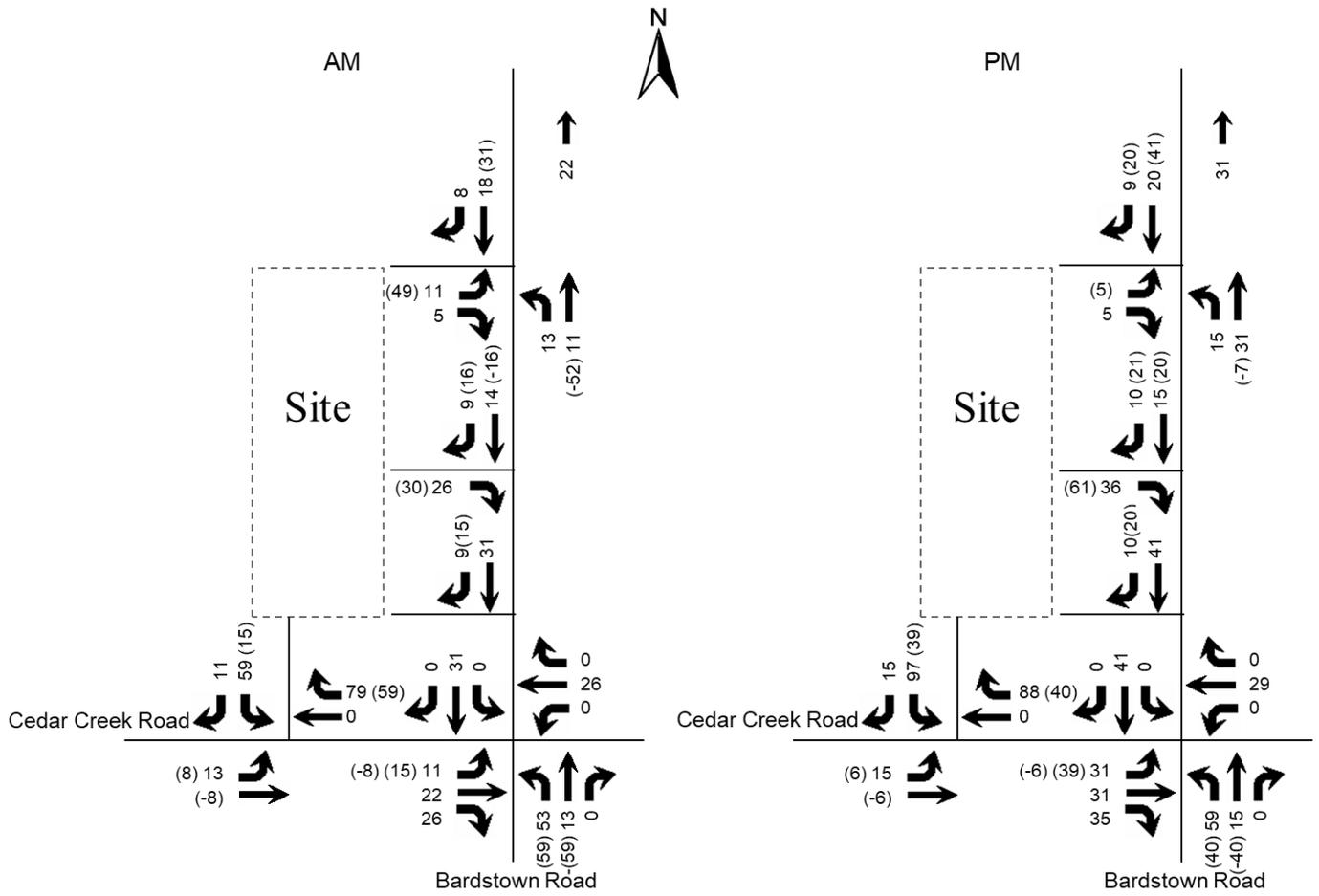


Figure 5. Peak Hour Trips Generated by Site

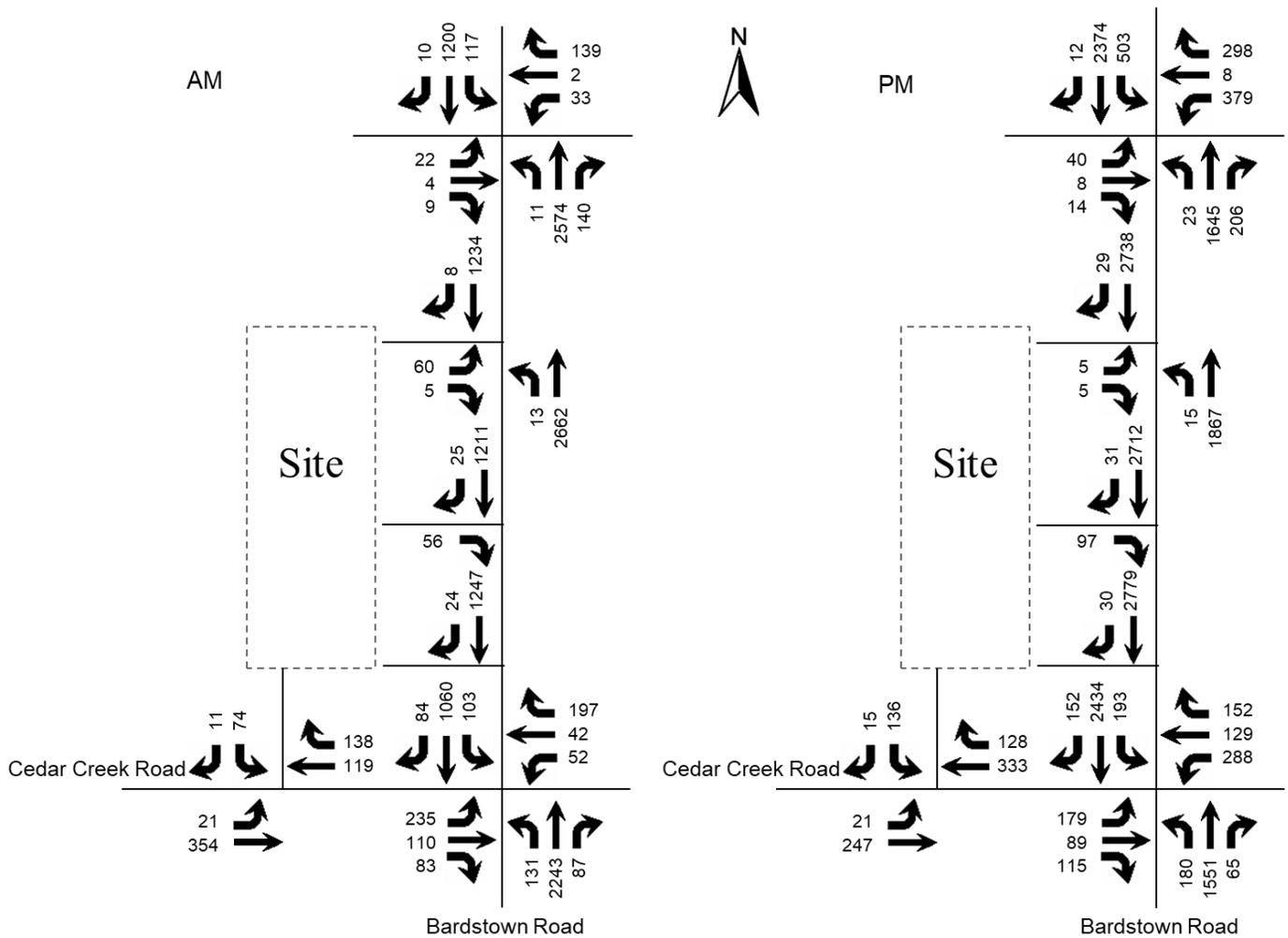


Figure 6. Build 2023 Peak Hour Volumes

ANALYSIS

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a “Level of Service”. Level of Service is a ranking scale from A through F, “A” is the best operating condition and “F” is the worst. Level of Service results depend upon the facility that is analyzed. In this case, the Level of Service is based upon the total delay experienced at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersections were determined using procedures detailed in the Highway Capacity Manual, 6th edition. Future delays and Level of Service were determined for the intersections using the HCS Streets (version 7.9) software. The delays and Level of Service are summarized in **Table 2**.

Table 2. Peak Hour Level of Service

Approach	A.M.			P.M.		
	2020 Existing	2023 No Build	2023 Build	2020 Existing	2023 No Build	2023 Build
Bardstown Road at Cedar Creek Road	C 22.7	C 32.2	C 32.8	D 46.4	D 35.2	D 46.5
Cedar Creek Road Eastbound	E 73.6	F 87.0	F 107.1	E 74.2	E 77.7	F 85.3
Brentlinger Lane Westbound	E 79.3	E 73.3	E 70.8	F 115.6	F 109.2	F 123.5
Bardstown Road Northbound	B 19.9	C 30.0	C 26.8	C 24.4	C 28.0	D 37.6
Bardstown Road Southbound	A 8.8	A 9.8	A 8.4	D 45.6	C 20.1	C 29.4
Bardstown Road at Southpointe Blvd		C 21.1	B 19.5		D 39.1	D 40.1
Bartley Drive Eastbound		F 83.3	F 88.0		F 113.4	F 113.4
Southpointe Boulevard Westbound		E 77.5	E 77.5		F 84.6	F 84.7
Bardstown Road Northbound		B 13.9	B 11.7		B 15.0	B 16.1
Bardstown Road Southbound		C 27.0	C 25.9		D 42.0	D 43.4
Bardstown Road at Entrance						
Entrance Eastbound			D 30.9			F 154.0
Bardstown Road Northbound (left)			B 10.8			F 89.7
Cedar Creek Road at Entrance						
Cedar Creek Road Eastbound (left)			A 7.8			A 8.4
Entrance Southbound			B 14.4			C 19.8

Key: Level of Service, Delay in seconds per vehicle

The entrance was evaluated for turn lanes using the Kentucky Transportation Cabinet [Highway Design Guidance Manual](#) dated July, 2020. The traffic impact policy requires using volumes for ten years beyond build-out, or 2033. The 2033 volumes were determined by applying a 0.5 percent annual growth rate from 2023. **Figure 7** illustrates the 2033 No Build volumes. **Figure 8** illustrates the 2033 Build Volumes. Using the volumes in Figure 8, a right turn lane

is required at the entrance on Bardstown Road. No turn lanes are required on Cedar Creek Road at the entrance. **Table 3** summarizes the delay and Level of Service for 2033.

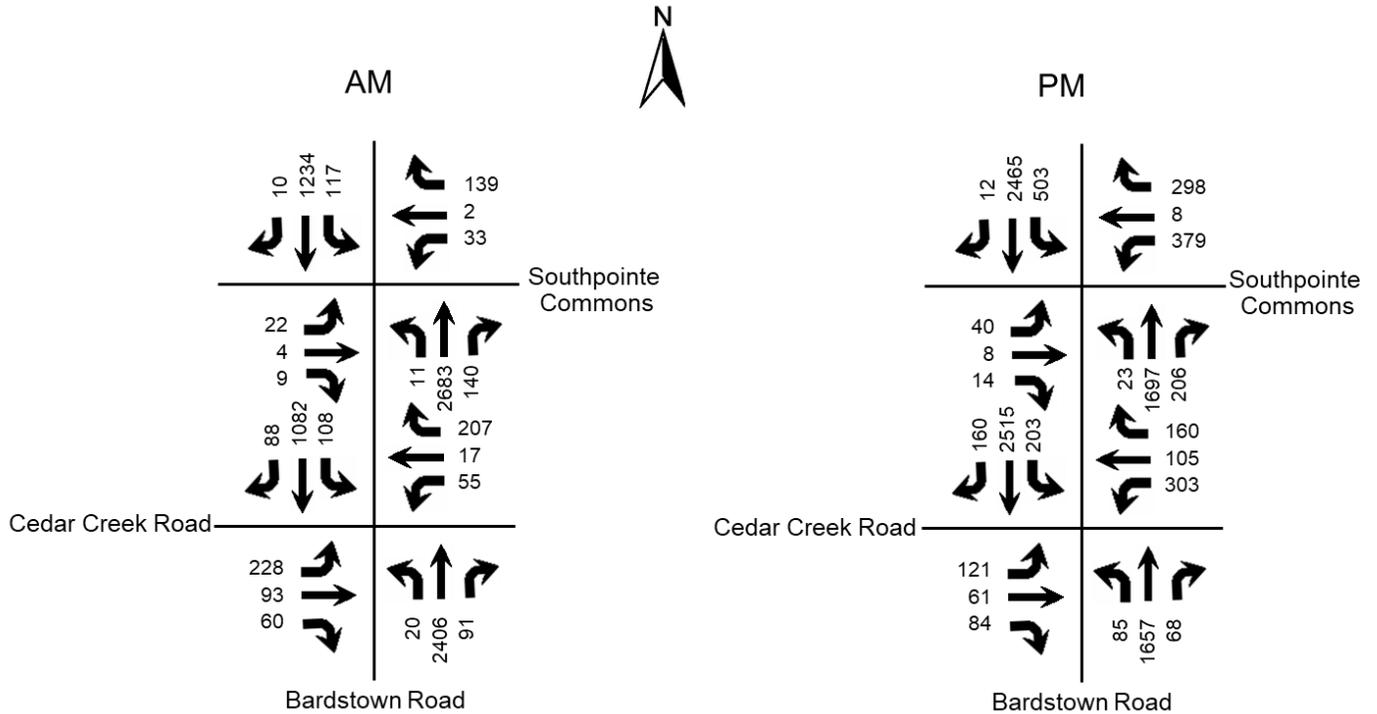


Figure 7. 2033 No Build Peak Hour Volumes

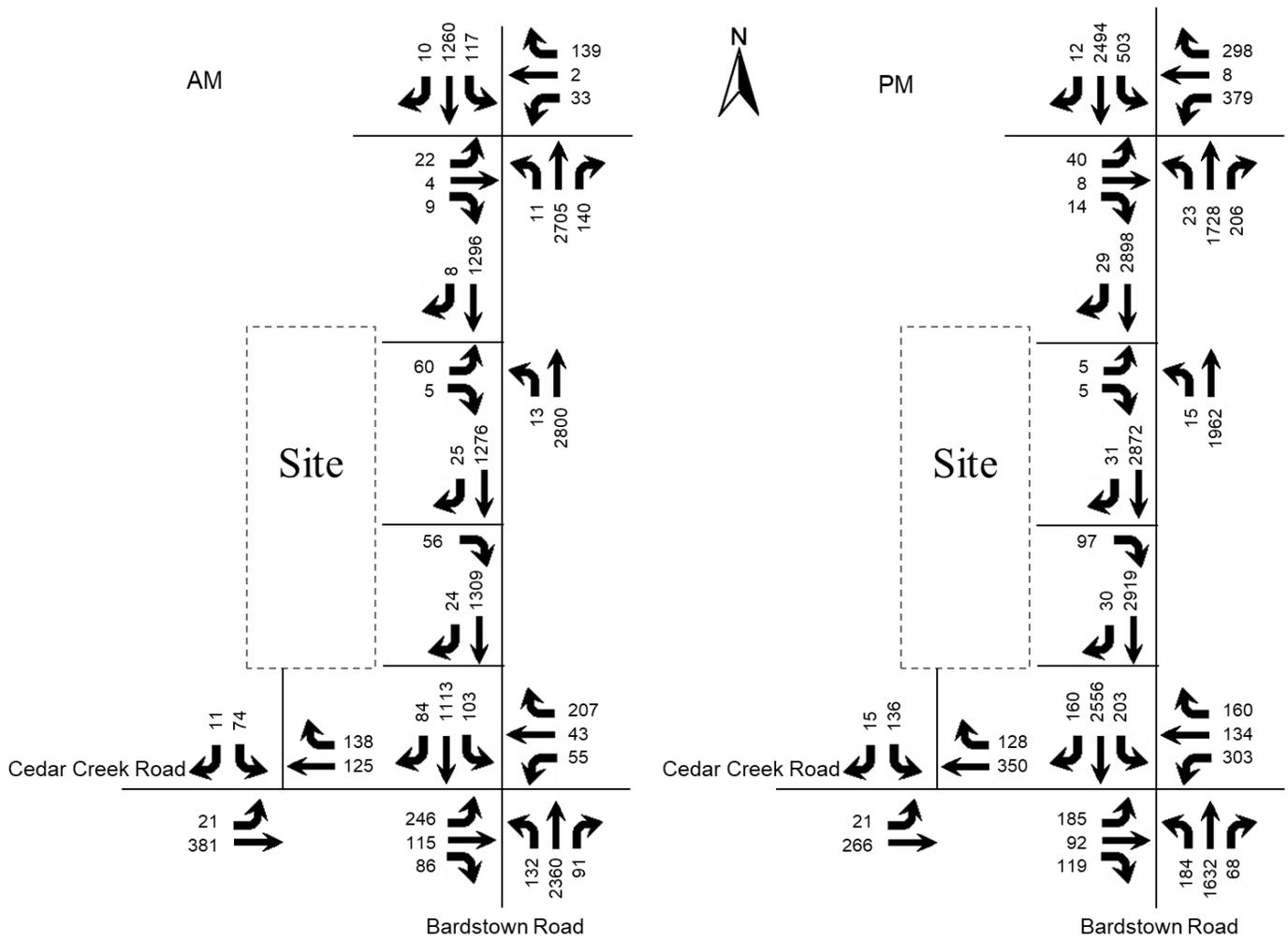


Figure 8. 2033 Build Peak Hour Volumes

Table 3. Peak Hour Level of Service

Approach	A.M.			P.M.		
	2020 Existing	2033 No Build	2033 Build	2020 Existing	2033 No Build	2033 Build
Bardstown Road at Cedar Creek Road	C 22.7	D 40.4	D 38.3	D 46.4	D 38.2	D 49.8
Cedar Creek Road Eastbound	E 73.6	F 92.4	F 115.5	E 74.2	E 78.0	F 88.2
Brentlinger Lane Westbound	E 79.3	E 72.3	E 71.5	F 115.6	F 118.7	F 135.9
Bardstown Road Northbound	B 19.9	D 42.7	C 33.9	C 24.4	C 29.8	D 39.1
Bardstown Road Southbound	A 8.8	B 10.3	A 9.3	D 45.6	C 22.1	C 31.4
Bardstown Road at Southpointe Blvd		C 21.2	B 19.8		D 39.2	D 39.8
Bartley Drive Eastbound		F 88.0	F 88.0		F 113.4	F 113.4
Southpointe Boulevard Westbound		E 77.6	E 77.7		F 85.1	F 85.2
Bardstown Road Northbound		B 14.9	B 12.7		B 14.3	B 16.2
Bardstown Road Southbound		C 25.8	C 25.7		D 43.4	D 43.4
Bardstown Road at Entrance						
Entrance Eastbound			D 34.6			F 381.6
Bardstown Road Northbound (left)			B 11.2			F 181.4
Cedar Creek Road at Entrance						
Cedar Creek Road Eastbound (left)			A 7.9			A 8.5
Entrance Southbound			B 14.9			C 21.1

Key: Level of Service, Delay in seconds per vehicle

CONCLUSIONS

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2023 and 2033, there will be an impact to the existing highway network. The delays experienced in the area will increase. A right turn lane will be required at the entrances on Bardstown Road. Proposed mitigation for the intersection of Bardstown Road at Cedar Creek Road is the construction of a southbound right turn lane on Bardstown Road.

APPENDIX

Cedar Creek Crossing
7714 Bardstown Road
Traffic Impact Study

Traffic Counts

Jefferson County (Louisville), KY
Classified Turn Movement Count



Marr Traffic
Transportation Data Collection

41 Peabody Street, Nashville, TN 37210
10 Glenlake Parkway, Suite 130, Atlanta, GA 30328
555 Fayetteville Street, Suite 201, Raleigh, NC 27601
1229 South Shelby Street, Louisville, KY 40203
6565 North MacArthur Boulevard, Suite 225, Dallas, TX 75039

hello@marrtraffic.com
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Site 3 of 3
US-150 Bardstown Rd (North)
Brentlinger Ln
US-150 Bardstown Rd (South)
Cedar Creek Rd

Lat/Long Weather
38.134142°, -85.579609° Fair
55°F

Date
Tuesday, March 3, 2020

Time	Southbound					Westbound					Northbound					Eastbound					Int				
	US-150 Bardstown Rd (North)					Brentlinger Ln					US-150 Bardstown Rd (South)					Cedar Creek Rd									
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left		Thru	Right	Peds	App
0700 - 0715	0	25	184	7	0	216	0	4	1	60	0	65	0	1	522	14	0	537	0	45	10	3	0	58	876
0715 - 0730	0	14	222	7	0	243	0	6	1	50	0	57	0	1	556	14	0	571	0	34	10	13	0	57	928
0730 - 0745	0	18	239	13	0	270	0	13	3	46	0	62	0	1	521	22	0	544	0	25	12	4	0	41	917
0745 - 0800	0	36	266	11	0	313	0	11	1	51	0	63	0	2	503	27	1	533	0	23	11	4	0	38	947
0800 - 0815	0	24	254	5	0	283	0	14	2	44	0	60	0	5	510	21	0	536	0	17	6	5	0	28	907
0815 - 0830	0	38	216	9	0	263	0	9	4	35	0	48	0	1	482	21	0	504	0	18	13	4	0	35	850
0830 - 0845	0	29	190	9	0	228	0	13	5	70	0	88	0	4	447	22	0	473	0	26	10	7	0	43	832
0845 - 0900	0	69	187	9	0	265	0	17	10	77	0	104	0	2	376	18	0	396	0	15	12	3	0	30	795
1600 - 1615	0	50	468	17	0	535	0	55	16	69	0	140	0	3	298	7	0	308	0	12	3	6	0	21	1004
1615 - 1630	0	41	509	11	0	561	0	70	9	37	0	116	0	7	350	12	0	369	0	19	8	8	0	35	1081
1630 - 1645	0	38	523	14	0	575	0	57	16	41	0	114	0	5	353	13	0	371	0	20	7	10	0	37	1097
1645 - 1700	0	35	489	14	0	538	0	69	5	35	0	109	1	8	349	16	0	374	0	12	6	12	0	30	1051
1700 - 1715	0	34	536	14	0	584	0	66	9	39	0	114	0	9	362	13	0	384	0	17	9	9	0	35	1117
1715 - 1730	0	48	534	14	0	596	0	69	13	38	0	120	0	6	359	19	0	384	0	8	6	11	0	25	1125
1730 - 1745	0	44	549	14	0	607	0	72	10	36	0	118	0	7	345	15	0	367	0	22	8	8	0	38	1130
1745 - 1800	0	49	441	20	0	510	0	60	19	26	0	105	0	6	318	19	0	343	0	15	4	6	0	25	983
0715 - 0730	0	14	222	7	0	243	0	6	1	50	0	57	0	1	556	14	0	571	0	34	10	13	0	57	928
0730 - 0745	0	18	239	13	0	270	0	13	3	46	0	62	0	1	521	22	0	544	0	25	12	4	0	41	917
0745 - 0800	0	36	266	11	0	313	0	11	1	51	0	63	0	2	503	27	1	533	0	23	11	4	0	38	947
0800 - 0815	0	24	254	5	0	283	0	14	2	44	0	60	0	5	510	21	0	536	0	17	6	5	0	28	907
AM PEAK	0	92	981	36	0	1109	0	44	7	191	0	242	0	9	2090	84	1	2184	0	99	39	26	0	164	3699
1645 - 1700	0	35	489	14	0	538	0	69	5	35	0	109	1	8	349	16	0	374	0	12	6	12	0	30	1051
1700 - 1715	0	34	536	14	0	584	0	66	9	39	0	114	0	9	362	13	0	384	0	17	9	9	0	35	1117
1715 - 1730	0	48	534	14	0	596	0	69	13	38	0	120	0	6	359	19	0	384	0	8	6	11	0	25	1125
1730 - 1745	0	44	549	14	0	607	0	72	10	36	0	118	0	7	345	15	0	367	0	22	8	8	0	38	1130
PM PEAK	0	161	2108	56	0	2325	0	276	37	148	0	461	1	30	1415	63	0	1509	0	59	29	40	0	128	4423

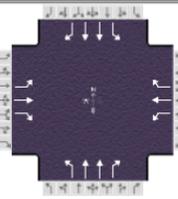
HCS Reports

HCS7 Signalized Intersection Results Summary															
General Information							Intersection Information								
Agency	DBZ Traffic						Duration, h	0.250							
Analyst	DBZ		Analysis Date	Feb 22, 2021			Area Type	Other							
Jurisdiction			Time Period	AM Peak			PHF	0.98							
Urban Street	Bardstown Road		Analysis Year	2023 No Build			Analysis Period	1> 7:15							
Intersection	Brentlinger/Cedar Creek		File Name	Bardstown AM 23 NB.xus											
Project Description	Cedar Creek Crossings														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				217	88	57	52	16	197	19	2289	87	103	1029	84
Signal Information															
Cycle, s	180.0	Reference Phase	2												
Offset, s	0	Reference Point	End	Green	3.7	2.3	125.6	28.4	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	0.0	1.7	3.0	0.0	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					5.0		5.0	1.1	3.0	1.1	4.0				
Phase Duration, s					35.0		35.0	10.2	132.4	12.6	134.8				
Change Period, (Y+R _c), s					6.6		6.6	6.5	6.8	6.5	6.8				
Max Allow Headway (MAH), s					5.2		5.2	5.0	0.0	5.0	0.0				
Queue Clearance Time (g _s), s					31.4		23.3	2.6		5.9					
Green Extension Time (g _e), s					0.0		1.7	0.0	0.0	0.2	0.0				
Phase Call Probability					1.00		1.00	0.62		0.99					
Max Out Probability					1.00		1.00	0.00		0.14					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				221	90	58	53	16	201	19	2336	89	99	542	527
Adjusted Saturation Flow Rate (s), veh/h/ln				1375	1841	1610	1255	1900	1572	1541	1781	1610	1781	1841	1791
Queue Service Time (g _s), s				28.1	7.7	5.6	7.0	1.3	21.3	0.6	101.8	1.5	3.9	8.9	8.6
Cycle Queue Clearance Time (g _c), s				29.4	7.7	5.6	14.8	1.3	21.3	0.6	101.8	1.5	3.9	8.9	8.6
Green Ratio (g/C)				0.16	0.16	0.16	0.16	0.16	0.19	0.72	0.70	0.86	0.73	0.71	0.71
Capacity (c), veh/h				254	301	263	184	300	301	369	2505	1378	121	1309	1274
Volume-to-Capacity Ratio (X)				0.870	0.299	0.221	0.288	0.054	0.667	0.053	0.932	0.064	0.821	0.414	0.414
Back of Queue (Q), ft/ln (95 th percentile)				462.8	172.7	107	111.3	29.3	360.1	10.9	1285.5	49.4	223	124.5	116.2
Back of Queue (Q), veh/ln (95 th percentile)				17.9	6.7	4.3	4.2	1.2	14.1	0.4	50.6	2.0	8.8	4.8	4.6
Queue Storage Ratio (RQ) (95 th percentile)				3.09	0.17	0.86	0.37	0.10	1.60	0.05	1.84	0.25	1.12	0.18	0.17
Uniform Delay (d ₁), s/veh				76.4	66.2	65.4	73.6	64.4	67.4	7.4	23.4	2.0	55.7	3.2	3.1
Incremental Delay (d ₂), s/veh				26.7	0.8	0.6	1.2	0.1	6.2	0.1	7.9	0.1	16.7	0.9	0.9
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				103.1	67.0	66.0	74.8	64.5	73.6	7.5	31.3	2.1	72.4	4.1	4.0
Level of Service (LOS)				F	E	E	E	E	E	A	C	A	E	A	A
Approach Delay, s/veh / LOS				88.5		F	73.3		E	30.0		C	9.8		A
Intersection Delay, s/veh / LOS				32.3						C					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.48		B	2.32		B	2.07		B	2.07		B
Bicycle LOS Score / LOS				1.10		A	0.93		A	2.50		C	1.51		B

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information												
Agency	DBZ Traffic			Duration, h	0.250											
Analyst	DBZ			Analysis Date	Feb 22, 2021								Area Type	Other		
Jurisdiction				Time Period	AM Peak								PHF	0.98		
Urban Street	Bardstown Road			Analysis Year	2023 No Build								Analysis Period	1> 7:15		
Intersection	Brentlinger/Cedar Creek			File Name	Bardstown AM 23 NB.xus											
Project Description	Cedar Creek Crossings															
Demand Information				EB			WB			NB			SB			
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h	217	88	57	52	16	197	19	2289	87	103	1029	84				
Signal Information																
Cycle, s	180.0	Reference Phase	2	Green	3.7	2.3	125.6	28.4	0.0	0.0						
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	5.1	3.6	0.0	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Red	3.0	0.0	1.7	3.0	0.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On													
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4		8	5	2	1	6					
Case Number					5.0		5.0	1.1	3.0	1.1	4.0					
Phase Duration, s					35.0		35.0	10.2	132.4	12.6	134.8					
Change Period, (Y+R _c), s					6.6		6.6	6.5	6.8	6.5	6.8					
Max Allow Headway (MAH), s					5.2		5.2	5.0	0.0	5.0	0.0					
Queue Clearance Time (g _s), s					31.4		23.3	2.6		5.9						
Green Extension Time (g _e), s					0.0		1.7	0.0	0.0	0.2	0.0					
Phase Call Probability					1.00		1.00	0.62		0.99						
Max Out Probability					1.00		1.00	0.00		0.14						
Movement Group Results				EB			WB			NB			SB			
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16				
Adjusted Flow Rate (v), veh/h	221	90	58	53	16	201	19	2336	89	99	542	527				
Adjusted Saturation Flow Rate (s), veh/h/ln	1397	1900	1585	1255	1900	1572	1541	1781	1610	1781	1841	1791				
Queue Service Time (g _s), s	28.1	7.5	5.7	7.0	1.3	21.3	0.6	101.8	1.5	3.9	8.6	8.4				
Cycle Queue Clearance Time (g _c), s	29.4	7.5	5.7	14.5	1.3	21.3	0.6	101.8	1.5	3.9	8.6	8.4				
Green Ratio (g/C)	0.16	0.16	0.16	0.16	0.16	0.19	0.72	0.70	0.86	0.73	0.71	0.71				
Capacity (c), veh/h	258	310	259	186	300	301	370	2505	1378	121	1309	1274				
Volume-to-Capacity Ratio (X)	0.858	0.289	0.225	0.285	0.054	0.667	0.052	0.932	0.064	0.820	0.414	0.414				
Back of Queue (Q), ft/ln (95 th percentile)	450.7	167	108.8	111.1	29.3	360	10.9	1285.5	49.4	223.6	120.5	113.3				
Back of Queue (Q), veh/ln (95 th percentile)	17.7	6.7	4.3	4.2	1.2	14.1	0.4	50.6	2.0	8.8	4.7	4.5				
Queue Storage Ratio (RQ) (95 th percentile)	3.00	0.17	0.87	0.37	0.10	1.60	0.05	1.84	0.25	1.12	0.17	0.16				
Uniform Delay (d ₁), s/veh	76.2	66.1	65.4	73.4	64.4	67.4	7.4	23.4	2.0	55.8	3.1	3.0				
Incremental Delay (d ₂), s/veh	24.6	0.7	0.6	1.2	0.1	6.2	0.1	7.9	0.1	16.7	0.9	0.9				
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (d), s/veh	100.7	66.9	66.0	74.6	64.5	73.6	7.5	31.3	2.1	72.5	4.0	3.9				
Level of Service (LOS)	F	E	E	E	E	E	A	C	A	E	A	A				
Approach Delay, s/veh / LOS	87.0	F	73.3	E	30.0	C	9.8	A								
Intersection Delay, s/veh / LOS	32.2						C									
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS	2.48	B	2.32	B	2.07	B	2.07	B								
Bicycle LOS Score / LOS	1.10	A	0.93	A	2.50	C	1.51	B								

HCS7 Signalized Intersection Results Summary

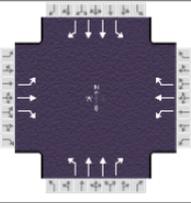
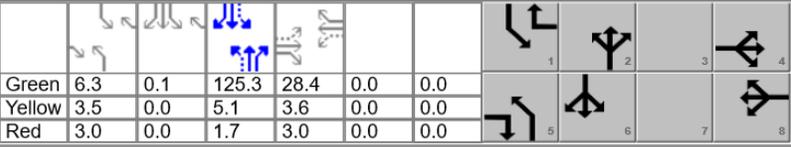
General Information				Intersection Information															
Agency	DBZ Traffic			Duration, h	0.250														
Analyst	DBZ			Analysis Date	Mar 29, 2021														
Jurisdiction				Area Type	Other														
Urban Street	Bardstown Road			Time Period	AM Peak														
Intersection	Brentlinger/Cedar Creek			PHF	0.98														
Project Description	Cedar Creek Crossings			Analysis Year	2023 Build														
	File Name	Bardstown AM 23 B.xus		Analysis Period	1> 7:15														
Demand Information				EB			WB			NB			SB						
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R				
Demand (v), veh/h				235	110	83	52	42	197	131	2243	87	103	1060	84				
Signal Information																			
Cycle, s	180.0	Reference Phase	2																
Offset, s	0	Reference Point	End																
Uncoordinated	No	Simult. Gap E/W	On	Green	6.0	0.3	125.5	28.4	0.0	0.0									
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0									
				Red	3.0	0.0	1.7	3.0	0.0	0.0									
Timer Results				EBL		EBT		WBL		WBT		NBL		NBT		SBL		SBT	
Assigned Phase						4				8		5		2		1		6	
Case Number						5.0		5.0		1.1		3.0		1.1		3.0			
Phase Duration, s						35.0		35.0		12.7		132.5		12.5		132.3			
Change Period, (Y+R _c), s						6.6		6.6		6.5		6.8		6.5		6.8			
Max Allow Headway (MAH), s						5.2		5.2		5.0		0.0		5.0		0.0			
Queue Clearance Time (g _s), s						31.4		23.1		6.0				5.2					
Green Extension Time (g _e), s						0.0		2.0		0.3		0.0		0.2		0.0			
Phase Call Probability						1.00		1.00		1.00				0.99					
Max Out Probability						1.00		0.98		0.18				0.08					
Movement Group Results				EB			WB			NB			SB						
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R				
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16				
Adjusted Flow Rate (v), veh/h				240	112	85	53	43	201	134	2289	89	97	994	79				
Adjusted Saturation Flow Rate (s), veh/h/ln				1364	1900	1598	1230	1900	1572	1767	1781	1610	1781	1752	1585				
Queue Service Time (g _s), s				25.9	9.5	8.0	7.3	3.5	21.1	4.0	95.8	1.5	3.2	7.9	0.8				
Cycle Queue Clearance Time (g _c), s				29.4	9.5	8.0	16.7	3.5	21.1	4.0	95.8	1.5	3.2	7.9	0.8				
Green Ratio (g/C)				0.16	0.16	0.20	0.16	0.16	0.20	0.73	0.70	0.86	0.73	0.70	0.70				
Capacity (c), veh/h				236	310	325	169	300	318	468	2508	1379	124	2443	1105				
Volume-to-Capacity Ratio (X)				1.015	0.362	0.260	0.313	0.143	0.633	0.285	0.913	0.064	0.778	0.407	0.071				
Back of Queue (Q), ft/ln (95 th percentile)				552	206.5	151.3	113	78.1	352.8	66.8	1209.4	49.3	161.9	100.7	12.7				
Back of Queue (Q), veh/ln (95 th percentile)				21.7	8.3	6.0	4.3	3.1	13.8	2.6	47.6	2.0	6.4	3.9	0.5				
Queue Storage Ratio (RQ) (95 th percentile)				3.68	0.21	1.21	0.38	0.26	1.57	0.33	1.73	0.25	0.81	0.14	0.06				
Uniform Delay (d ₁), s/veh				79.2	67.0	60.3	75.3	65.3	65.7	7.2	22.4	2.0	51.1	3.1	2.3				
Incremental Delay (d ₂), s/veh				62.5	1.0	0.6	1.5	0.3	4.7	0.5	6.4	0.1	12.8	0.5	0.1				
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (d), s/veh				141.7	68.0	60.9	76.8	65.6	70.4	7.7	28.8	2.1	63.8	3.5	2.4				
Level of Service (LOS)				F	E	E	E	E	E	A	C	A	E	A	A				
Approach Delay, s/veh / LOS				107.1	F	70.8	E	26.8	C	8.4	A								
Intersection Delay, s/veh / LOS				32.8						C									
Multimodal Results				EB			WB			NB			SB						
Pedestrian LOS Score / LOS				2.48	B	2.48	B	2.07	B	2.07	B								
Bicycle LOS Score / LOS				1.21	A	0.98	A	2.56	C	1.54	B								

Cedar Creek Crossing
7714 Bardstown Road
Traffic Impact Study

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information															
Agency	DBZ Traffic			Duration, h	0.250														
Analyst	DBZ			Analysis Date	Feb 25, 2021							Area Type	Other						
Jurisdiction				Time Period	AM Peak							PHF	0.98						
Urban Street	Bardstown Road			Analysis Year	2033 No Build							Analysis Period	1> 7:15						
Intersection	Brentlinger/Cedar Creek			File Name	Bardstown AM 33 NB.xus														
Project Description	Cedar Creek Crossings																		
Demand Information				EB			WB			NB			SB						
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R				
Demand (v), veh/h				228	93	60	55	17	207	20	2406	91	108	1082	88				
Signal Information																			
Cycle, s	180.0	Reference Phase	2																
Offset, s	0	Reference Point	End																
Uncoordinated	No	Simult. Gap E/W	On	Green	3.8	4.1	123.8	28.4	0.0	0.0									
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0									
				Red	3.0	0.0	1.7	3.0	0.0	0.0									
Timer Results				EBL		EBT		WBL		WBT		NBL		NBT		SBL		SBT	
Assigned Phase						4				8		5		2		1		6	
Case Number						5.0		5.0		1.1		3.0		1.1		4.0			
Phase Duration, s						35.0		35.0		10.3		130.6		14.4		134.7			
Change Period, (Y+R _c), s						6.6		6.6		6.5		6.8		6.5		6.8			
Max Allow Headway (MAH), s						5.2		5.2		5.0		0.0		5.0		0.0			
Queue Clearance Time (g _s), s						31.4		24.3		2.7				7.8					
Green Extension Time (g _e), s						0.0		1.5		0.0		0.0		0.2		0.0			
Phase Call Probability						1.00		1.00		0.64				0.99					
Max Out Probability						1.00		1.00		0.00				0.58					
Movement Group Results				EB			WB			NB			SB						
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R				
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16				
Adjusted Flow Rate (v), veh/h				233	95	61	56	17	211	20	2455	93	99	544	530				
Adjusted Saturation Flow Rate (s), veh/h/ln				1396	1900	1585	1249	1900	1572	1541	1781	1610	1781	1841	1792				
Queue Service Time (g _s), s				28.0	7.9	6.1	7.5	1.4	22.3	0.7	122.6	1.7	5.8	8.3	8.1				
Cycle Queue Clearance Time (g _c), s				29.4	7.9	6.1	15.4	1.4	22.3	0.7	122.6	1.7	5.8	8.3	8.1				
Green Ratio (g/C)				0.16	0.16	0.16	0.16	0.16	0.20	0.71	0.69	0.85	0.75	0.71	0.71				
Capacity (c), veh/h				257	310	259	182	300	317	370	2469	1361	130	1308	1273				
Volume-to-Capacity Ratio (X)				0.905	0.306	0.236	0.308	0.058	0.666	0.055	0.994	0.068	0.763	0.416	0.416				
Back of Queue (Q), ft/ln (95 th percentile)				488.1	177.1	2.2	118.3	31.1	372.3	12	1608.3	54.2	222.6	115.5	108.5				
Back of Queue (Q), veh/ln (95 th percentile)				19.2	7.1	0.1	4.5	1.2	14.5	0.4	63.3	2.2	8.8	4.5	4.3				
Queue Storage Ratio (RQ) (95 th percentile)				3.25	0.18	0.02	0.39	0.10	1.65	0.06	2.30	0.27	1.11	0.16	0.16				
Uniform Delay (d ₁), s/veh				76.9	66.3	65.5	74.0	64.4	66.2	7.8	27.6	2.3	68.0	2.9	2.8				
Incremental Delay (d ₂), s/veh				32.8	0.8	0.7	1.3	0.1	5.8	0.1	16.9	0.1	13.0	0.9	0.9				
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (d), s/veh				109.7	67.1	66.2	75.4	64.5	72.1	7.9	44.5	2.4	81.0	3.8	3.8				
Level of Service (LOS)				F	E	E	E	E	E	A	D	A	F	A	A				
Approach Delay, s/veh / LOS				92.4	F		72.3	E		42.7	D		10.3	B					
Intersection Delay, s/veh / LOS				40.4						D									
Multimodal Results				EB			WB			NB			SB						
Pedestrian LOS Score / LOS				2.48	B		2.32	B		2.08	B		2.07	B					
Bicycle LOS Score / LOS				1.13	A		0.96	A		2.61	C		1.56	B					

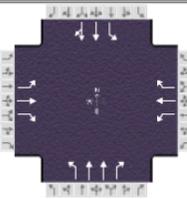
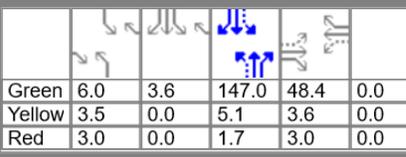
Cedar Creek Crossing
7714 Bardstown Road
Traffic Impact Study

HCS7 Signalized Intersection Results Summary														
General Information						Intersection Information								
Agency	DBZ Traffic					Duration, h	0.250							
Analyst	DBZ		Analysis Date	Mar 29, 2021		Area Type	Other							
Jurisdiction			Time Period	AM Peak		PHF	0.98							
Urban Street	Bardstown Road		Analysis Year	2033 Build		Analysis Period	1> 7:15							
Intersection	Brentlinger/Cedar Creek		File Name	Bardstown AM 33 B.xus										
Project Description	Cedar Creek Crossings													
Demand Information			EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R		
Demand (v), veh/h	246	115	86	55	43	207	132	2360	91	103	1113	84		
Signal Information														
Cycle, s	180.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	6.3	0.1	125.3	28.4	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0				
				Red	3.0	0.0	1.7	3.0	0.0	0.0				
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase				4				8	5	2	1	6		
Case Number				5.0				5.0	1.1	3.0	1.1	3.0		
Phase Duration, s				35.0				35.0	12.8	132.1	12.9	132.2		
Change Period, (Y+R _c), s				6.6				6.6	6.5	6.8	6.5	6.8		
Max Allow Headway (MAH), s				5.2				5.2	5.0	0.0	5.0	0.0		
Queue Clearance Time (g _s), s				31.4				24.2	6.0		6.3			
Green Extension Time (g _e), s				0.0				1.7	0.3	0.0	0.2	0.0		
Phase Call Probability				1.00				1.00	1.00		0.99			
Max Out Probability				1.00				1.00	0.19		0.18			
Movement Group Results			EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R		
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16		
Adjusted Flow Rate (v), veh/h	251	117	88	56	44	211	135	2408	93	93	1005	76		
Adjusted Saturation Flow Rate (s), veh/h/ln	1362	1900	1598	1224	1900	1572	1767	1781	1610	1781	1752	1585		
Queue Service Time (g _s), s	25.8	9.9	8.3	7.8	3.6	22.2	4.0	112.3	1.6	4.3	8.1	0.8		
Cycle Queue Clearance Time (g _c), s	29.4	9.9	8.3	17.7	3.6	22.2	4.0	112.3	1.6	4.3	8.1	0.8		
Green Ratio (g/C)	0.16	0.16	0.20	0.16	0.16	0.20	0.73	0.70	0.86	0.73	0.70	0.70		
Capacity (c), veh/h	235	310	326	166	300	322	458	2498	1374	114	2442	1104		
Volume-to-Capacity Ratio (X)	1.066	0.378	0.269	0.339	0.146	0.656	0.294	0.964	0.068	0.815	0.412	0.069		
Back of Queue (Q), ft/ln (95 th percentile)	594.7	214.4	157	120.5	80	370	67.8	1434.4	52.4	213.4	101.9	12.3		
Back of Queue (Q), veh/ln (95 th percentile)	23.4	8.6	6.2	4.6	3.2	14.5	2.6	56.5	2.1	8.4	4.0	0.5		
Queue Storage Ratio (RQ) (95 th percentile)	3.96	0.21	1.26	0.40	0.27	1.64	0.34	2.05	0.26	1.07	0.15	0.06		
Uniform Delay (d ₁), s/veh	79.3	67.1	60.4	75.9	65.3	65.7	7.4	25.2	2.0	60.9	3.1	2.3		
Incremental Delay (d ₂), s/veh	77.3	1.1	0.6	1.7	0.3	5.4	0.5	11.4	0.1	16.8	0.5	0.1		
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay (d), s/veh	156.6	68.2	61.0	77.6	65.7	71.1	7.9	36.6	2.1	77.7	3.5	2.4		
Level of Service (LOS)	F	E	E	E	E	E	A	D	A	E	A	A		
Approach Delay, s/veh / LOS	115.5	F	71.5	E	33.9	C	9.3	A						
Intersection Delay, s/veh / LOS	38.3						D							
Multimodal Results			EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.48	B	2.48	B	2.07	B	2.07	B						
Bicycle LOS Score / LOS	1.24	A	1.00	A	2.66	C	1.58	B						

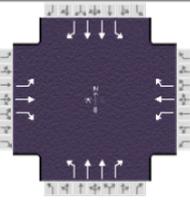
HCS7 Signalized Intersection Results Summary

General Information													Intersection Information														
Agency	DBZ Traffic							Duration, h	0.250																		
Analyst	DBZ							Analysis Date	Feb 8, 2021						Area Type	Other											
Jurisdiction								Time Period	PM Peak						PHF	0.98											
Urban Street	Bardstown Road							Analysis Year	2020						Analysis Period	1> 4:45											
Intersection	Brentlinger/Cedar Creek							File Name	Bardstown PM 20.xus																		
Project Description	Cedar Creek Crossings																										
Demand Information				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h	59	29	40	276	37	148	31	1415	63	161	2108	56															
Signal Information																											
Cycle, s	225.0	Reference Phase	2																								
Offset, s	0	Reference Point	End																								
Uncoordinated	No	Simult. Gap E/W	On	Green	5.2	5.3	146.2	48.4	0.0	0.0																	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0																	
				Red	3.0	0.0	1.7	3.0	0.0	0.0																	
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase							4						8			5			2			1			6		
Case Number							6.0						5.0			1.1			3.0			1.1			4.0		
Phase Duration, s							55.0						55.0			11.7			153.0			17.0			158.3		
Change Period, (Y+R _c), s							6.6						6.6			6.5			6.8			6.5			6.8		
Max Allow Headway (MAH), s							5.2						5.2			5.0			0.0			5.0			0.0		
Queue Clearance Time (g _s), s							13.9						51.4			3.3						9.9					
Green Extension Time (g _e), s							3.9						0.0			0.1			0.0			0.5			0.0		
Phase Call Probability							1.00						1.00			0.86						1.00					
Max Out Probability							0.00						1.00			0.00						0.13					
Movement Group Results				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16															
Adjusted Flow Rate (v), veh/h	60	70		282	38	151	32	1444	64	186	1251	1251															
Adjusted Saturation Flow Rate (s), veh/h/ln	1370	1680		1351	1900	1610	1810	1781	1572	1810	1885	1868															
Queue Service Time (g _s), s	8.3	7.7		41.7	3.6	17.2	1.3	53.7	1.3	7.9	142.3	146.5															
Cycle Queue Clearance Time (g _c), s	11.9	7.7		49.4	3.6	17.2	1.3	53.7	1.3	7.9	142.3	146.5															
Green Ratio (g/C)	0.22	0.22		0.22	0.22	0.26	0.67	0.65	0.87	0.70	0.68	0.68															
Capacity (c), veh/h	305	361		282	409	421	75	2315	1360	270	1278	1266															
Volume-to-Capacity Ratio (X)	0.197	0.195		0.998	0.092	0.359	0.420	0.624	0.047	0.688	0.979	0.988															
Back of Queue (Q), ft/ln (95 th percentile)	136.8	157.3		709.2	80.3	291.9	57.3	765.8	57	166.4	1843.4	1898.7															
Back of Queue (Q), veh/ln (95 th percentile)	5.4	6.1		28.4	3.2	11.7	2.3	30.2	2.2	6.7	73.2	74.8															
Queue Storage Ratio (RQ) (95 th percentile)	0.91	0.16		2.36	0.27	1.30	0.29	1.09	0.29	0.83	2.30	2.35															
Uniform Delay (d ₁), s/veh	75.5	72.3		94.0	70.3	67.7	59.7	23.2	2.1	23.8	32.8	32.6															
Incremental Delay (d ₂), s/veh	0.4	0.4		52.9	0.1	0.7	5.2	1.3	0.1	2.8	13.5	15.3															
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0															
Control Delay (d), s/veh	75.9	72.7		146.9	70.5	68.4	64.9	24.5	2.2	26.6	46.3	47.8															
Level of Service (LOS)	E	E		F	E	E	E	C	A	C	D	D															
Approach Delay, s/veh / LOS	74.2		E	115.6		F	24.4		C	45.6		D															
Intersection Delay, s/veh / LOS				46.4						D																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS	2.48		B	2.33		B	2.09		B	1.90		B															
Bicycle LOS Score / LOS	0.70		A	1.26		A	1.76		B	2.44		B															

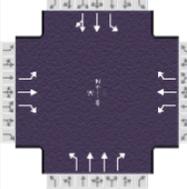
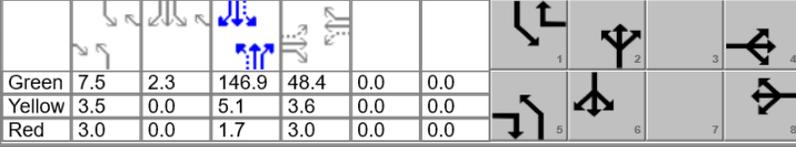
HCS7 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	DBZ Traffic			Duration, h	0.250										
Analyst	DBZ			Analysis Date	Feb 22, 2021										
Jurisdiction				Area Type	Other										
Urban Street	Bardstown Road			Time Period	PM Peak										
Intersection	Brentlinger/Cedar Creek			PHF	0.98										
Project Description	Cedar Creek Crossings			Analysis Year	2023 No Build										
	File Name			Bardstown PM 23 NB.xus											
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				115	58	80	288	100	152	81	1576	65	193	2393	152
Signal Information															
Cycle, s	225.0	Reference Phase	2	Green	6.0	3.6	147.0	48.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	5.1	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On	Red	3.0	0.0	1.7	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					5.0		5.0	1.1	3.0	1.1	4.0				
Phase Duration, s					55.0		55.0	12.5	153.8	16.2	157.5				
Change Period, ($Y+R_c$), s					6.6		6.6	6.5	6.8	6.5	6.8				
Max Allow Headway (MAH), s					5.2		5.2	5.0	0.0	5.0	0.0				
Queue Clearance Time (g_s), s					30.7		51.4	5.8		9.2					
Green Extension Time (g_e), s					4.5		0.0	0.2	0.0	0.5	0.0				
Phase Call Probability					1.00		1.00	0.99		1.00					
Max Out Probability					0.14		1.00	0.00		0.08					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				117	59	82	294	102	155	83	1608	66	181	1193	1193
Adjusted Saturation Flow Rate (s), veh/h/ln				1293	1856	1598	1365	1900	1610	1810	1781	1572	1810	1885	1845
Queue Service Time (g_s), s				18.6	5.8	9.2	43.6	10.0	17.8	3.8	64.2	1.3	7.2	108.6	119.5
Cycle Queue Clearance Time (g_c), s				28.7	5.8	9.2	49.4	10.0	17.8	3.8	64.2	1.3	7.2	108.6	119.5
Green Ratio (g/C)				0.22	0.22	0.24	0.22	0.22	0.26	0.68	0.65	0.87	0.70	0.67	0.67
Capacity (c), veh/h				252	399	387	296	409	416	100	2327	1366	228	1270	1244
Volume-to-Capacity Ratio (X)				0.465	0.148	0.211	0.992	0.250	0.373	0.827	0.691	0.049	0.794	0.939	0.959
Back of Queue (Q), ft/ln (95 th percentile)				268.7	130.6	174.5	729.4	216.8	300.1	163.9	895	58.2	205.7	892.1	940.5
Back of Queue (Q), veh/ln (95 th percentile)				10.6	5.1	6.9	29.2	8.7	12.0	6.6	35.2	2.3	8.2	35.4	37.0
Queue Storage Ratio (RQ) (95 th percentile)				1.79	0.13	1.16	2.43	0.72	1.33	0.82	1.28	0.29	1.03	1.12	1.17
Uniform Delay (d_1), s/veh				85.1	71.6	68.1	92.9	72.8	68.5	59.5	24.6	2.0	35.1	15.2	15.2
Incremental Delay (d_2), s/veh				1.9	0.2	0.4	49.9	0.5	0.8	20.9	1.7	0.1	2.0	2.9	4.2
Initial Queue Delay (d_3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				87.0	71.8	68.5	142.8	73.3	69.3	80.4	26.4	2.1	37.1	18.2	19.4
Level of Service (LOS)				F	E	E	F	E	E	F	C	A	D	B	B
Approach Delay, s/veh / LOS				77.7		E	109.2		F	28.0		C	20.1		C
Intersection Delay, s/veh / LOS				35.2						D					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.48		B	2.33		B	2.09		B	2.09		B
Bicycle LOS Score / LOS				0.91		A	1.40		A	1.94		B	2.79		C

HCS7 Signalized Intersection Results Summary

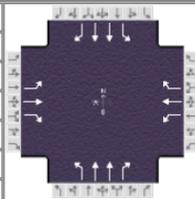
General Information				Intersection Information																							
Agency	DBZ Traffic			Duration, h	0.250																						
Analyst	DBZ			Analysis Date	Feb 22, 2021																						
Jurisdiction				Area Type	Other																						
Urban Street	Bardstown Road			Time Period	PM Peak																						
Intersection	Brentlinger/Cedar Creek			PHF	0.98																						
Project Description	Cedar Creek Crossings			Analysis Year	2023 Build																						
	File Name	Bardstown PM 23 B.xus			Analysis Period	1> 4:45																					
Demand Information				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R															
Demand (v), veh/h	179	89	115	288	129	152	180	1551	65	193	2434	152															
Signal Information																											
Cycle, s	225.0	Reference Phase	2																								
Offset, s	0	Reference Point	End																								
Uncoordinated	No	Simult. Gap E/W	On	Green	10.4	1.6	138.2	48.4	0.0	0.0																	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	5.1	3.6	0.0	0.0																	
				Red	3.0	3.0	1.7	3.0	0.0	0.0																	
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase							4						8			5			2			1			6		
Case Number							5.0						5.0			1.1			3.0			1.1			3.0		
Phase Duration, s							55.0						55.0			25.0			153.1			16.9			145.0		
Change Period, (Y+R _c), s							6.6						6.6			6.5			6.8			6.5			6.8		
Max Allow Headway (MAH), s							5.3						5.3			5.0			0.0			5.0			0.0		
Queue Clearance Time (g _s), s							47.2						51.4			20.8						9.9					
Green Extension Time (g _e), s							0.7						0.0			0.0			0.0			0.5			0.0		
Phase Call Probability							1.00						1.00			1.00						1.00					
Max Out Probability							1.00						1.00			1.00						0.12					
Movement Group Results				EB			WB			NB			SB														
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16															
Adjusted Flow Rate (v), veh/h	183	91	117	294	132	155	184	1583	66	177	2234	139															
Adjusted Saturation Flow Rate (s), veh/h/ln	1258	1856	1598	1326	1900	1610	1810	1781	1572	1810	1795	1585															
Queue Service Time (g _s), s	32.1	9.0	12.4	40.4	13.1	17.5	18.8	62.9	1.3	7.9	139.2	3.3															
Cycle Queue Clearance Time (g _c), s	45.2	9.0	12.4	49.4	13.1	17.5	18.8	62.9	1.3	7.9	139.2	3.3															
Green Ratio (g/C)	0.22	0.22	0.31	0.22	0.22	0.27	0.71	0.65	0.87	0.66	0.62	0.62															
Capacity (c), veh/h	235	407	489	270	409	435	189	2316	1361	234	2221	981															
Volume-to-Capacity Ratio (X)	0.778	0.223	0.240	1.089	0.322	0.357	0.973	0.683	0.049	0.757	1.006	0.142															
Back of Queue (Q), ft/ln (95 th percentile)	436	200.8	223.3	781	269.4	295.9	494	880.2	58.8	165.9	1214	45.2															
Back of Queue (Q), veh/ln (95 th percentile)	17.2	7.8	8.9	31.2	10.8	11.8	19.8	34.7	2.3	6.6	48.2	1.8															
Queue Storage Ratio (RQ) (95 th percentile)	2.91	0.20	1.49	2.60	0.90	1.32	2.47	1.26	0.29	0.83	1.21	0.23															
Uniform Delay (d ₁), s/veh	92.6	72.0	58.4	94.7	74.1	66.3	89.6	24.7	2.1	32.2	21.7	6.5															
Incremental Delay (d ₂), s/veh	16.0	0.4	0.4	80.5	0.6	0.7	57.5	1.7	0.1	1.5	8.8	0.0															
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0															
Control Delay (d), s/veh	108.7	72.4	58.8	175.2	74.7	67.0	147.1	26.4	2.2	33.7	30.5	6.5															
Level of Service (LOS)	F	E	E	F	E	E	F	C	A	C	F	A															
Approach Delay, s/veh / LOS	85.3		F	123.5		F	37.6		D	29.4		C															
Intersection Delay, s/veh / LOS	46.5						D																				
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS	2.48	B		2.48	B		2.09	B		2.10	B																
Bicycle LOS Score / LOS	1.13	A		1.45	A		2.00	B		2.83	C																

HCS7 Signalized Intersection Results Summary

HCS7 Signalized Intersection Results Summary																
General Information						Intersection Information										
Agency	DBZ Traffic					Duration, h	0.250									
Analyst	DBZ					Analysis Date	Feb 25, 2021									
Jurisdiction						Area Type	Other									
Urban Street	Bardstown Road					Time Period	PM Peak									
Intersection	Brentlinger/Cedar Creek					PHF	0.98									
Project Description	Cedar Creek Crossings					Analysis Year	2033 No Build									
	File Name	Bardstown PM 33 NB.xus														
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				121	61	84	303	105	160	85	1657	68	203	2515	160	
Signal Information																
Cycle, s	225.0	Reference Phase	2													
Offset, s	0	Reference Point	End													
Uncoordinated	No	Simult. Gap E/W	On	Green	7.5	2.3	146.9	48.4	0.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	0.0	0.0						
				Red	3.0	0.0	1.7	3.0	0.0	0.0						
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4		8	5	2	1	6					
Case Number					5.0		5.0	1.1	3.0	1.1	4.0					
Phase Duration, s					55.0		55.0	14.0	153.7	16.3	156.0					
Change Period, (Y+R _c), s					6.6		6.6	6.5	6.8	6.5	6.8					
Max Allow Headway (MAH), s					5.2		5.2	5.0	0.0	5.0	0.0					
Queue Clearance Time (g _s), s					32.4		51.4	7.3		9.2						
Green Extension Time (g _e), s					4.6		0.0	0.2	0.0	0.5	0.0					
Phase Call Probability					1.00		1.00	1.00		1.00						
Max Out Probability					0.20		1.00	0.01		0.09						
Movement Group Results				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16	
Adjusted Flow Rate (v), veh/h				123	62	86	309	107	163	87	1691	69	183	1202	1202	
Adjusted Saturation Flow Rate (s), veh/h/ln				1287	1856	1598	1361	1900	1610	1810	1781	1572	1810	1885	1845	
Queue Service Time (g _s), s				19.9	6.1	9.6	43.3	10.6	18.8	5.3	70.6	1.4	7.2	116.6	129.8	
Cycle Queue Clearance Time (g _c), s				30.4	6.1	9.6	49.4	10.6	18.8	5.3	70.6	1.4	7.2	116.6	129.8	
Green Ratio (g/C)				0.22	0.22	0.25	0.22	0.22	0.26	0.69	0.65	0.87	0.70	0.67	0.67	
Capacity (c), veh/h				248	399	397	294	409	416	104	2326	1365	211	1258	1232	
Volume-to-Capacity Ratio (X)				0.497	0.156	0.216	1.052	0.262	0.392	0.837	0.727	0.051	0.866	0.956	0.976	
Back of Queue (Q), ft/ln (95 th percentile)				282.2	137.6	182	792.4	225.8	314.4	238.2	975.3	61.1	222.7	935.4	999.8	
Back of Queue (Q), veh/ln (95 th percentile)				11.1	5.4	7.2	31.7	9.0	12.6	9.5	38.4	2.4	8.9	37.1	39.4	
Queue Storage Ratio (RQ) (95 th percentile)				1.88	0.14	1.21	2.64	0.75	1.40	1.19	1.39	0.31	1.11	1.17	1.24	
Uniform Delay (d ₁), s/veh				86.1	71.7	67.1	93.2	73.1	68.8	70.1	25.8	2.0	41.0	15.8	15.9	
Incremental Delay (d ₂), s/veh				2.2	0.3	0.4	66.9	0.5	0.9	21.2	2.0	0.1	3.7	3.6	5.4	
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh				88.3	72.0	67.5	160.2	73.5	69.7	91.4	27.8	2.1	44.7	19.4	21.4	
Level of Service (LOS)				F	E	E	F	E	E	F	C	A	D	B	C	
Approach Delay, s/veh / LOS				78.0		E	118.7		F	29.8		C	22.1		C	
Intersection Delay, s/veh / LOS				38.2						D						
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS				2.48		B	2.33		B	2.09		B	2.09		B	
Bicycle LOS Score / LOS				0.94		A	1.44		A	2.01		B	2.91		C	

HCS7 Signalized Intersection Results Summary

General Information												Intersection Information					
Agency	DBZ Traffic						Duration, h	0.250									
Analyst	DBZ						Analysis Date	Feb 25, 2021									
Jurisdiction							Time Period	PM Peak									
Urban Street	Bardstown Road						Analysis Year	2033 Build									
Intersection	Brentlinger/Cedar Creek						File Name	Bardstown PM 33 B.xus									
Project Description	Cedar Creek Crossings																



Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	185	92	119	303	134	160	184	1632	68	203	2556	160			

Signal Information				Signal Timing (s)											
Cycle, s	225.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On	Green	10.3	1.7	138.2	48.4	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	5.1	3.6	0.0	0.0					
				Red	3.0	3.0	1.7	3.0	0.0	0.0					

Timer Results		EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase			4		8	5	2	1	6
Case Number			5.0		5.0	1.1	3.0	1.1	3.0
Phase Duration, s			55.0		55.0	25.0	153.2	16.8	145.0
Change Period, (Y+R c), s			6.6		6.6	6.5	6.8	6.5	6.8
Max Allow Headway (MAH), s			5.3		5.3	5.0	0.0	5.0	0.0
Queue Clearance Time (g s), s			49.3		51.4	21.4		9.8	
Green Extension Time (g e), s			0.0		0.0	0.0	0.0	0.5	0.0
Phase Call Probability			1.00		1.00	1.00		1.00	
Max Out Probability			1.00		1.00	1.00		0.12	

Movement Group Results		EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16	
Adjusted Flow Rate (v), veh/h	189	94	121	309	137	163	188	1665	69	179	2252	141	
Adjusted Saturation Flow Rate (s), veh/h/ln	1252	1856	1598	1323	1900	1610	1810	1781	1572	1810	1795	1585	
Queue Service Time (g s), s	33.6	9.4	12.8	40.0	13.7	18.5	19.4	69.0	1.4	7.8	139.2	3.3	
Cycle Queue Clearance Time (g c), s	47.3	9.4	12.8	49.4	13.7	18.5	19.4	69.0	1.4	7.8	139.2	3.3	
Green Ratio (g/C)	0.22	0.22	0.31	0.22	0.22	0.27	0.71	0.65	0.87	0.67	0.62	0.62	
Capacity (c), veh/h	231	407	489	267	409	434	189	2318	1362	224	2221	981	
Volume-to-Capacity Ratio (X)	0.818	0.230	0.248	1.156	0.335	0.376	0.994	0.719	0.051	0.798	1.014	0.144	
Back of Queue (Q), ft/ln (95 th percentile)	459.7	206.4	230.1	854.2	278.4	310.1	512	956.4	61.6	205.9	1223.7	44.1	
Back of Queue (Q), veh/ln (95 th percentile)	18.1	8.1	9.1	34.2	11.1	12.4	20.5	37.7	2.4	8.2	48.6	1.7	
Queue Storage Ratio (RQ) (95 th percentile)	3.06	0.21	1.53	2.85	0.93	1.38	2.56	1.37	0.31	1.03	1.22	0.22	
Uniform Delay (d 1), s/veh	93.8	72.2	58.6	94.9	74.3	66.8	90.1	25.8	2.1	38.2	21.4	6.4	
Incremental Delay (d 2), s/veh	20.9	0.4	0.4	104.0	0.7	0.8	63.8	2.0	0.1	1.9	10.9	0.0	
Initial Queue Delay (d 3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	114.7	72.6	59.0	198.9	75.0	67.5	153.9	27.7	2.2	40.1	32.3	6.4	
Level of Service (LOS)	F	E	E	F	E	E	F	C	A	D	F	A	
Approach Delay, s/veh / LOS	88.2		F	135.9		F	39.1		D	31.4		C	
Intersection Delay, s/veh / LOS	49.8						D						

Multimodal Results		EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.48	B			2.48	B			2.09	B			
Bicycle LOS Score / LOS	1.15	A			1.49	A			2.07	B			

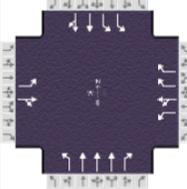
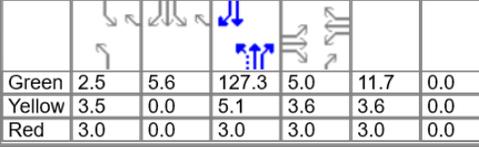
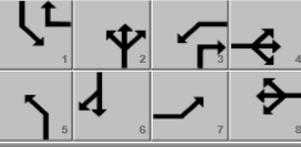
Cedar Creek Crossing
7714 Bardstown Road
Traffic Impact Study

HCS7 Signalized Intersection Results Summary

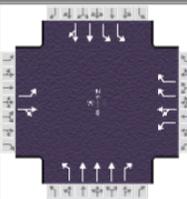
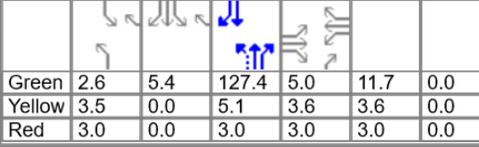
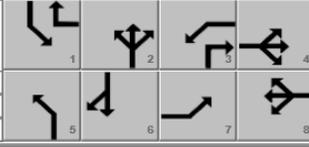
General Information				Intersection Information																				
Agency	DBZ Traffic			Duration, h	0.250																			
Analyst	DBZ			Analysis Date	Feb 22, 2021								Area Type	Other										
Jurisdiction				Time Period	AM Peak								PHF	0.95										
Urban Street	Bardstown Road			Analysis Year	2023 No Build								Analysis Period	1> 7:15										
Intersection	Bartley/Southpointe			File Name	Bardstown AM 23 NB.xus																			
Project Description	Cedar Creek Crossings																							
Demand Information				EB			WB			NB			SB											
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h	22	4	9	33	2	139	11	2552	140	117	1174	10												
Signal Information																								
Cycle, s	180.0	Reference Phase	2																					
Offset, s	0	Reference Point	End																					
Uncoordinated	No	Simult. Gap E/W	On																					
Force Mode	Fixed	Simult. Gap N/S	On	Green	2.6	5.7	124.0	8.4	11.6	0.0	Yellow	3.5	0.0	5.1	3.6	3.6	0.0	Red	3.0	0.0	3.0	3.0	3.0	0.0
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT													
Assigned Phase				4			8			5			2			1			6					
Case Number				10.0			9.0			1.1			3.0			2.0			4.0					
Phase Duration, s				15.0			18.2			9.1			132.1			14.7			137.7					
Change Period, (Y+R c), s				6.6			6.6			6.5			8.1			6.5			8.1					
Max Allow Headway (MAH), s				3.2			5.3			3.1			0.0			5.1			0.0					
Queue Clearance Time (g s), s				10.4			10.8			2.3			7.8											
Green Extension Time (g e), s				0.0			0.8			0.0			0.0			0.5			0.0					
Phase Call Probability				0.84			1.00			0.43			1.00											
Max Out Probability				1.00			0.02			0.00			0.00											
Movement Group Results				EB			WB			NB			SB											
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16												
Adjusted Flow Rate (v), veh/h	23	14		18	19	146	11	2604	143	112	568	566												
Adjusted Saturation Flow Rate (s), veh/h/ln	1781	1663		1781	1791	1403	1781	1698	1585	1702	1870	1865												
Queue Service Time (g s), s	2.3	1.4		1.7	1.8	8.8	0.3	52.9	2.6	5.8	33.4	33.2												
Cycle Queue Clearance Time (g c), s	2.3	1.4		1.7	1.8	8.8	0.3	52.9	2.6	5.8	33.4	33.2												
Green Ratio (g/C)	0.05	0.05		0.06	0.06	0.11	0.70	0.69	0.75	0.83	0.72	0.72												
Capacity (c), veh/h	83	78		115	116	309	325	3508	1194	156	1347	1343												
Volume-to-Capacity Ratio (X)	0.279	0.176		0.157	0.163	0.473	0.034	0.742	0.120	0.719	0.421	0.421												
Back of Queue (Q), ft/ln (95 th percentile)	48.5	28.5		37.7	39.2	149.1	6.1	570.5	35.7	123.6	570	564.5												
Back of Queue (Q), veh/ln (95 th percentile)	1.9	1.1		1.5	1.5	5.9	0.2	22.5	1.4	4.8	22.4	22.2												
Queue Storage Ratio (RQ) (95 th percentile)	0.16	0.09		0.25	0.20	0.37	0.04	0.52	0.18	0.31	0.63	0.63												
Uniform Delay (d 1), s/veh	82.9	82.5		79.6	79.6	75.2	10.6	14.1	3.3	81.6	19.9	19.7												
Incremental Delay (d 2), s/veh	0.7	0.4		0.9	0.9	1.6	0.0	0.4	0.1	8.1	0.9	0.9												
Initial Queue Delay (d 3), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh	83.5	82.9		80.5	80.5	76.8	10.6	14.5	3.4	89.7	20.9	20.6												
Level of Service (LOS)	F	F		F	F	E	B	B	A	F	C	C												
Approach Delay, s/veh / LOS	83.3		F	77.5		E	13.9		B	27.0		C												
Intersection Delay, s/veh / LOS	21.1						C																	
Multimodal Results				EB			WB			NB			SB											
Pedestrian LOS Score / LOS	2.63		C	2.63		C	2.40		B	1.88		B												
Bicycle LOS Score / LOS	0.55		A	0.79		A	2.05		B	1.62		B												

Cedar Creek Crossing
7714 Bardstown Road
Traffic Impact Study

HCS7 Signalized Intersection Results Summary

HCS7 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency	DBZ Traffic					Duration, h	0.250								
Analyst	DBZ					Analysis Date	Feb 22, 2021		Area Type	Other					
Jurisdiction						Time Period	AM Peak		PHF	0.95					
Urban Street	Bardstown Road					Analysis Year	2023 Build		Analysis Period	1> 7:15					
Intersection	Bartley/Southpointe					File Name	Bardstown AM 23 B.xus								
Project Description	Cedar Creek Crossings														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				22	4	9	33	2	139	11	2574	140	117	1200	10
Signal Information															
Cycle, s	180.0	Reference Phase	2	Green	2.5	5.6	127.3	5.0	11.7	0.0					
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	5.1	3.6	3.6	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Red	3.0	0.0	3.0	3.0	3.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	1.1	3.0	2.0	4.0				
Phase Duration, s					11.6		18.3	9.0	135.4	14.6	141.0				
Change Period, (Y+R _c), s					6.6		6.6	6.5	8.1	6.5	8.1				
Max Allow Headway (MAH), s					3.2		5.3	3.0	0.0	5.0	0.0				
Queue Clearance Time (g _s), s					4.3		10.8	2.3		7.7					
Green Extension Time (g _e), s					0.0		0.9	0.0	0.0	0.5	0.0				
Phase Call Probability					0.84		1.00	0.42		1.00					
Max Out Probability					0.19		0.00	0.00		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				23	14		18	19	146	11	2585	141	110	569	567
Adjusted Saturation Flow Rate (s), veh/h/ln				1781	1663		1781	1791	1403	1781	1698	1585	1702	1870	1865
Queue Service Time (g _s), s				2.3	1.5		1.7	1.8	8.8	0.3	47.7	2.1	5.7	33.1	32.8
Cycle Queue Clearance Time (g _c), s				2.3	1.5		1.7	1.8	8.8	0.3	47.7	2.1	5.7	33.1	32.8
Green Ratio (g/C)				0.03	0.03		0.07	0.07	0.11	0.72	0.71	0.77	0.05	0.74	0.74
Capacity (c), veh/h				50	47		116	117	310	335	3603	1224	154	1381	1377
Volume-to-Capacity Ratio (X)				0.463	0.293		0.156	0.161	0.473	0.033	0.718	0.115	0.713	0.412	0.412
Back of Queue (Q), ft/ln (95 th percentile)				50.4	29.5		37.7	39.2	149	5.2	460.4	25.9	120.6	555.5	549.7
Back of Queue (Q), veh/ln (95 th percentile)				2.0	1.2		1.5	1.5	5.9	0.2	18.1	1.0	4.7	21.9	21.6
Queue Storage Ratio (RQ) (95 th percentile)				0.17	0.10		0.25	0.20	0.37	0.03	0.42	0.13	0.30	0.62	0.61
Uniform Delay (d ₁), s/veh				86.1	85.7		79.5	79.5	75.2	9.6	11.9	2.5	82.6	19.0	18.7
Incremental Delay (d ₂), s/veh				2.5	1.3		0.9	0.9	1.6	0.0	0.3	0.1	8.0	0.9	0.9
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				88.6	87.0		80.3	80.4	76.7	9.6	12.2	2.6	90.5	19.8	19.6
Level of Service (LOS)				F	F		F	F	E	A	B	A	F	B	B
Approach Delay, s/veh / LOS				88.0		F	77.5		E	11.7		B	25.9		C
Intersection Delay, s/veh / LOS				19.5					B						
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.63		C	2.63		C	2.39		B	1.87		B
Bicycle LOS Score / LOS				0.55		A	0.79		A	2.07		B	1.64		B

HCS7 Signalized Intersection Results Summary

HCS7 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency	DBZ Traffic					Duration, h	0.250								
Analyst	DBZ					Analysis Date	Feb 25, 2021		Area Type	Other					
Jurisdiction						Time Period	AM Peak		PHF	0.95					
Urban Street	Bardstown Road					Analysis Year	2033 No Build		Analysis Period	1 > 7:15					
Intersection	Bartley/Southpointe					File Name	Bardstown AM 33 NB.xus								
Project Description	Cedar Creek Crossings														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				22	4	9	33	2	139	11	2683	140	117	1234	10
Signal Information															
Cycle, s	180.0	Reference Phase	2	Green	2.6	5.4	127.4	5.0	11.7	0.0					
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	5.1	3.6	3.6	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Red	3.0	0.0	3.0	3.0	3.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	1.1	3.0	2.0	4.0				
Phase Duration, s					11.6		18.3	9.1	135.5	14.5	140.9				
Change Period, (Y+R _c), s					6.6		6.6	6.5	8.1	6.5	8.1				
Max Allow Headway (MAH), s					3.2		5.3	3.0	0.0	5.0	0.0				
Queue Clearance Time (g _s), s					4.3		10.8	2.3		7.5					
Green Extension Time (g _e), s					0.0		0.9	0.0	0.0	0.5	0.0				
Phase Call Probability					0.84		1.00	0.43		1.00					
Max Out Probability					0.19		0.00	0.00		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				23	14		18	19	146	11	2745	143	107	570	568
Adjusted Saturation Flow Rate (s), veh/h/ln				1781	1663		1781	1791	1403	1781	1698	1585	1702	1870	1865
Queue Service Time (g _s), s				2.3	1.5		1.7	1.8	8.8	0.3	59.2	2.7	5.5	33.1	32.9
Cycle Queue Clearance Time (g _c), s				2.3	1.5		1.7	1.8	8.8	0.3	59.2	2.7	5.5	33.1	32.9
Green Ratio (g/C)				0.03	0.03		0.07	0.07	0.11	0.72	0.71	0.77	0.04	0.74	0.74
Capacity (c), veh/h				50	47		116	117	307	334	3607	1225	151	1380	1376
Volume-to-Capacity Ratio (X)				0.463	0.293		0.156	0.161	0.476	0.034	0.761	0.117	0.709	0.413	0.413
Back of Queue (Q), ft/ln (95 th percentile)				50.4	29.5		37.7	39.2	149.3	5.3	586.8	33.4	117.6	555.9	550.6
Back of Queue (Q), veh/ln (95 th percentile)				2.0	1.2		1.5	1.5	5.9	0.2	23.1	1.3	4.6	21.9	21.7
Queue Storage Ratio (RQ) (95 th percentile)				0.17	0.10		0.25	0.20	0.37	0.04	0.53	0.17	0.29	0.62	0.61
Uniform Delay (d ₁), s/veh				86.1	85.7		79.5	79.5	75.3	9.6	15.2	3.3	82.6	19.0	18.7
Incremental Delay (d ₂), s/veh				2.5	1.3		0.9	0.9	1.6	0.0	0.3	0.0	8.0	0.9	0.9
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				88.6	87.0		80.3	80.4	76.9	9.6	15.5	3.3	90.6	19.8	19.6
Level of Service (LOS)				F	F		F	F	E	A	B	A	F	B	B
Approach Delay, s/veh / LOS				88.0		F	77.6		E	14.9		B	25.8		C
Intersection Delay, s/veh / LOS				21.2						C					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.63		C	2.63		C	2.39		B	1.87		B
Bicycle LOS Score / LOS				0.55		A	0.79		A	2.13		B	1.67		B

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	DBZ Traffic			Duration, h	0.250										
Analyst	DBZ			Analysis Date	Feb 25, 2021										
Jurisdiction				Time Period	AM Peak										
Urban Street	Bardstown Road			Analysis Year	2033 Build										
Intersection	Bartley/Southpointe			File Name	Bardstown AM 33 B.xus										
Project Description	Cedar Creek Crossings														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	22	4	9	33	2	139	11	2705	140	117	1260	10			
Signal Information															
Cycle, s	180.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On	Green	2.5	5.3	127.6	5.0	11.7	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	5.1	3.6	3.6	0.0					
				Red	3.0	0.0	3.0	3.0	3.0	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	1.1	3.0	2.0	4.0				
Phase Duration, s					11.6		18.3	9.0	135.7	14.4	141.0				
Change Period, (Y+R _c), s					6.6		6.6	6.5	8.1	6.5	8.1				
Max Allow Headway (MAH), s					3.2		5.3	3.0	0.0	5.0	0.0				
Queue Clearance Time (g _s), s					4.3		10.8	2.3		7.4					
Green Extension Time (g _e), s					0.0		0.9	0.0	0.0	0.5	0.0				
Phase Call Probability					0.84		1.00	0.42		0.99					
Max Out Probability					0.19		0.00	0.00		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	23	14		18	19	146	11	2714	140	105	571	569			
Adjusted Saturation Flow Rate (s), veh/h/ln	1781	1663		1781	1791	1403	1781	1698	1585	1702	1870	1865			
Queue Service Time (g _s), s	2.3	1.5		1.7	1.8	8.8	0.3	54.2	2.3	5.4	33.2	33.0			
Cycle Queue Clearance Time (g _c), s	2.3	1.5		1.7	1.8	8.8	0.3	54.2	2.3	5.4	33.2	33.0			
Green Ratio (g/C)	0.03	0.03		0.07	0.07	0.11	0.72	0.71	0.77	0.04	0.74	0.74			
Capacity (c), veh/h	50	47		116	117	305	333	3610	1227	149	1381	1377			
Volume-to-Capacity Ratio (X)	0.463	0.293		0.156	0.161	0.479	0.033	0.752	0.115	0.706	0.414	0.414			
Back of Queue (Q), ft/ln (95 th percentile)	50.4	29.5		37.7	39.2	149.5	5.2	506.1	27.6	115.4	557.3	551.7			
Back of Queue (Q), veh/ln (95 th percentile)	2.0	1.2		1.5	1.5	5.9	0.2	19.9	1.1	4.5	21.9	21.7			
Queue Storage Ratio (RQ) (95 th percentile)	0.17	0.10		0.25	0.20	0.37	0.03	0.46	0.14	0.29	0.62	0.61			
Uniform Delay (d ₁), s/veh	86.1	85.7		79.5	79.5	75.4	9.6	13.0	2.7	82.6	19.0	18.7			
Incremental Delay (d ₂), s/veh	2.5	1.3		0.9	0.9	1.7	0.0	0.2	0.0	8.0	0.9	0.9			
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	88.6	87.0		80.3	80.4	77.1	9.6	13.2	2.8	90.6	19.8	19.6			
Level of Service (LOS)	F	F		F	F	E	A	B	A	F	B	B			
Approach Delay, s/veh / LOS	88.0		F	77.7		E	12.7		B	25.7		C			
Intersection Delay, s/veh / LOS				19.8						B					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.63		C	2.63		C	2.39		B	1.87		B			
Bicycle LOS Score / LOS	0.55		A	0.79		A	2.14		B	1.69		B			

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	DBZ Traffic			Duration, h	0.250										
Analyst	DBZ			Analysis Date	Feb 22, 2021										
Jurisdiction				Area Type	Other										
Urban Street	Bardstown Road			Time Period	PM Peak										
Intersection	Bartley/Wingfield			PHF	0.96										
Project Description	Cedar Creek Crossings			Analysis Year	2023 No Build							Analysis Period	1> 4:45		
File Name	Bardstown PM 23 NB.xus														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	40	8	14	379	8	298	23	1614	206	503	2345	12			
Signal Information															
Cycle, s	225.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	Off	Green	4.6	26.4	128.8	7.0	28.5	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	3.5	4.7	3.6	3.6	0.0					
				Red	0.0	2.7	1.5	3.0	3.0	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	1.1	3.0	2.0	4.0				
Phase Duration, s					13.6		35.1	8.6	135.0	41.2	167.7				
Change Period, (Y+R c), s					6.6		6.6	4.0	6.2	6.2	6.2				
Max Allow Headway (MAH), s					3.2		3.2	3.1	0.0	3.1	0.0				
Queue Clearance Time (g s), s					7.2		26.9	3.2		33.9					
Green Extension Time (g e), s					0.0		1.6	0.0	0.0	1.2	0.0				
Phase Call Probability					0.98		1.00	0.77		1.00					
Max Out Probability					1.00		0.00	0.00		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	42	23		201	202	310	23	1647	210	497	1165	1165			
Adjusted Saturation Flow Rate (s), veh/h/ln	1795	1691		1781	1786	1403	1795	1698	1585	1730	1885	1882			
Queue Service Time (g s), s	5.2	3.0		24.9	24.9	20.1	1.2	31.0	3.9	31.9	108.8	109.1			
Cycle Queue Clearance Time (g c), s	5.2	3.0		24.9	24.9	20.1	1.2	31.0	3.9	31.9	108.8	109.1			
Green Ratio (g/C)	0.04	0.04		0.13	0.13	0.29	0.59	0.57	0.70	0.82	0.72	0.72			
Capacity (c), veh/h	64	53		233	234	792	104	2917	1108	554	1361	1359			
Volume-to-Capacity Ratio (X)	0.650	0.433		0.862	0.862	0.392	0.226	0.565	0.190	0.897	0.856	0.858			
Back of Queue (Q), ft/ln (95 th percentile)	117.8	61.8		444.8	442.1	297.7	24.8	392	56.1	426.6	1354.5	1364.6			
Back of Queue (Q), veh/ln (95 th percentile)	4.7	2.5		17.5	17.5	11.7	1.0	15.4	2.2	16.8	53.7	53.7			
Queue Storage Ratio (RQ) (95 th percentile)	0.39	0.21		1.48	1.47	0.74	0.17	0.33	0.28	1.07	1.50	1.50			
Uniform Delay (d 1), s/veh	107.1	106.4		95.8	95.8	65.1	35.7	15.6	3.7	96.0	29.6	29.1			
Incremental Delay (d 2), s/veh	9.0	2.1		3.7	3.7	0.1	0.3	0.6	0.3	0.7	0.9	0.9			
Initial Queue Delay (d 3), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	116.1	108.4		99.5	99.5	65.3	36.0	16.1	4.0	96.7	30.5	30.1			
Level of Service (LOS)	F	F		F	F	E	D	B	A	F	C	C			
Approach Delay, s/veh / LOS	113.4		F	84.6		F	15.0		B	42.0		D			
Intersection Delay, s/veh / LOS				39.1						D					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.63		C	2.64		C	2.43		B	1.89		B			
Bicycle LOS Score / LOS	0.59		A	1.66		B	1.54		B	2.95		C			

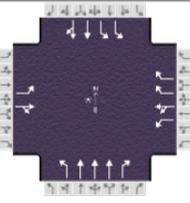
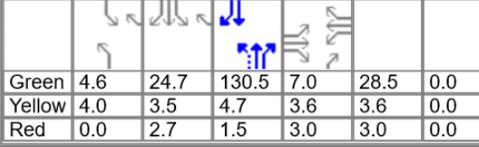
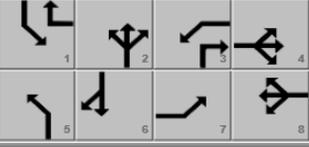
HCS7 Signalized Intersection Results Summary

HCS7 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency	DBZ Traffic					Duration, h	0.250								
Analyst	DBZ					Analysis Date	Feb 22, 2021								
Jurisdiction						Area Type	Other								
Urban Street	Bardstown Road					Time Period	PM Peak								
Intersection	Bartley/Wingfield					PHF	0.96								
Project Description	Cedar Creek Crossings					Analysis Year	2023 Build		Analysis Period	1> 4:45					
File Name	Bardstown PM 23 B.xus														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	40	8	14	379	8	298	23	1645	206	503	2374	12			
Signal Information															
Cycle, s	225.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	Off	Green	4.6	26.0	129.3	7.0	28.5	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	3.5	4.7	3.6	3.6	0.0					
				Red	0.0	2.7	1.5	3.0	3.0	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4					8	5	2	1	6	
Case Number					10.0					9.0	1.1	3.0	2.0	4.0	
Phase Duration, s					13.6					35.1	8.6	135.5	40.8	167.7	
Change Period, (Y+R c), s					6.6					6.6	4.0	6.2	6.2	6.2	
Max Allow Headway (MAH), s					3.2					3.2	3.0	0.0	3.0	0.0	
Queue Clearance Time (g s), s					7.2					26.9	3.2	33.6			
Green Extension Time (g e), s					0.0					1.6	0.0	0.0	1.1	0.0	
Phase Call Probability					0.98					1.00	0.77	1.00			
Max Out Probability					1.00					0.00	0.00	0.00			
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	42	23		201	202	310	23	1677	210	492	1168	1168			
Adjusted Saturation Flow Rate (s), veh/h/ln	1795	1691		1781	1786	1403	1795	1698	1585	1730	1885	1882			
Queue Service Time (g s), s	5.2	3.0		24.9	24.9	20.1	1.2	33.4	3.9	31.6	111.1	111.3			
Cycle Queue Clearance Time (g c), s	5.2	3.0		24.9	24.9	20.1	1.2	33.4	3.9	31.6	111.1	111.3			
Green Ratio (g/C)	0.04	0.04		0.13	0.13	0.28	0.59	0.57	0.70	0.16	0.72	0.72			
Capacity (c), veh/h	64	53		233	234	787	102	2927	1111	548	1361	1359			
Volume-to-Capacity Ratio (X)	0.650	0.433		0.862	0.862	0.394	0.230	0.573	0.189	0.899	0.858	0.860			
Back of Queue (Q), ft/ln (95 th percentile)	117.8	61.8		444.8	442.1	298.5	24.3	400.4	53.3	418.8	1355.5	1364.5			
Back of Queue (Q), veh/ln (95 th percentile)	4.7	2.5		17.5	17.5	11.8	1.0	15.8	2.1	16.5	53.8	53.7			
Queue Storage Ratio (RQ) (95 th percentile)	0.39	0.21		1.48	1.47	0.75	0.16	0.33	0.27	1.05	1.51	1.50			
Uniform Delay (d 1), s/veh	107.1	106.4		95.8	95.8	65.5	36.6	16.8	3.7	96.3	31.4	31.0			
Incremental Delay (d 2), s/veh	9.0	2.1		3.7	3.7	0.1	0.3	0.6	0.3	0.7	0.9	0.9			
Initial Queue Delay (d 3), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	116.1	108.4		99.5	99.5	65.6	36.9	17.4	4.0	97.0	32.3	31.9			
Level of Service (LOS)	F	F		F	F	E	D	B	A	F	C	C			
Approach Delay, s/veh / LOS	113.4		F	84.7		F	16.1		B	43.4		D			
Intersection Delay, s/veh / LOS	40.1						D								
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.63	C		2.64	C		2.43	B		1.89	B				
Bicycle LOS Score / LOS	0.59	A		1.66	B		1.56	B		2.97	C				

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information															
Agency	DBZ Traffic			Duration, h	0.250														
Analyst	DBZ			Analysis Date	Feb 25, 2021											Area Type	Other		
Jurisdiction				Time Period	PM Peak											PHF	0.96		
Urban Street	Bardstown Road			Analysis Year	2033 No Build											Analysis Period	1 > 4:45		
Intersection	Bartley/Wingfield			File Name	Bardstown PM 33 NB.xus														
Project Description	Cedar Creek Crossings																		
Demand Information				EB			WB			NB			SB						
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R				
Demand (v), veh/h	40	8	14	379	8	298	23	1697	206	503	2465	12							
Signal Information																			
Cycle, s	225.0	Reference Phase	2																
Offset, s	0	Reference Point	End																
Uncoordinated	No	Simult. Gap E/W	Off	Green	4.6	25.0	130.2	7.0	28.5	0.0									
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	3.5	4.7	3.6	3.6	0.0									
				Red	0.0	2.7	1.5	3.0	3.0	0.0									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT								
Assigned Phase					4		8	5	2	1	6								
Case Number					10.0		9.0	1.1	3.0	2.0	4.0								
Phase Duration, s					13.6		35.1	8.6	136.4	39.8	167.6								
Change Period, (Y+R _c), s					6.6		6.6	4.0	6.2	6.2	6.2								
Max Allow Headway (MAH), s					3.2		3.2	3.0	0.0	3.0	0.0								
Queue Clearance Time (g _s), s					7.2		26.9	3.2		32.6									
Green Extension Time (g _e), s					0.0		1.6	0.0	0.0	1.0	0.0								
Phase Call Probability					0.98		1.00	0.77		1.00									
Max Out Probability					1.00		0.00	0.00		0.00									
Movement Group Results				EB			WB			NB			SB						
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R	L	T	R				
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16							
Adjusted Flow Rate (v), veh/h	42	23		201	202	310	24	1742	212	477	1175	1175							
Adjusted Saturation Flow Rate (s), veh/h/ln	1795	1691		1781	1786	1403	1795	1698	1585	1730	1885	1882							
Queue Service Time (g _s), s	5.2	3.0		24.9	24.9	20.3	1.2	32.5	3.5	30.6	112.6	112.9							
Cycle Queue Clearance Time (g _c), s	5.2	3.0		24.9	24.9	20.3	1.2	32.5	3.5	30.6	112.6	112.9							
Green Ratio (g/C)	0.04	0.04		0.13	0.13	0.28	0.60	0.58	0.71	0.15	0.72	0.72							
Capacity (c), veh/h	64	53		233	234	775	101	2949	1118	532	1361	1359							
Volume-to-Capacity Ratio (X)	0.650	0.433		0.862	0.862	0.401	0.235	0.591	0.189	0.897	0.864	0.865							
Back of Queue (Q), ft/ln (95 th percentile)	117.8	61.8		444.8	442.1	300.1	25.6	365.8	47.6	407.2	1372.8	1382.8							
Back of Queue (Q), veh/ln (95 th percentile)	4.7	2.5		17.5	17.5	11.8	1.0	14.4	1.9	16.0	54.5	54.4							
Queue Storage Ratio (RQ) (95 th percentile)	0.39	0.21		1.48	1.47	0.75	0.17	0.30	0.24	1.02	1.53	1.52							
Uniform Delay (d ₁), s/veh	107.1	106.4		95.8	95.8	66.3	37.5	14.7	3.2	96.9	31.6	31.2							
Incremental Delay (d ₂), s/veh	9.0	2.1		3.7	3.7	0.1	0.3	0.6	0.3	0.6	1.0	1.0							
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0							
Control Delay (d), s/veh	116.1	108.4		99.5	99.5	66.4	37.8	15.3	3.5	97.5	32.6	32.2							
Level of Service (LOS)	F	F		F	F	E	D	B	A	F	C	C							
Approach Delay, s/veh / LOS	113.4		F	85.1		F	14.3		B	43.4		D							
Intersection Delay, s/veh / LOS				39.2						D									
Multimodal Results				EB			WB			NB			SB						
Pedestrian LOS Score / LOS	2.63		C	2.64		C	2.43		B	1.89		B							
Bicycle LOS Score / LOS	0.59		A	1.66		B	1.59		B	3.05		C							

HCS7 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	DBZ Traffic			Duration, h	0.250										
Analyst	DBZ			Analysis Date	Feb 25, 2021										
Jurisdiction				Area Type	Other										
Urban Street	Bardstown Road			Time Period	PM Peak										
Intersection	Bartley/Wingfield			PHF	0.96										
Project Description	Cedar Creek Crossings			Analysis Year	2033 Build										
	File Name	Bardstown PM 33 B.xus			Analysis Period	1> 4:45									
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	40	8	14	379	8	298	23	1728	206	503	2494	12			
Signal Information															
Cycle, s	225.0	Reference Phase	2	Green	4.6	24.7	130.5	7.0	28.5	0.0					
Offset, s	0	Reference Point	End	Yellow	4.0	3.5	4.7	3.6	3.6	0.0					
Uncoordinated	No	Simult. Gap E/W	Off	Red	0.0	2.7	1.5	3.0	3.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	1.1	3.0	2.0	4.0				
Phase Duration, s					13.6		35.1	8.6	136.7	39.5	167.6				
Change Period, (Y+R _c), s					6.6		6.6	4.0	6.2	6.2	6.2				
Max Allow Headway (MAH), s					3.2		3.2	3.0	0.0	3.0	0.0				
Queue Clearance Time (g _s), s					7.2		26.9	3.2		32.3					
Green Extension Time (g _e), s					0.0		1.6	0.0	0.0	1.0	0.0				
Phase Call Probability					0.98		1.00	0.77		1.00					
Max Out Probability					1.00		0.00	0.00		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	42	23		201	202	310	24	1772	211	473	1178	1178			
Adjusted Saturation Flow Rate (s), veh/h/ln	1795	1691		1781	1786	1403	1795	1698	1585	1730	1885	1882			
Queue Service Time (g _s), s	5.2	3.0		24.9	24.9	20.3	1.2	36.3	3.8	30.3	113.1	113.4			
Cycle Queue Clearance Time (g _c), s	5.2	3.0		24.9	24.9	20.3	1.2	36.3	3.8	30.3	113.1	113.4			
Green Ratio (g/C)	0.04	0.04		0.13	0.13	0.28	0.60	0.58	0.71	0.15	0.72	0.72			
Capacity (c), veh/h	64	53		233	234	771	100	2956	1120	528	1361	1359			
Volume-to-Capacity Ratio (X)	0.650	0.433		0.862	0.862	0.403	0.235	0.600	0.189	0.895	0.865	0.867			
Back of Queue (Q), ft/ln (95 th percentile)	117.8	61.8		444.8	442.1	300.6	25.8	424.2	51.9	404.3	1378.4	1388.4			
Back of Queue (Q), veh/ln (95 th percentile)	4.7	2.5		17.5	17.5	11.8	1.0	16.7	2.0	15.9	54.7	54.7			
Queue Storage Ratio (RQ) (95 th percentile)	0.39	0.21		1.48	1.47	0.75	0.17	0.35	0.26	1.01	1.53	1.53			
Uniform Delay (d ₁), s/veh	107.1	106.4		95.8	95.8	66.5	37.6	16.8	3.5	97.3	31.7	31.3			
Incremental Delay (d ₂), s/veh	9.0	2.1		3.7	3.7	0.1	0.3	0.6	0.2	0.6	1.0	1.0			
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	116.1	108.4		99.5	99.5	66.7	37.9	17.4	3.8	97.9	32.7	32.2			
Level of Service (LOS)	F	F		F	F	E	D	B	A	F	C	C			
Approach Delay, s/veh / LOS	113.4		F	85.2		F	16.2		B	43.4		D			
Intersection Delay, s/veh / LOS	39.8						D								
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.63		C	2.64		C	2.43		B	1.89		B			
Bicycle LOS Score / LOS	0.59		A	1.66		B	1.61		B	3.07		C			

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	North Entrance							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	3/29/21							East/West Street	Entrance							
Analysis Year	2023							North/South Street	Bardstown Road							
Time Analyzed	AM Peak							Peak Hour Factor	0.98							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Hogan Bardstown															
Lanes																
<p>Major Street: North-South</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	0	1		0	0	0	0	1	3	0	0	0	2	1
Configuration		L		R						L	T				T	R
Volume (veh/h)		60		5					0	13	2662				1234	8
Percent Heavy Vehicles (%)		1		1					3	1						
Proportion Time Blocked		0.580		0.190						0.190						
Percent Grade (%)	0															
Right Turn Channelized	No															
Median Type Storage	Left Only								2							
Critical and Follow-up Headways																
Base Critical Headway (sec)		6.4		6.9						4.1						
Critical Headway (sec)		5.72		6.92						4.12						
Base Follow-Up Headway (sec)		3.8		3.3						2.2						
Follow-Up Headway (sec)		3.81		3.31						2.21						
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)		61		5						13						
Capacity, c (veh/h)		190		791						634						
v/c Ratio		0.32		0.01						0.02						
95% Queue Length, Q ₉₅ (veh)		1.3		0.0						0.1						
Control Delay (s/veh)		32.7		9.6						10.8						
Level of Service (LOS)		D		A						B						
Approach Delay (s/veh)	30.9								0.1							
Approach LOS	D															

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	North Entrance							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	3/29/2021							East/West Street	Entrance							
Analysis Year	2033							North/South Street	Bardstown Road							
Time Analyzed	AM Peak							Peak Hour Factor	0.98							
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25							
Project Description	Hogan Bardstown															
Lanes																
<p style="text-align: center;">Major Street: North-South</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	0	1		0	0	0	0	1	3	0	0	0	2	1
Configuration		L		R						L	T				T	R
Volume (veh/h)		60		5					0	13	2800				1296	8
Percent Heavy Vehicles (%)		1		1					3	1						
Proportion Time Blocked		0.670		0.190						0.190						
Percent Grade (%)	0															
Right Turn Channelized	No								No							
Median Type Storage	Left Only								2							
Critical and Follow-up Headways																
Base Critical Headway (sec)		6.4		6.9						4.1						
Critical Headway (sec)		5.72		6.92						4.12						
Base Follow-Up Headway (sec)		3.8		3.3						2.2						
Follow-Up Headway (sec)		3.81		3.31						2.21						
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)		61		5						13						
Capacity, c (veh/h)		174		747						593						
v/c Ratio		0.35		0.01						0.02						
95% Queue Length, Q ₉₅ (veh)		1.5		0.0						0.1						
Control Delay (s/veh)		36.6		9.9						11.2						
Level of Service (LOS)		E		A						B						
Approach Delay (s/veh)	34.6								0.1							
Approach LOS	D															

HCS7 Two-Way Stop-Control Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	North Entrance									
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction										
Date Performed	2/22/2021							East/West Street	Entrance									
Analysis Year	2023							North/South Street	Bardstown Road									
Time Analyzed	PM Peak							Peak Hour Factor	0.98									
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25									
Project Description	Hogan Bardstown																	
Lanes																		
<p>Major Street: North-South</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		1	0	1		0	0	0	0	1	3	0	0	0	2	1		
Configuration		L		R						L	T				T	R		
Volume (veh/h)		5		5					0	15	1867				2738	31		
Percent Heavy Vehicles (%)		1		1					3	1								
Proportion Time Blocked		0.720		0.720						0.720								
Percent Grade (%)		0																
Right Turn Channelized		No												No				
Median Type Storage		Left Only									2							
Critical and Follow-up Headways																		
Base Critical Headway (sec)		6.4		6.9						4.1								
Critical Headway (sec)		5.72		6.92						4.12								
Base Follow-Up Headway (sec)		3.8		3.3						2.2								
Follow-Up Headway (sec)		3.81		3.31						2.21								
Delay, Queue Length, and Level of Service																		
Flow Rate, v (veh/h)		5		5						15								
Capacity, c (veh/h)		17		305						57								
v/c Ratio		0.30		0.02						0.27								
95% Queue Length, Q ₉₅ (veh)		0.8		0.1						0.9								
Control Delay (s/veh)		291.0		17.0						89.7								
Level of Service (LOS)		F		C						F								
Approach Delay (s/veh)		154.0								0.7								
Approach LOS		F																

HCS7 Two-Way Stop-Control Report																		
General Information								Site Information										
Analyst	DBZ							Intersection	North Entrance									
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction										
Date Performed	2/25/2021							East/West Street	Entrance									
Analysis Year	2033							North/South Street	Bardstown Road									
Time Analyzed	PM Peak							Peak Hour Factor	0.98									
Intersection Orientation	North-South							Analysis Time Period (hrs)	0.25									
Project Description	Hogan Bardstown																	
Lanes																		
<p>Major Street: North-South</p>																		
Vehicle Volumes and Adjustments																		
Approach	Eastbound				Westbound				Northbound				Southbound					
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		1	0	1		0	0	0	0	1	3	0	0	0	2	1		
Configuration		L		R						L	T				T	R		
Volume (veh/h)		5		5					0	15	1962				2898	29		
Percent Heavy Vehicles (%)		1		1					3	1								
Proportion Time Blocked		0.720		0.720						0.720								
Percent Grade (%)		0																
Right Turn Channelized		No												No				
Median Type Storage		Left Only									2							
Critical and Follow-up Headways																		
Base Critical Headway (sec)		6.4		6.9						4.1								
Critical Headway (sec)		5.72		6.92						4.12								
Base Follow-Up Headway (sec)		3.8		3.3						2.2								
Follow-Up Headway (sec)		3.81		3.31						2.21								
Delay, Queue Length, and Level of Service																		
Flow Rate, v (veh/h)		5		5						15								
Capacity, c (veh/h)		8		305						34								
v/c Ratio		0.64		0.02						0.45								
95% Queue Length, Q ₉₅ (veh)		1.2		0.1						1.5								
Control Delay (s/veh)		746.2		17.0						181.4								
Level of Service (LOS)		F		C						F								
Approach Delay (s/veh)		381.6									1.4							
Approach LOS		F																

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Cedar Creek Entrance							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	3/29/21							East/West Street	Cedar Creek Road							
Analysis Year	2023							North/South Street	Entrance							
Time Analyzed	AM Peak							Peak Hour Factor	0.92							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Cedar Creek Crossings															
Lanes																
<p style="text-align: center;">Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	0		0	0	0		1	0	1
Configuration		L	T					TR						L		R
Volume (veh/h)		21	354				119	138						74		11
Percent Heavy Vehicles (%)		1												1		1
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized													No			
Median Type Storage	Undivided															
Critical and Follow-up Headways																
Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.11												6.41		6.21
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.21												3.51		3.31
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)		23												80		12
Capacity, c (veh/h)		1289												436		839
v/c Ratio		0.02												0.18		0.01
95% Queue Length, Q ₉₅ (veh)		0.1												0.7		0.0
Control Delay (s/veh)		7.8												15.1		9.4
Level of Service (LOS)		A												C		A
Approach Delay (s/veh)	0.4								14.4							
Approach LOS									B							

HCS7 Two-Way Stop-Control Report																
General Information								Site Information								
Analyst	DBZ							Intersection	Cedar Creek Entrance							
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction								
Date Performed	3/29/2021							East/West Street	Cedar Creek Road							
Analysis Year	2033							North/South Street	Entrance							
Time Analyzed	AM Peak							Peak Hour Factor	0.92							
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25							
Project Description	Cedar Creek Crossings															
Lanes																
<p style="text-align: center;">Major Street: East-West</p>																
Vehicle Volumes and Adjustments																
Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	0	1	0	0	0	0		1	0	1	
Configuration		L	T					TR					L		R	
Volume (veh/h)		21	381				125	138					74		11	
Percent Heavy Vehicles (%)		1											1		1	
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized													No			
Median Type Storage	Undivided															
Critical and Follow-up Headways																
Base Critical Headway (sec)		4.1											7.1		6.2	
Critical Headway (sec)		4.11											6.41		6.21	
Base Follow-Up Headway (sec)		2.2											3.5		3.3	
Follow-Up Headway (sec)		2.21											3.51		3.31	
Delay, Queue Length, and Level of Service																
Flow Rate, v (veh/h)		23											80		12	
Capacity, c (veh/h)		1282											416		832	
v/c Ratio		0.02											0.19		0.01	
95% Queue Length, Q ₉₅ (veh)		0.1											0.7		0.0	
Control Delay (s/veh)		7.9											15.7		9.4	
Level of Service (LOS)		A											C		A	
Approach Delay (s/veh)	0.4												14.9			
Approach LOS													B			

HCS7 Two-Way Stop-Control Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Cedar Creek Entrance								
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction									
Date Performed	2/22/2021							East/West Street	Cedar Creek Road								
Analysis Year	2023							North/South Street	Entrance								
Time Analyzed	PM Peak							Peak Hour Factor	0.92								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Cedar Creek Crossings																
Lanes																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	0	0	0	0		1	0	1		
Configuration		L	T					TR					L		R		
Volume (veh/h)		21	247				333	128					136		15		
Percent Heavy Vehicles (%)		1											1		1		
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized													No				
Median Type Storage	Undivided																
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1											7.1		6.2		
Critical Headway (sec)		4.11											6.41		6.21		
Base Follow-Up Headway (sec)		2.2											3.5		3.3		
Follow-Up Headway (sec)		2.21											3.51		3.31		
Delay, Queue Length, and Level of Service																	
Flow Rate, v (veh/h)		23											148		16		
Capacity, c (veh/h)		1068											374		626		
v/c Ratio		0.02											0.39		0.03		
95% Queue Length, Q ₉₅ (veh)		0.1											1.8		0.1		
Control Delay (s/veh)		8.4											20.7		10.9		
Level of Service (LOS)		A											C		B		
Approach Delay (s/veh)		0.7												19.8			
Approach LOS														C			

HCS7 Two-Way Stop-Control Report																	
General Information								Site Information									
Analyst	DBZ							Intersection	Cedar Creek Entrance								
Agency/Co.	Diane B Zimmerman Traffic Engineering							Jurisdiction									
Date Performed	2/25/2021							East/West Street	Cedar Creek Road								
Analysis Year	2033							North/South Street	Entrance								
Time Analyzed	PM Peak							Peak Hour Factor	0.92								
Intersection Orientation	East-West							Analysis Time Period (hrs)	0.25								
Project Description	Cedar Creek Crossings																
Lanes																	
<p>Major Street: East-West</p>																	
Vehicle Volumes and Adjustments																	
Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6			7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	0			0	0	0		1	0	1
Configuration		L	T					TR						L		R	
Volume (veh/h)		21	266				350	128						136		15	
Percent Heavy Vehicles (%)		1												1		1	
Proportion Time Blocked																	
Percent Grade (%)														0			
Right Turn Channelized														No			
Median Type Storage	Undivided																
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1												7.1		6.2	
Critical Headway (sec)		4.11												6.41		6.21	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.21												3.51		3.31	
Delay, Queue Length, and Level of Service																	
Flow Rate, v (veh/h)		23												148		16	
Capacity, c (veh/h)		1052												355		611	
v/c Ratio		0.02												0.42		0.03	
95% Queue Length, Q ₉₅ (veh)		0.1												2.0		0.1	
Control Delay (s/veh)		8.5												22.2		11.1	
Level of Service (LOS)		A												C		B	
Approach Delay (s/veh)	0.6												21.1				
Approach LOS													C				