

Docket No. 15SUBDIV1003

Conservation Subdivision on 122 acres on already zoned
R-4 property located at 13605 and 13615 Factory Lane

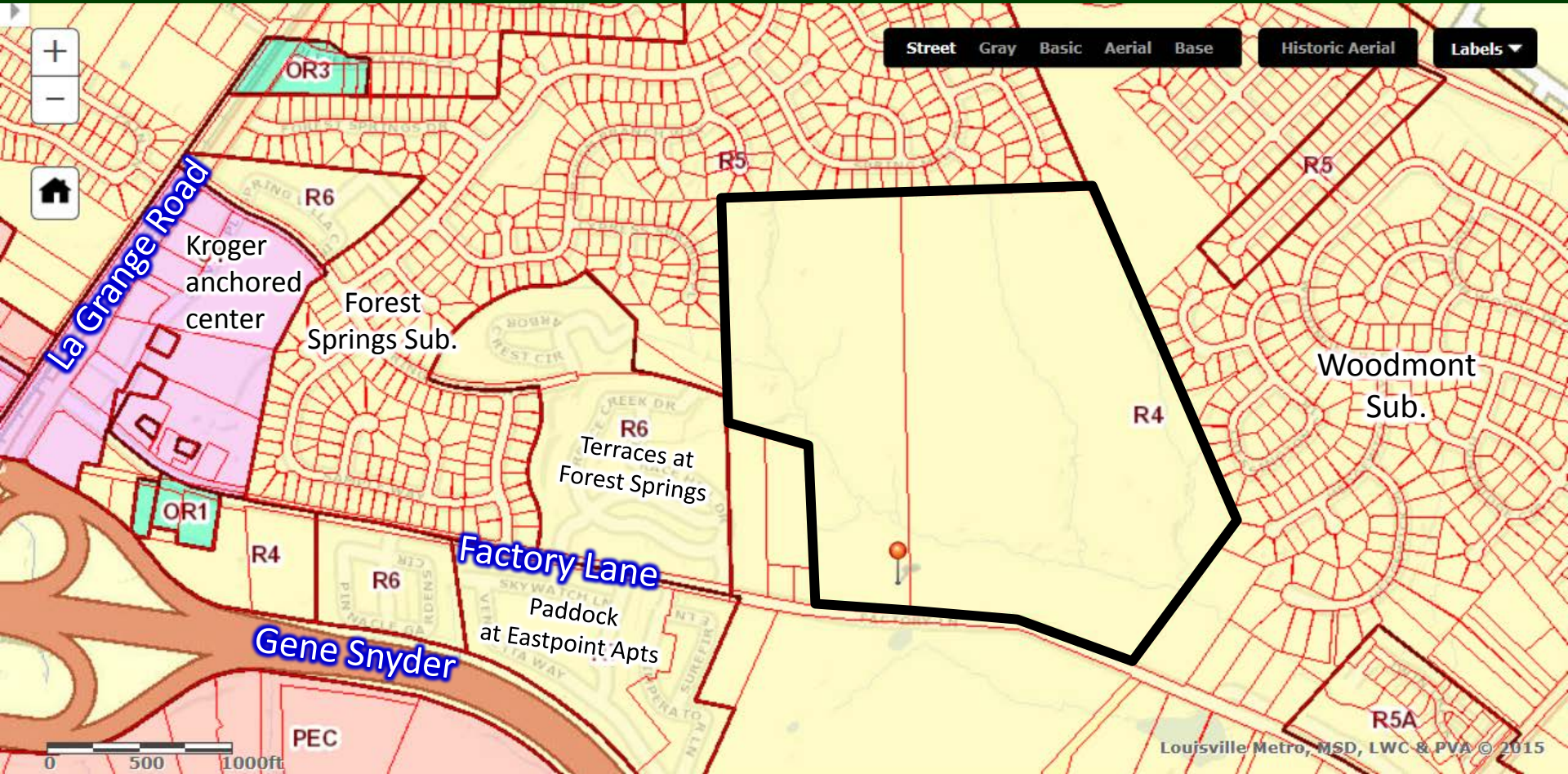


Attorneys: Bardenwerper Talbott & Roberts, PLLC
Land Planners, Landscape Architects and Engineers: Sabak Wilson & Lingo
Traffic Engineer: Diane B. Zimmerman Traffic Engineering, LLC

Index

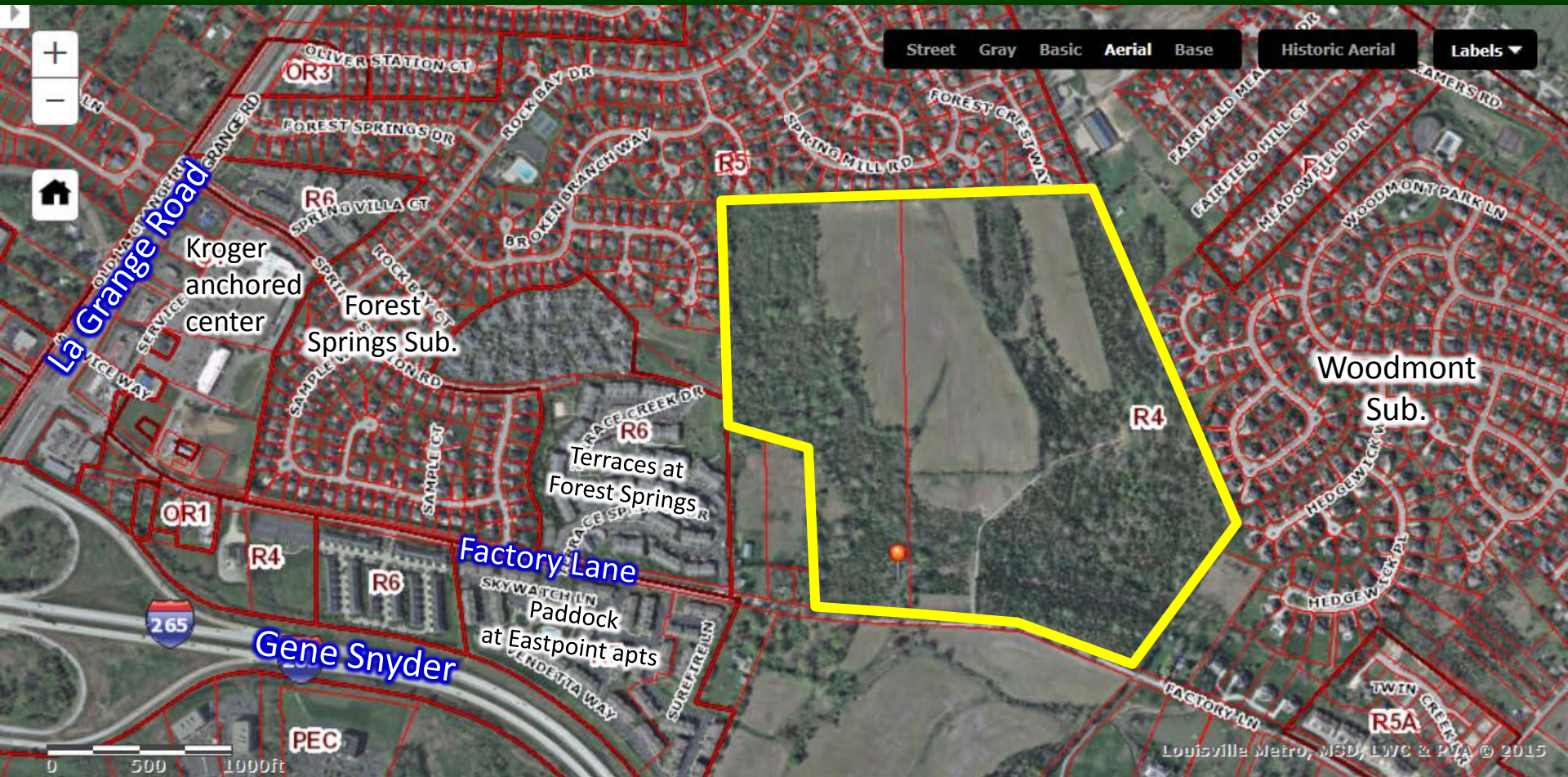
1. LOJIC Zoning Map
2. Aerial photographs of the site and surrounding area
3. Explanation of progression of development in this east Louisville Metro area, leading to development of remaining infill sites, notably the St. Joseph Catholic Orphan Society properties
4. 3/17/15 neighborhood meeting notice list map, letter to neighbors inviting them to the meeting and summary of meeting
5. Ground level photographs of the site and surrounding area
6. Environmental Resource Map (and infrastructure plan)
7. Conservation Subdivision Plan
8. Photographs of proposed home types
9. Traffic Study
10. Statutory/Case Law regarding contents of subdivision regulations and review of subdivisions
11. Written justification, in addition to Conservation Plan and exhibits, demonstrating compliance with LDC Section 7.11.4.B.9 requirement

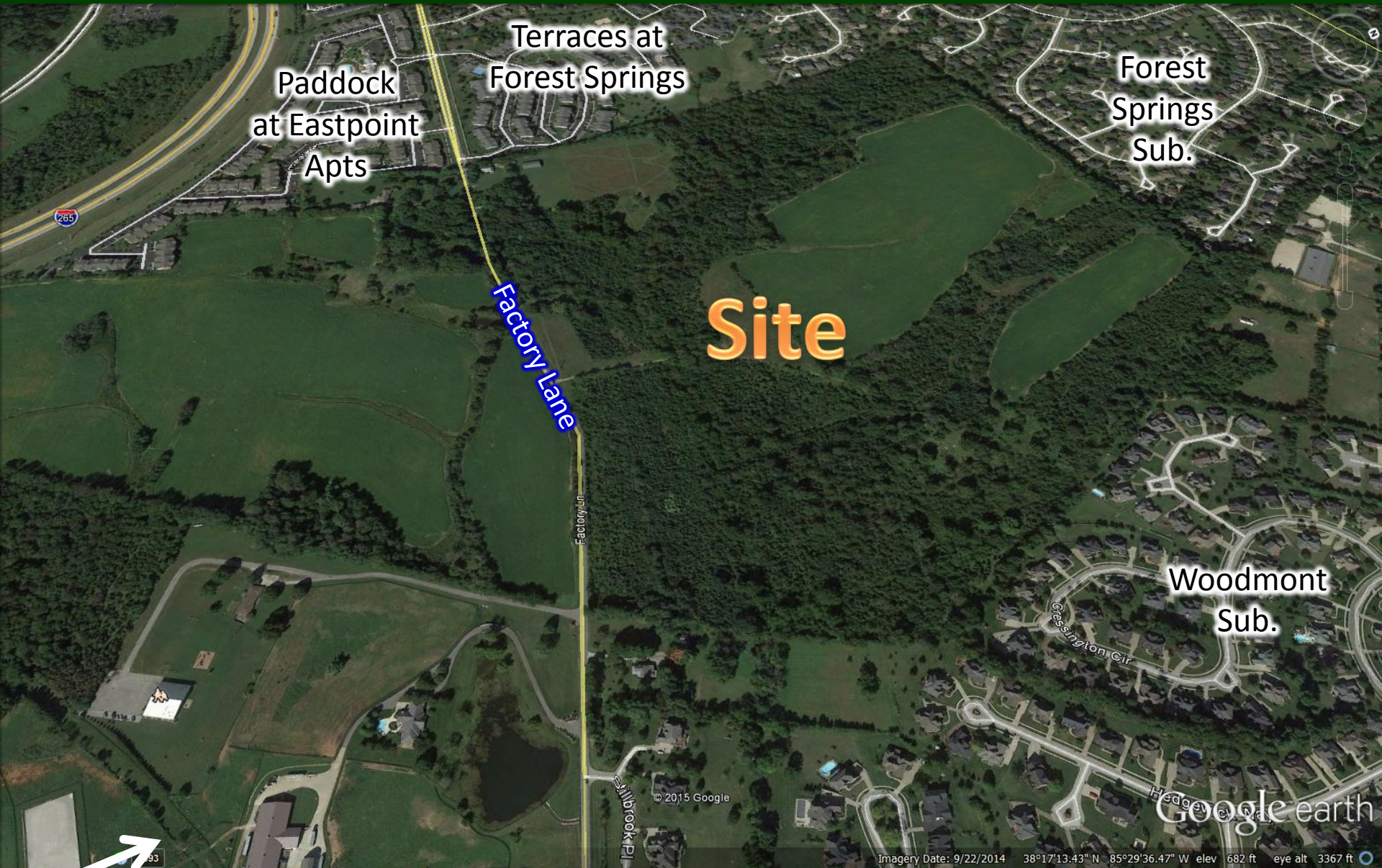
Tab 1 LOJIC Zoning Map



Tab 2

Aerial photograph of the site and
surrounding area





N

Aerial view looking west towards site.



Woodmont
Sub.

Site

**(approx 1/2 of St. Joseph
Catholic Orphan Society
property)**

**approx other
1/2 of St. Joseph
Catholic
Orphan Society
property**

Factory Lane

Terraces at
Forest Springs

Paddock
at Eastpoint
Apts

Bellisima Pl
Google earth

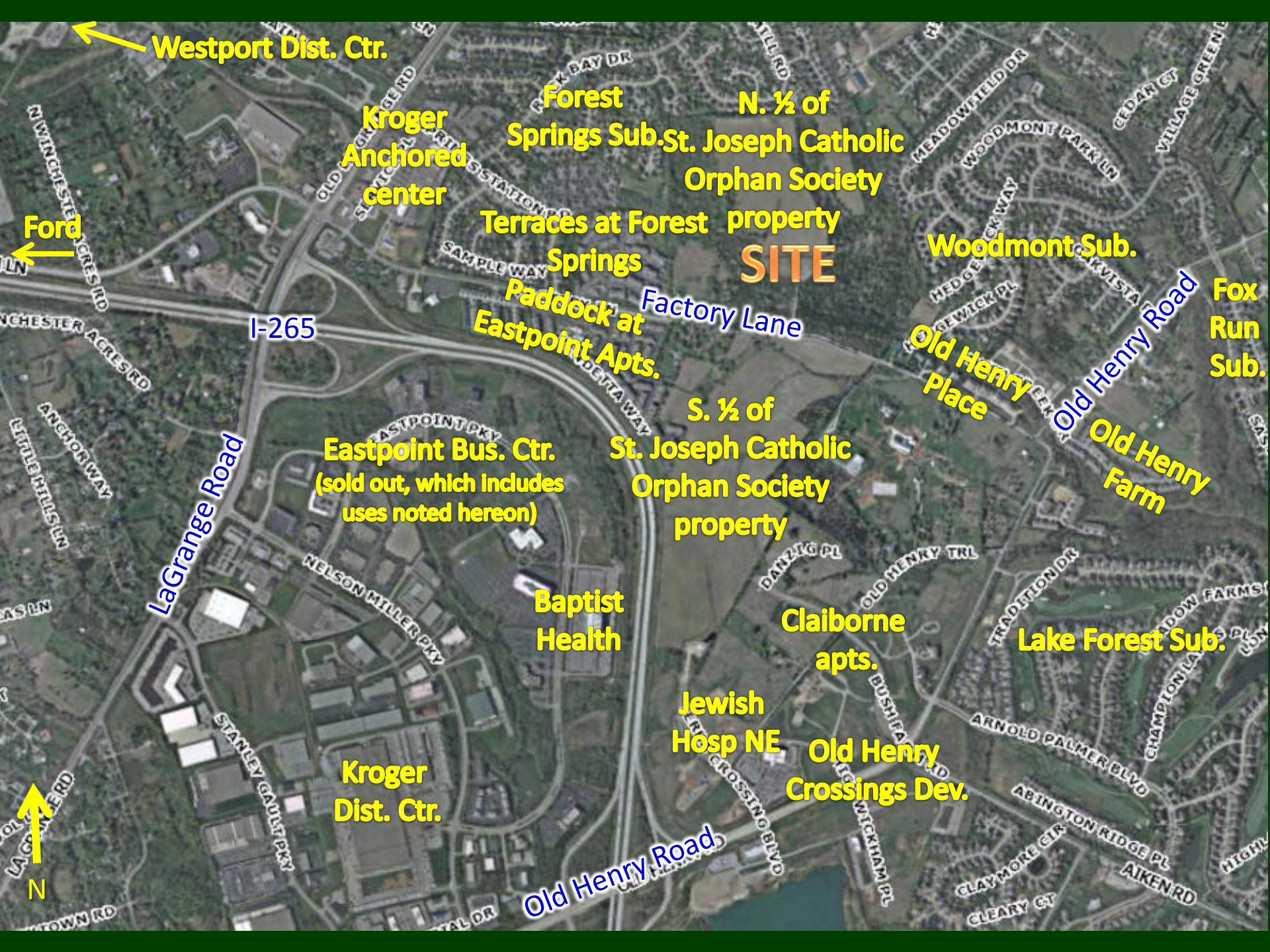
Imagery Date: 9/22/2014 38°17'19.70" N 85°29'52.31" W elev 699 ft eye alt 2708 ft



Aerial view looking east towards site.

Tab 3

Explanation of progression of development in this east Louisville Metro area, leading to development of remaining infill sites, notably the St. Joseph Catholic Orphan Society properties



Westport Dist. Ctr.

Ford

Kroger Anchored center

Forest Springs Sub. St. Joseph Catholic Orphan Society

SITE

Terraces at Forest Springs

Woodmont Sub.

Paddock at Eastpoint Apts.

Factory Lane

Fox Run Sub.

I-265

Old Henry Place

Old Henry Road

Eastpoint Bus. Ctr. (sold out, which includes uses noted hereon)

S. 1/2 of St. Joseph Catholic Orphan Society property

Old Henry Farm

LaGrange Road

Baptist Health

Claiborne apts.

Lake Forest Sub.

Jewish Hosp NE

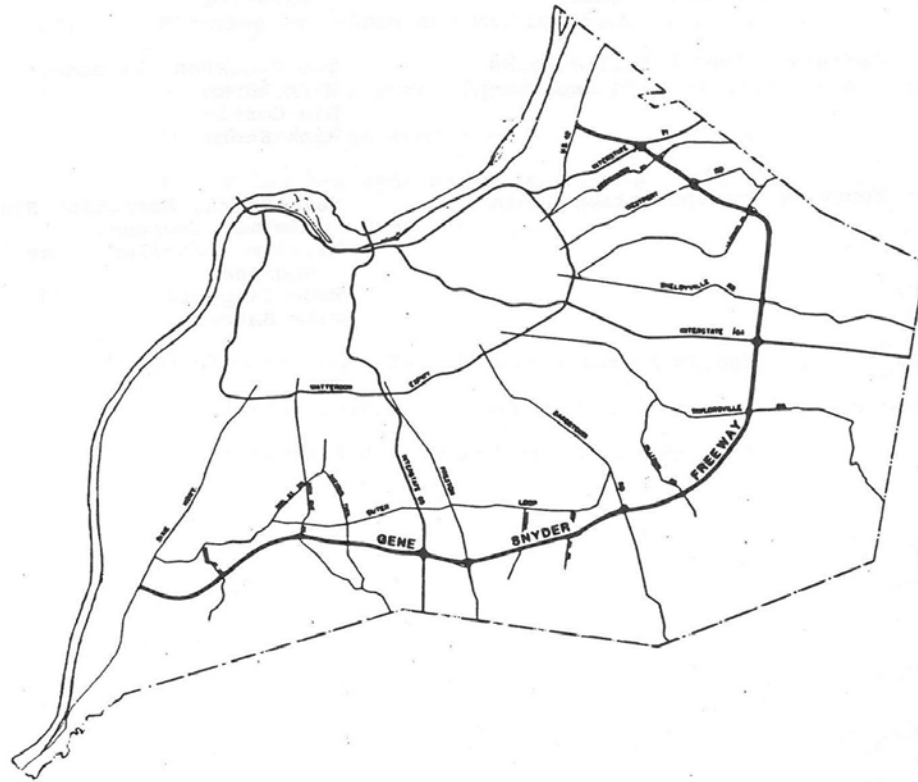
Old Henry Crossings Dev.

Kroger Dist. Ctr.

Old Henry Road

N

SNYDER FREEWAY CORRIDOR STUDY:



Louisville and Jefferson County
Planning Commission

May, 1989



I-265

Street Gray Basic Aerial

Base

**Eastpoint Business Center
beg. @ 1989**

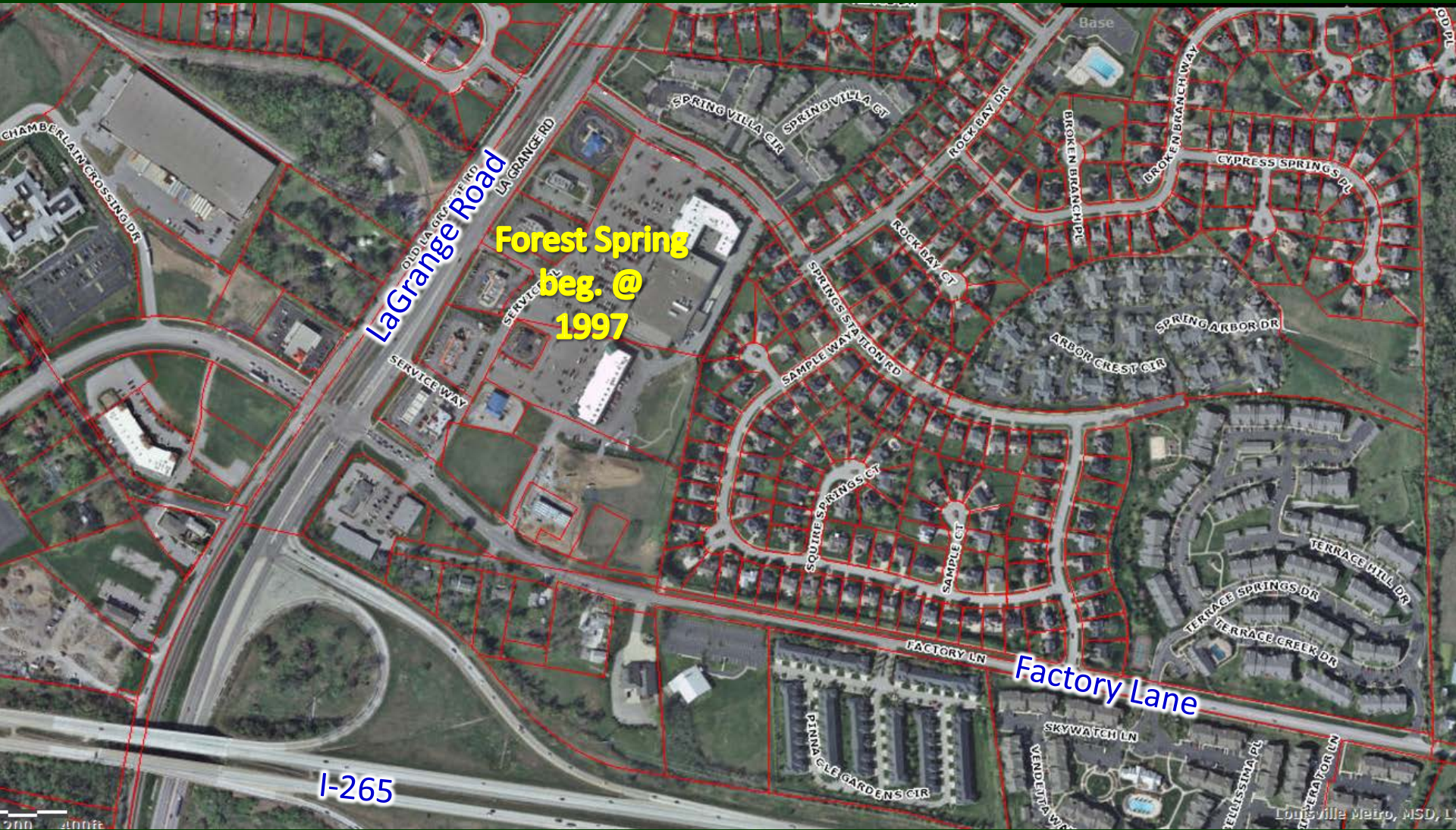
Old LaGrange Rd

Nelson Miller Parkway

Stanley Gault Parkway

Old Henry Road

Louisville



OLD LA GRANGE RD
LaGrange Road

**Forest Spring
beg. @
1997**

Factory Lane

I-265

0 200 400ft

Louisville Metro, MSD, L

SITE

**Woodmont Subdivision
beg. @ 1998**

Factory Lane

Old Henry Road

MEADOW CREST CT

SPRING HILL PL

FOREST CREST WAY

FAIRFIELD MEADOWS DR

FAIRFIELD HILL CT

MEADOWFIELD DR

REAMERS RD

CEDAR CT

HILLSIDE

VILLAGE GR

WOODMONT PARKS LN

HEDGEWICK WAY

HEDGEWICK PL

OAKWISTA PL

OAKWISTA WAY

EK DR

INGS DR

SUREFIRE LN

FACTORY LN

CYPRESS GLEN DR

TWIN CREEK DR

RIDGE BR



LaGrange Road

Forest Springs
Sub. beg. @ 1998

Factory Lane

I-265

200 400ft

Louisville Metro, MSD, L

CHAMBERLAIN CROSSING DR

OLD LA GRANGE RD

SERVICE PL

SERVICE WAY

SPRING VILLA CIR

SPRING VILLA CT

ROCK BAY DR

BROKEN BRANCH PL

BROKEN BRANCH WAY

CYPRESS SPRINGS PL

SPRINGS STATION RD

ROCK BAY CT

ARBOR CREST CIR

SPRING ARBOR DR

SAMPLE WAY

SOUTHERN SPRINGS

SAMPLE CT

SPRING ARBOR DR

TERRACE HILL DR

TERRACE SPRINGS DR

TERRACE CREEK DR

FACTORY LN

PINNACLE GARDENS CIR

SKYWATCH LN

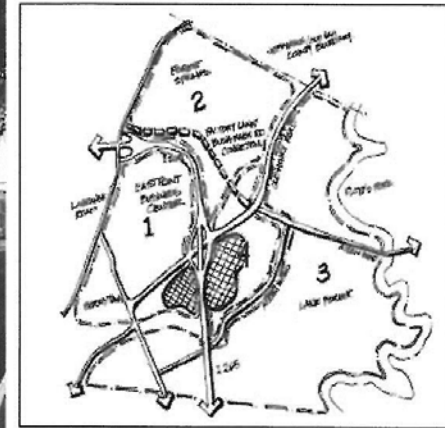
VENDETTA WAY

TELLISSIMA PL

GENERATOR LN

OLD HENRY ROAD

Subarea Plan

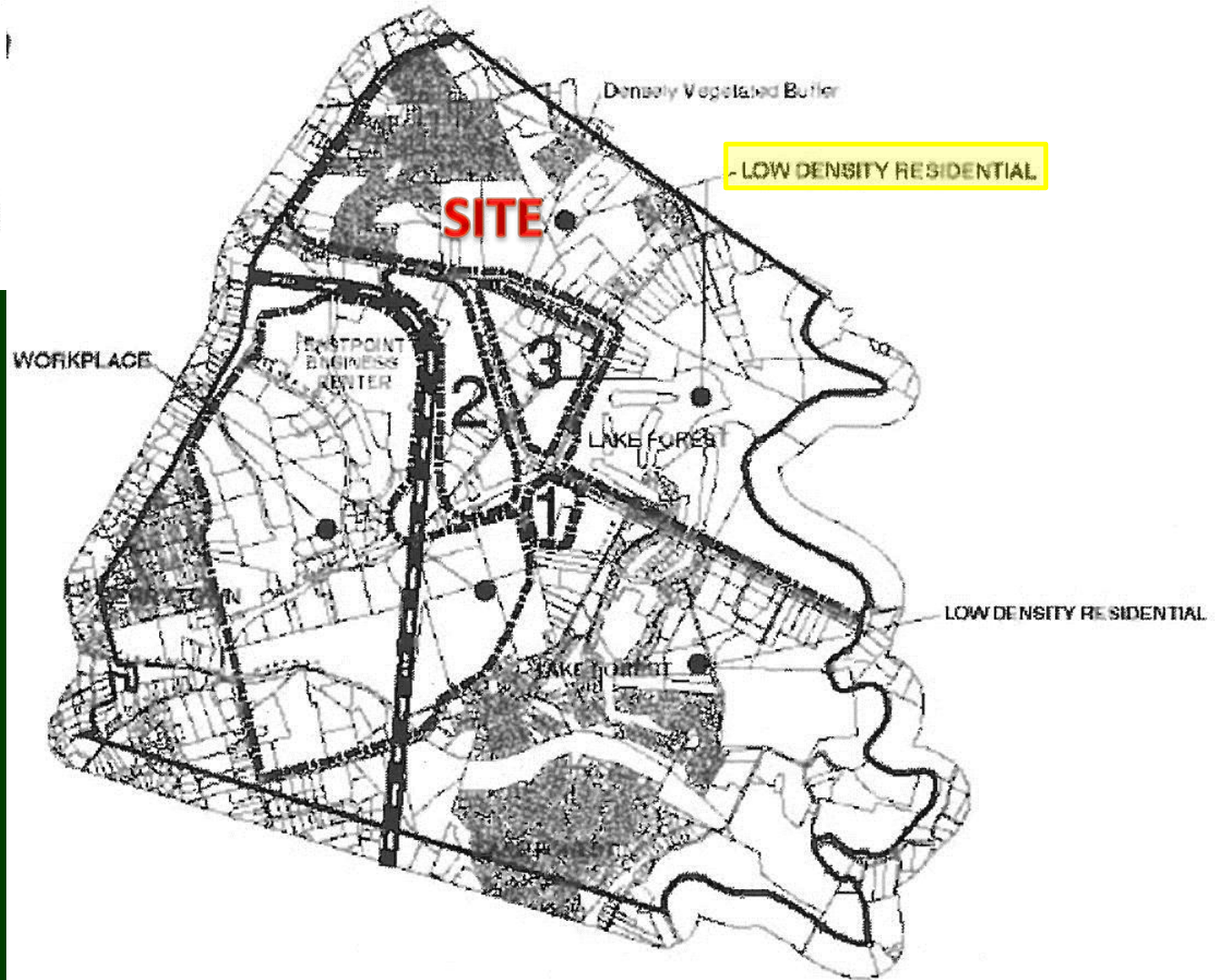


FINAL DRAFT
MARCH, 2000
PLANNING COMMISSION
TRANSMITTAL FOR FISCAL COURT

OLD HENRY SUBAREA PLAN

Figure 10:
Recommended Land Use Plan

- 1 Neighborhood Activity Center
- 2 Workplace
- 3 Residential (Low to Medium)



1-265
interchange

Old Henry Road

Quarry site



Old Henry Crossing Master Plan



**Old Henry Crossing
Beg. @ early 2000s**

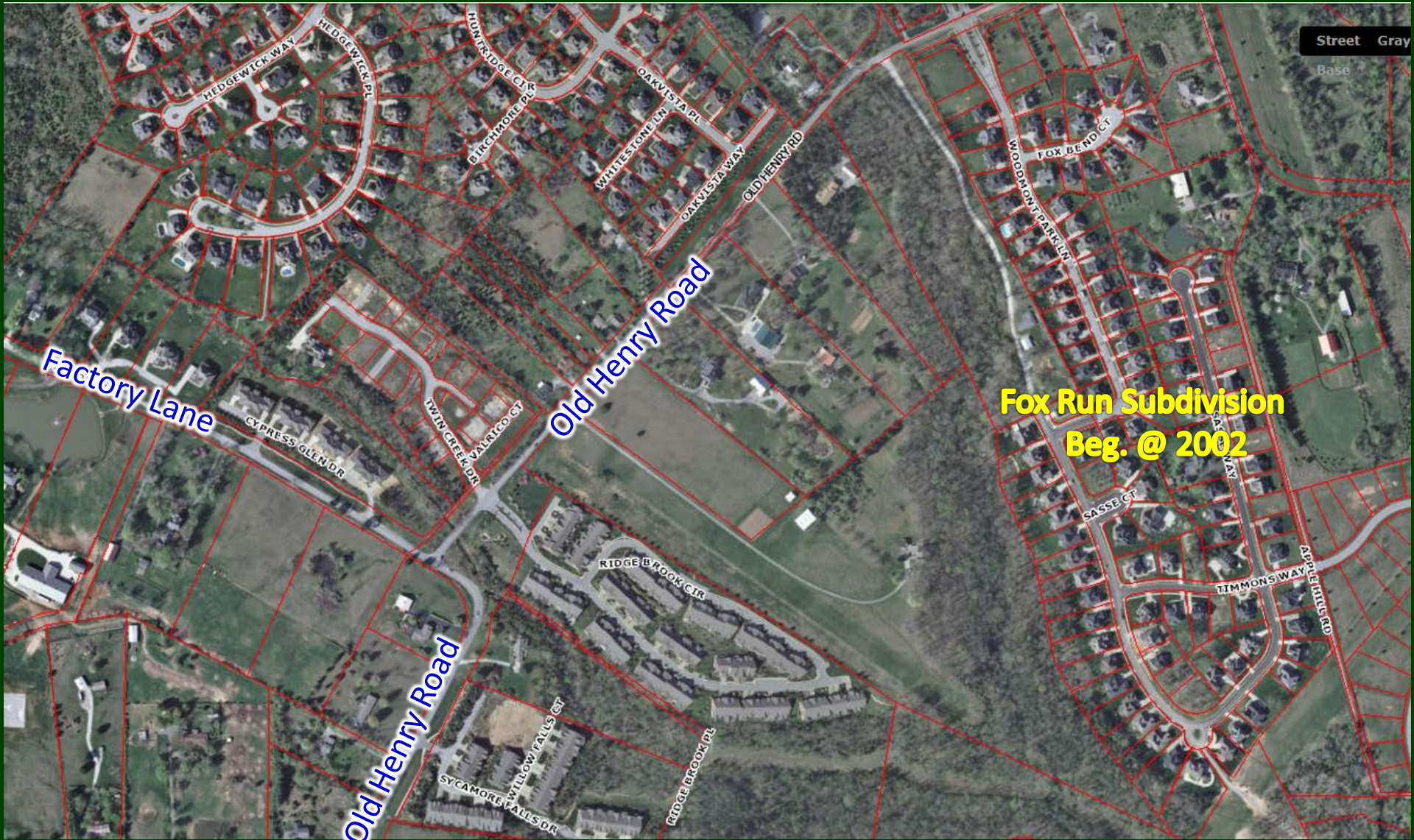
*Signature Entrance for Hamilton Springs
Residential Section – Lot 3*



Factory Lane

Paddock at Eastpoint Apts
beg. @2001





Street Gray
Base

Factory Lane

Old Henry Road

Old Henry Road

Fox Run Subdivision
Beg. @ 2002

HEDGEWICK WAY
HEDGEWICK PL

TOWARDRIDGE CIR
BIRCHMORE PL

OAKVISTA PL
WINTESTON LN

OAKVISTA WAY
OLD HENRY RD

WOODMONT PARK LN
FOX BEND CT

CYPRESS GLEN DR

TWIN CREEK DR
PARICO CT

RIDGE BROOK CIR

SYCAMORE FALLS DR
WILLOW FALLS CT

RIDGE BROOK PL

SASSE CT

TIMMONS WAY
POPPLE HILL DR



Retail Ctr.
beg @
2002

Retail Ctr.
beg @
2002

LaGrange Road

Factory Lane

I-265

200 400ft

Louisville Metro, MSD, L



**Claiborne Apts
beg. @ 2007**



Old Henry Road



**Jewish Hospital Northeast
beg. @ 2009**

Old Henry Road



I-265

Street Gray Basic Aerial

Base



Old LaGrange Rd

Nelson Miller Parkway

Stanley Gault Parkway

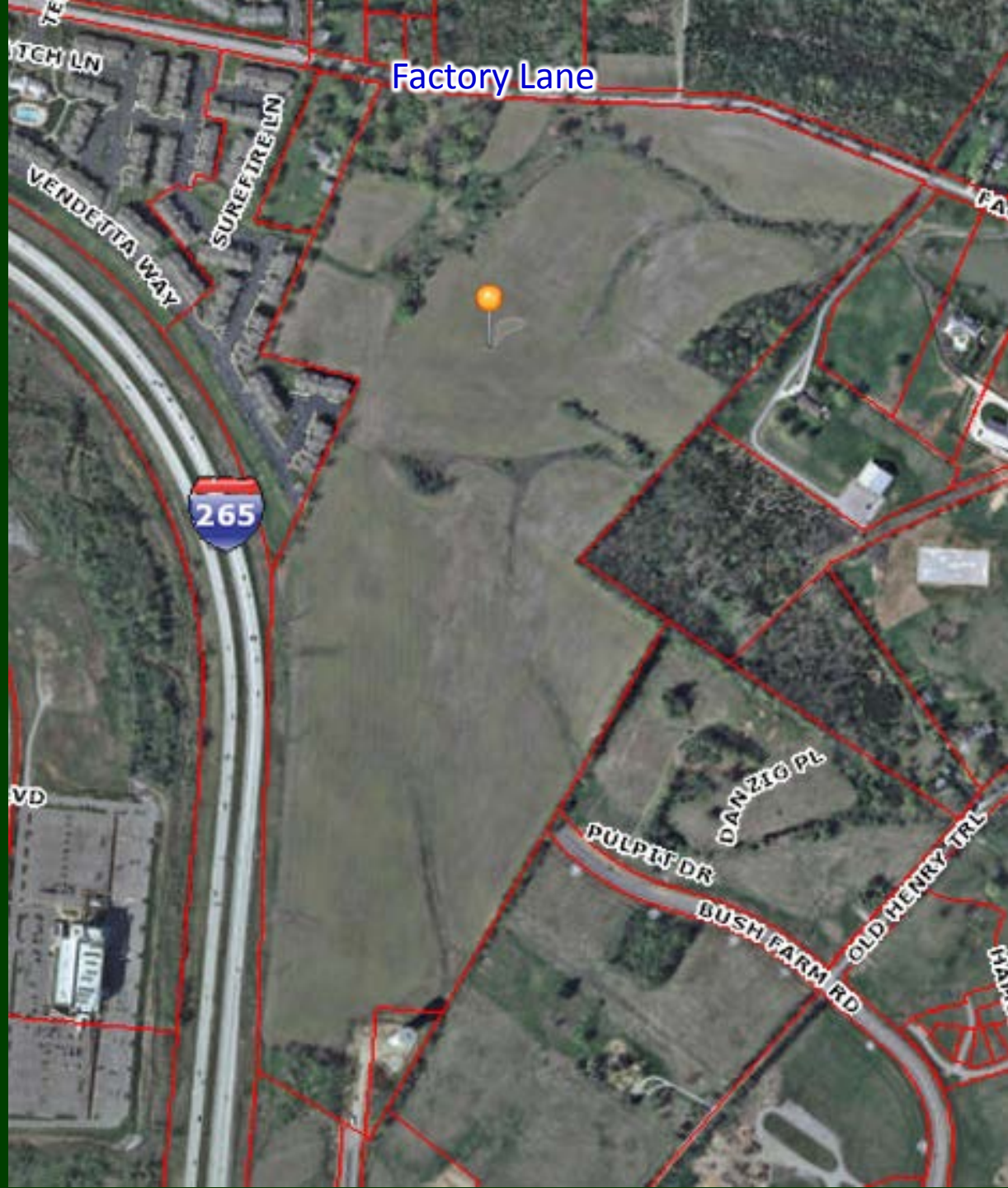
Bapt. Heath
Beg. @ 2009

Old Henry Road

Louisville

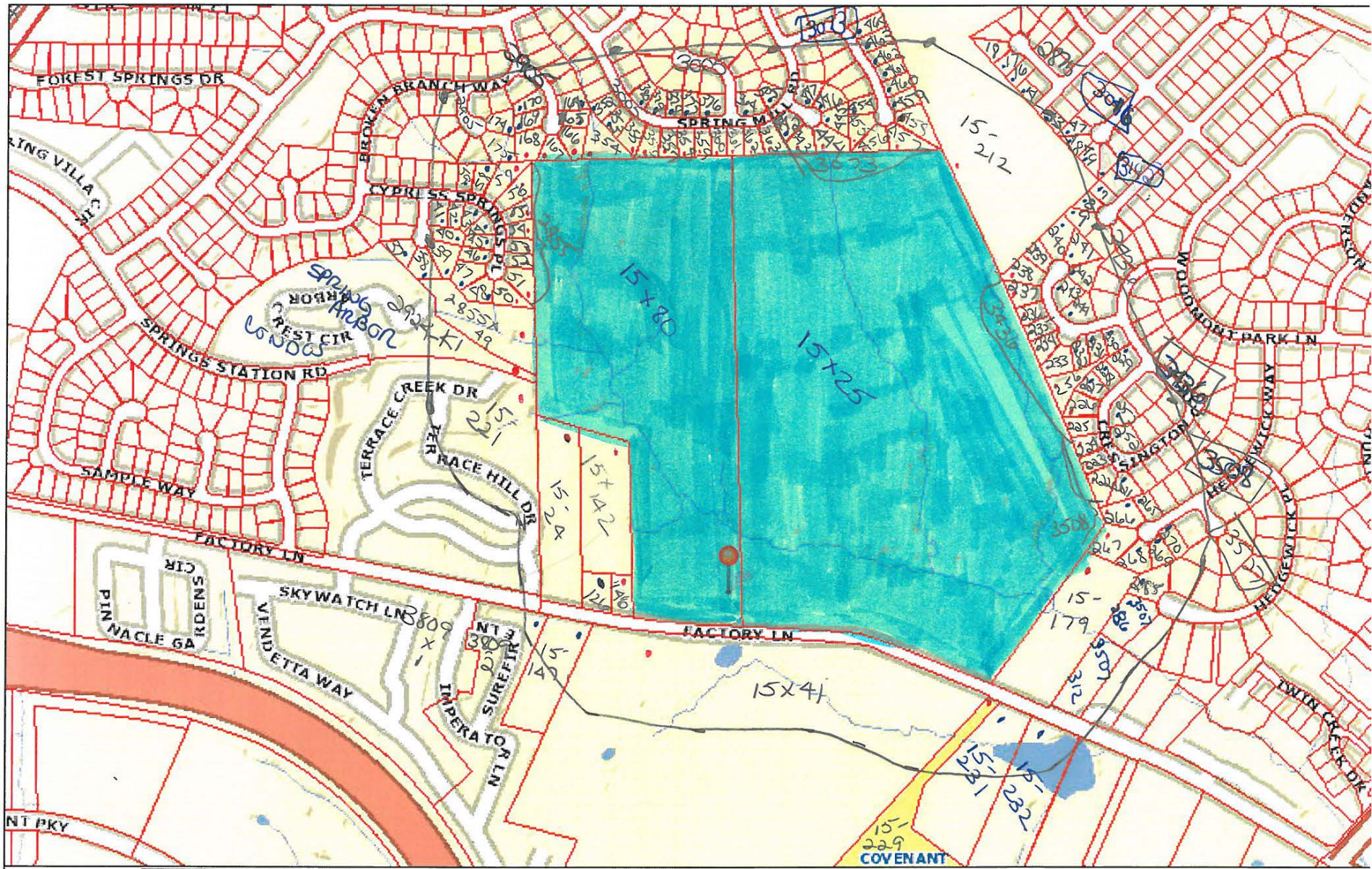
Southern half of St. Joseph Catholic Orphan Society property

(development, including Bush
Farm Road extension pending for
2016 review)



Tab 4

3/17/15 neighborhood meeting notice
list map, letter to neighbor inviting
them to the meeting and summary of
meeting



0 500 1000ft



3609 Walden Drive
Lexington, KY 40517
Phone: 859-268-1191

March 3, 2015

Re: Proposed Conservation Subdivision on a portion of the vacant St. Joseph Orphanage land, specifically 122 acres located at 13605 & 13615 Factory Lane

Dear Neighbor,

We are writing to invite you to a meeting we have scheduled to present neighbors with our plan for a conservation subdivision with 408 lots to be located as above.

This meeting will be held on Tuesday, March 17th, at 7:00 p.m. at The Community Presbyterian Church located at 13902 Factory Lane to discuss our plan with you.

As a neighbor, you are invited to this meeting so that we may describe our plan and address questions or concerns that you or your neighbors may have.

If you cannot attend the meeting but have questions or concerns; please call me, our attorney Bill Bardenwerper at 426-6688, or our land planning and engineering firm representative Kelli Jones at 584-6271.

We look forward to seeing you.

Sincerely,

Rena Wiseman

Associate General Counsel/Development Manager

Cc: Hon. Glen Stuckel, District 17, Metro Councilman
Joe Reverman, Planning Manager with Department of Planning & Design Services
Bill Bardenwerper attorney with Bardenwerper, Talbott & Roberts, PLLC
Kelli Jones, RLA/ASLA, land planner with Sabak, Wilson & Lingo, Inc.

Ball Homes/St. Joe's Property Neighbor Meeting

The neighborhood meeting was called to order by local zoning attorney Bill Bardenwerper at 7 PM on March 17, 2015 at the Community Presbyterian Church on Factory Lane close to the subject property. People invited included more than just two tiers of adjoining property owners but actually multiple tiers, thus a large crowd was present. Among those present were Ball Homes representative Rocco Pigneri and land planner/landscape architect Kelli Jones was the land planning/landscape architecture/engineering firm of Sabak, Wilson and Lingo.

Mr. Bardenwerper began the meeting with a PowerPoint presentation. It showed aerial photographs of the site and area, including farther-out and nearer-in views, in order that neighbors could determine exactly where their homes are located relative to the subject property. Also the proposed subdivision plan was shown, as were plans of the site's varied environmental features.

Mr. Bardenwerper explained the conservation subdivision plan regulatory process and how it involves a ministerial, not discretionary, review. He explained how the plan was designed and why it lays out the way it does with lots as shown. Ms. Jones went into greater detail about plan details. Mr. Pigneri explained home designs based on the elevation renderings that Mr. Bardenwerper showed. Diane Zimmerman, transportation engineer with Jacobs group, explained expected traffic generation and distribution at the morning and evening peak hours.

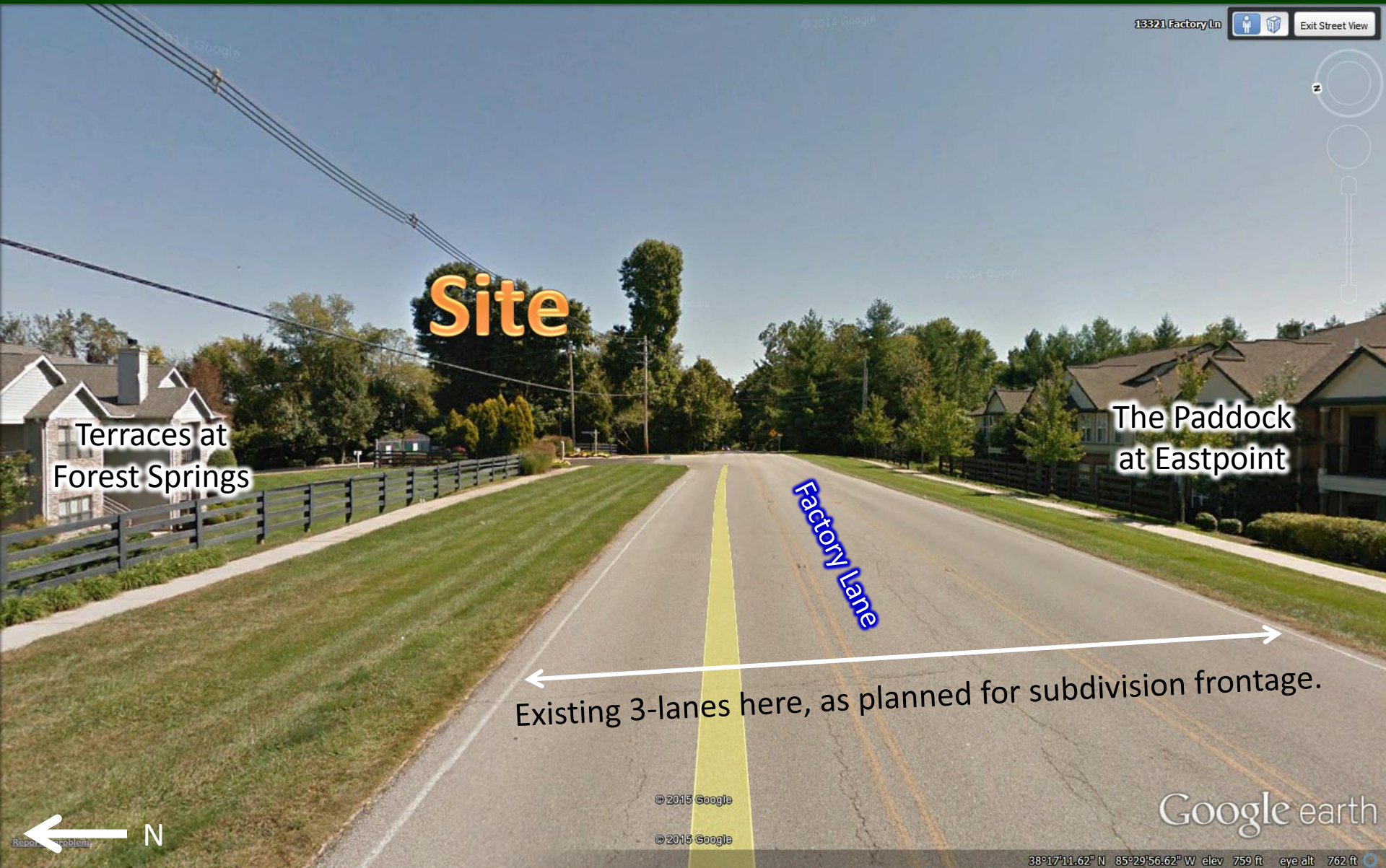
After these introductory comments, the following issues were specifically raised:

- Klemenz Family will—Orphanage cannot sell property due to conditions of Klemenz Family Will providing property to the orphanage.
- Perimeter buffer depth—30' buffer should be bigger, up to 300' suggested.
- Additional buffer landscaping—Additional landscaping should be added to buffer at north boundary where vegetation has been removed on the orphanage property behind several Forest Springs homes.
- Stub street to west—Representative of Klemenz family to contact me for meeting to discuss why stub at this location, their plans, etc.
- Traffic on Factory Lane—Traffic already too much, current unacceptable conditions to I-265 from site along Factory Lane and Old Henry intersections.
- More brick, less or no siding—especially for houses on lots backing to 30' buffer.
- Many environmental and traffic concerns cited from analysis of south parcel for VA hospital location.
- Lot size, house size, home prices incompatible with surrounding homes. No real attention given to the townhouses so did not discuss size or rental status. Was mentioned that townhomes are planned with front garages.
- We are too greedy with already too many yachts to ski behind.

Many attendees stayed after the meeting to ask questions. Mr. Bardenwerper gave them contacts at the Metro Division of Planning and Design Services (DPDS), Metro Works and Metro Transportation Planning Department.

Tab 5

Ground level photographs of the site
and surrounding area



Site

Terraces at Forest Springs

The Paddock at Eastpoint

Factory Lane

Existing 3-lanes here, as planned for subdivision frontage.

← N

Google earth

38°17'11.62" N 85°29'56.62" W elev 759 ft eye alt 762 ft

Looking east down Factory Lane towards Old Henry Road. Site is to the left.



Factory Lane



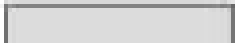
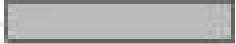














Site

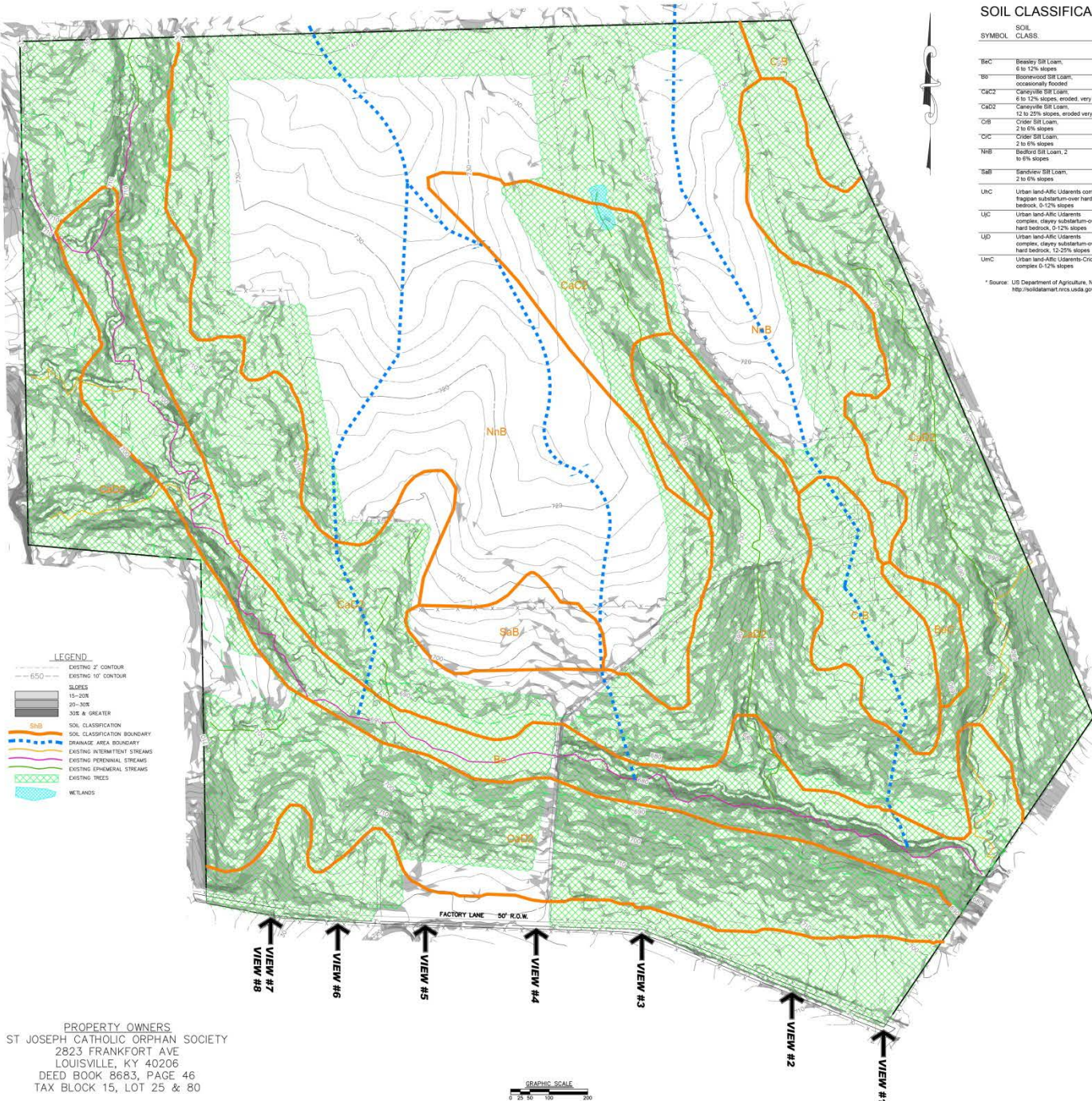
3-lanes (with center turn lane) planned for subdivision frontage. Balance of Factory Lane toward Old Henry Road is min. 18 ft. in width. Road System Development fee is also paid to improve area roads.

Looking west down Factory Lane towards LaGrange Road. Site is to the right.

Tab 6
Environmental Resource Map
(and infrastructure plan)

Environmental Resource and Infrastructure Legend

	EXISTING 2' CONTOUR
	EXISTING 10' CONTOUR
<u>SLOPES</u>	
	15-20%
	20-30%
	30% & GREATER
SHB	
	SOIL CLASSIFICATION
	SOIL CLASSIFICATION BOUNDARY
	DRAINAGE AREA BOUNDARY
	EXISTING INTERMITTENT STREAMS
	EXISTING PERENIAL STREAMS
	EXISTING EPHEMERAL STREAMS
	EXISTING TREES
	WETLANDS
	PRIMARY CONSERVATION AREA
	SECONDARY CONSERVATION AREA
	SECONDARY CONSERVATION AREA (50% CREDIT)
	CONCEPTUAL STORM SEWER
	CONCEPTUAL SANITARY SEWER



SOIL CLASSIFICATION TABLE

SYMBOL	SOIL CLASS.	DEPTH TO RESTRICTIVE FEATURE	DWELLINGS w/OOUT BASEMENTS	DWELLINGS w/ BASEMENTS	LOCAL ROADS AND STREETS
*Bc	Beadley Silt Loam, 0 to 12% slopes	2-0'	Somewhat limited, due to shrink-swell & slope	Somewhat limited, due to shrink-swell & slope	Very limited, due to low strength, shrink-swell & slope
*Bb	Bloomfield Silt Loam, occasionally flocced	2-4'	Very limited, due to flooding, depth to saturated zone/hard bedrock	Very limited, due to flooding, depth to saturated zone, and depth to bedrock	Very limited, due to low strength, slope & shrink-swell
*CaC2	Caneyville Silt Loam, 6 to 12% slopes, eroded, very rocky	2-4'	Somewhat limited, due to depth of bedrock, shrink-swell & slope	Very limited, due to depth of bedrock, shrink-swell & slope	Very limited, low strength, depth of bedrock, shrink-swell & slope
*CaC2	Caneyville Silt Loam, 12 to 22% slopes, eroded very rocky	2-4'	Very limited, due to slope, depth of bedrock & shrink-swell	Very limited, due to depth of bedrock, slope & shrink-swell	Very limited, low strength, slope, depth of bedrock & shrink-swell
*Cr	Crozer Silt Loam, 2 to 6% slopes	>6'	Not limited	Not limited	Very limited due to low strength
*CtC	Crozer Silt Loam, 2 to 6% slopes	>6'	Somewhat limited, due to slope	Somewhat limited, due to depth of cemented pan, slope & shrink-swell	Very limited due to low strength and slope
*NnB	Bedford Silt Loam, 2 to 6% slopes	2-3'	Somewhat limited, due to shrink-swell, depth to saturated zone, cemented pan	Very limited, due to depth of saturated zone, thick cemented pan, shrink-swell	Very limited, due to frost action, low strength & depth of saturated zone, depth of cemented pan
*SbB	Bedview Silt Loam, 2 to 6% slopes	>6'	Not limited	Not limited	Very limited, due to low strength
Ubc	Urban land-A8C Ultisols complex, chagne subulturn-over hard bedrock, 0-12% slopes	4-0'	Not rated	Not rated	Not rated
Uc	Urban land-A8C Ultisols complex, clayey subulturn-over hard bedrock, 0-12% slopes	4-0'	Not rated	Not rated	Not rated
Ud	Urban land-A8C Ultisols complex, clayey subulturn-over hard bedrock, 12-22% slopes	4-0'	Not rated	Not rated	Not rated
Umc	Urban land-A8C Ultisols-Crozer complex 0-12% slopes	4-0'	Not limited	Not limited	Very limited due low strength

* Source: US Department of Agriculture, NRCS of Jefferson County, Kentucky
<http://soilstamant.nrcs.usda.gov/Manuals/ky1110/m1ap3/shw21.pdf>

Environmental Resource Map

VIEW SHED ANALYSIS

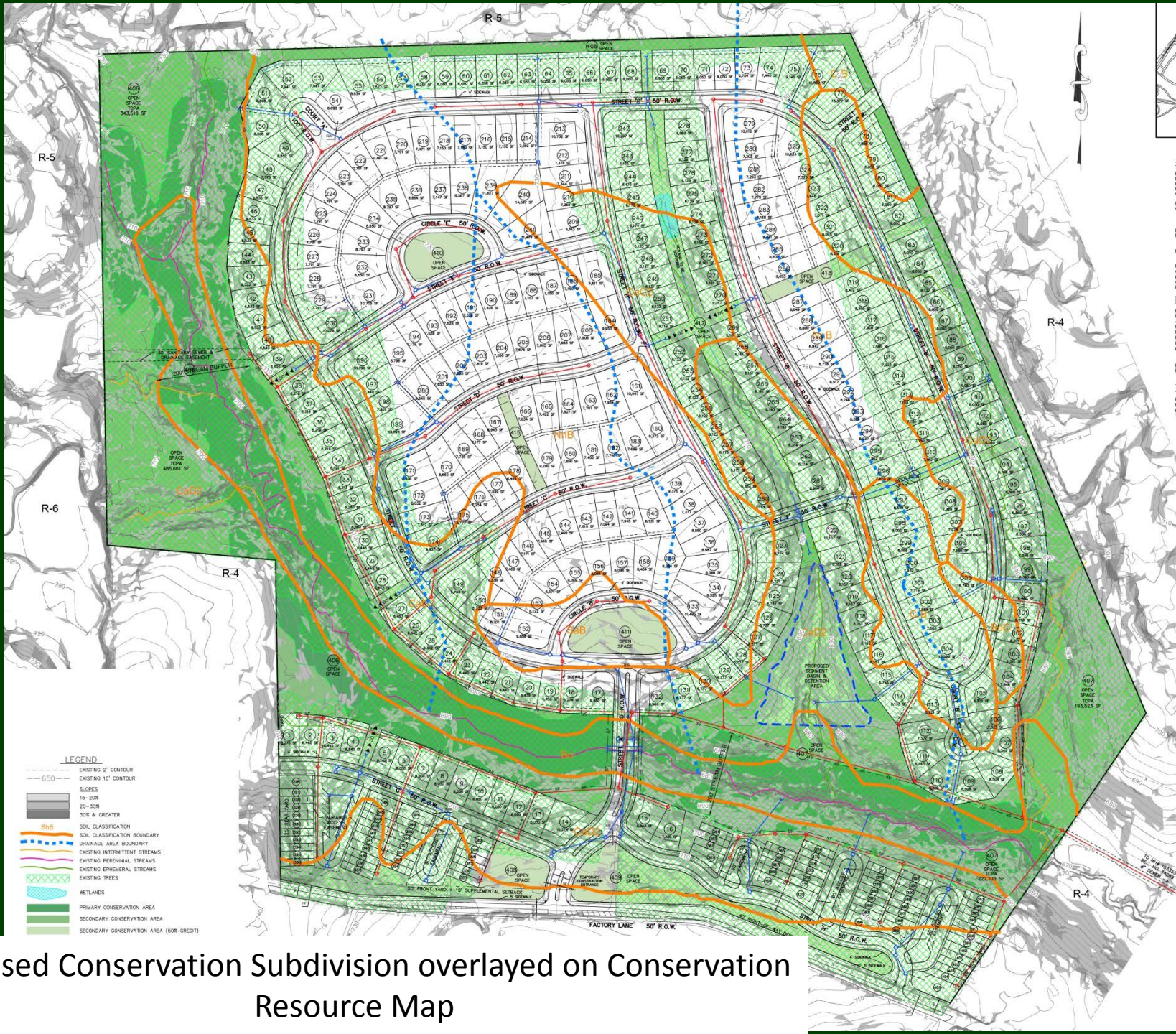
OVERLOOK AND REVERSE VEGETATION PREVENTS VIEWS INTO THE PROPERTY. AS A PART OF DEVELOPMENT, EXISTING VEGETATION WILL BE CLEARED UP AND REMOVED SPECIES REMOVED. EXISTING TREES THAT ARE IN GOOD HEALTH WILL BE PRESERVED. ADDITIONAL TREES AND VEGETATION WILL BE PLANTED IN ORDER TO MAINTAIN SCREENED VIEWS INTO THE SITE. EXISTING OVERHEAD POWERLINES WILL PREVENT THE FINISH OF SOME TREES NEARBY TO THE ROADWAY.



- LEGEND**
- 2' CONTOUR
 - 10' CONTOUR
 - SLOPES
 - 10-20%
 - 20-30%
 - 30% & GREATER
 - SOIL CLASSIFICATION
 - SOIL CLASSIFICATION BOUNDARY
 - DRAINAGE AREA BOUNDARY
 - EXISTING INTERMITTENT STREAMS
 - EXISTING PERENNIAL STREAMS
 - EXISTING EPHEMERAL STREAMS
 - EXISTING TREES
 - WETLANDS

PROPERTY OWNERS
 ST JOSEPH CATHOLIC ORPHAN SOCIETY
 2823 FRANKFORT AVE
 LOUISVILLE, KY 40206
 DEED BOOK 8683, PAGE 46
 TAX BLOCK 15, LOT 25 & 80

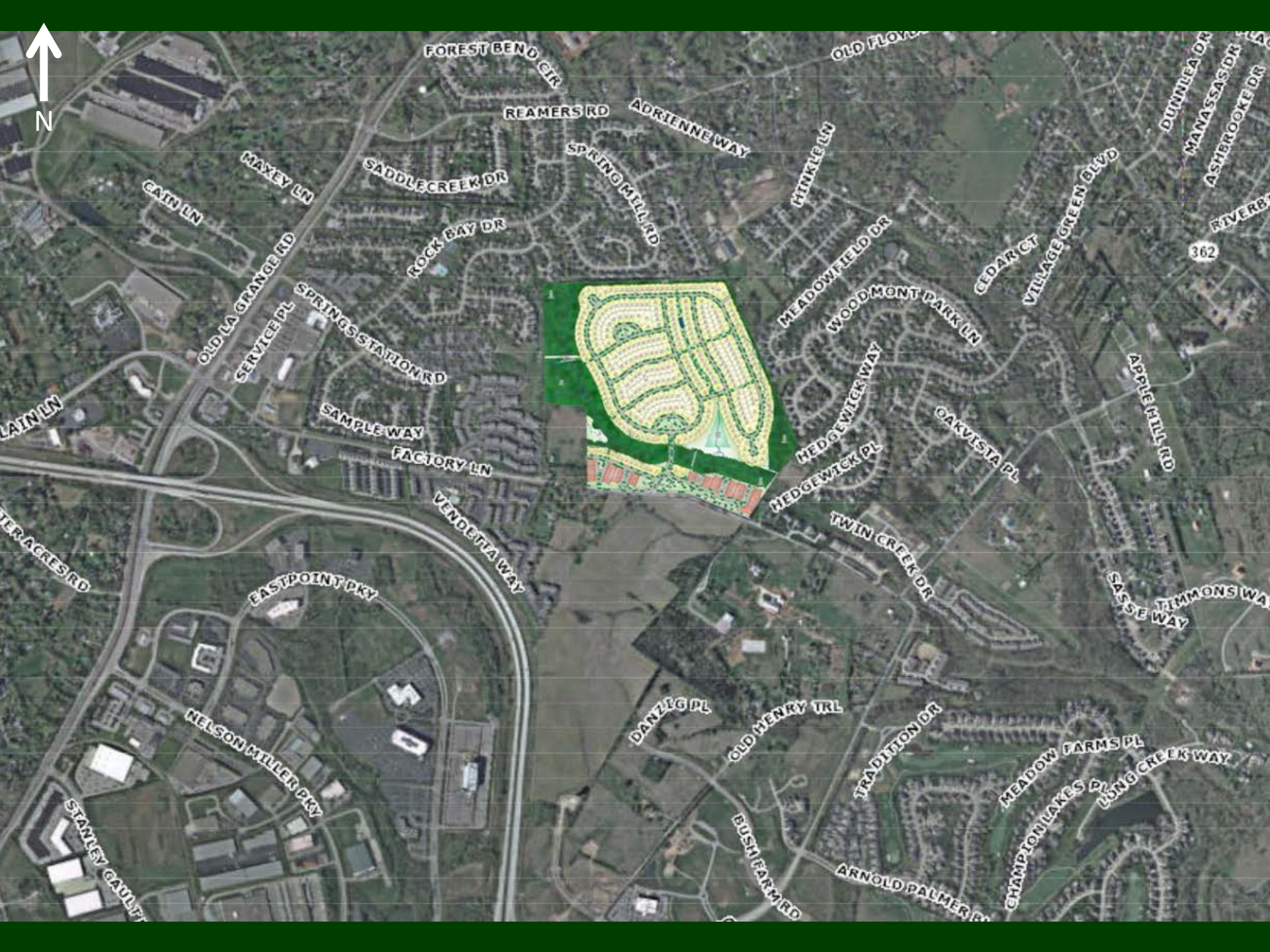




Proposed Conservation Subdivision overlaid on Conservation Resource Map

Tab 7

Conservation Subdivision Plan



FOREST BEND CIR

OLD FLOYD

REAMERS RD

ADRIENNE WAY

HINKLE LN

GAIN LN

MAXEY LN

SADDELCREEK DR

SPRINGMILL RD

ROCK BAY DR

DUNNEA DR

MANASSAS DR

ASHBROOKE DR

RIVERB...

362

OLD LA GRANGE RD

SERVICE PL

SPRINGS STATION RD

SAMPLE WAY

FACTORY LN

MEADOWFIELD DR

WOODMONT PARK LN

CEDAR CT

VILLAGE GREEN BLVD

LAIN LN

APPLE HILL RD

TER AGRES RD

EASTPOINT PKY

VENDETTA WAY

HEDGEWICK WAY

HEDGEWICK PL

OAKVISTA PL

TWIN CREEK DR

SASSE WAY

TIMMONS WA

NELSON MILLER PKY

DANZIG PL

OLD HENRY TRL

TRADITION DR

MEADOW FARMS PL

CHAMPION LAKES PL

LONG CREEK WAY

STANLEY GAULT

BUSH FARM RD

ARNOLD PALMER



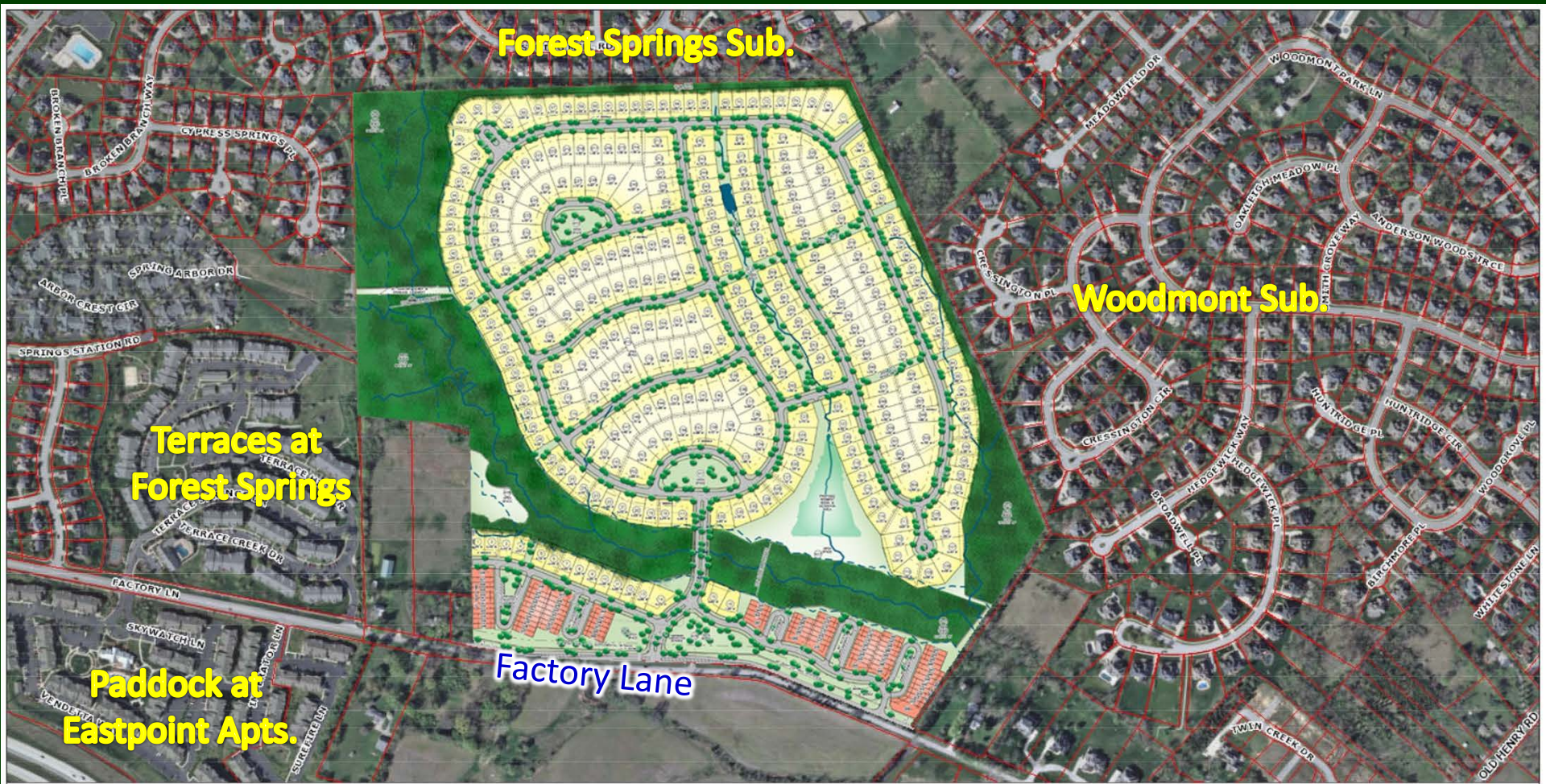
Forest Springs Sub.

Woodmont Sub.

Terraces at Forest Springs

Factory Lane

Paddock at Eastpoint Apts.





Woodmont Sub.






R-4

R-4

Factory Lane

Entrances



-  PRIMARY CONSERVATION AREA (PCA)
-  SECONDARY CONSERVATION AREA (SCA)
-  SECONDARY CONSERVATION AREA 50% CREDIT (SCA 50%)

CONSERVATION AREA MAP

NOT TO SCALE

Tab 8

Photographs of proposed home types



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball homes



Examples of likely Ball townhomes



Examples of likely Ball townhomes

Tab 9

Traffic Study



St. Joseph Orphanage Site

Ball Homes

Traffic Impact Study

August 5, 2015

Prepared for: Metro Transportation Planning



St. Joseph Orphanage Site

Project no: C9X22800
Document title: Traffic Impact Study
Document No.:
Revision: <revision>
Date: August 5, 2015
Client name: Ball Homes Inc.
Client no: Prepared for: Metro Transportation Planning
Project manager: Diane Zimmerman
Author: Diane Zimmerman
File name: C:\Users\diane.zimmerman\Documents\Ball Homes\Factory Lane\St. Joe Factory Lane
TIS Report.docx

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Appendix A. Traffic Counts

Appendix B. Highway Capacity Software Printouts

1. Introduction

The subdivision plan for the St. Joseph Orphanage site shows 325 detached lots and 80 attached lots. The site is located on the north side of Factory Lane in Louisville, KY. Access to the subdivision will be through two entrances on Factory Lane. The sole purpose of this study is to examine the traffic operation impacts of the proposed subdivision on the highway network. For this study, the impact area was defined to be the proposed intersections with Factory Lane, and six additional intersections: Factory Lane at La Grange Road (KY 146), Factory Lane at Colonial Springs Drive, Factory Lane at Terrace Springs Drive, Factory Lane at Old Henry Road, Old Henry Road at Arnold Palmer Boulevard, and Old Henry Road at Bush Farm Road. A map of the site is shown in **Figure 1**.



Figure 1 : Location Map

2. Existing Conditions

Factory Lane is a Metro maintained road with an estimated 2015 Average Annual Daily Traffic (AADT) of 6,600 vehicles per day at the proposed entrance, as estimated from the turning movement count. The road is a two-lane highway with ten-foot lanes with a one foot shoulder along the property frontage. The speed limit is 35 mph. There are sidewalks on both sides of Factory Lane from La Grange Road to Terrace Hill Drive.

The intersection of Factory Lane with La Grange Road (KY 146) is controlled with a traffic signal. The Factory Lane approach has a dedicated left turn lane, a shared thru and left lane and a dedicated right turn lane. The signal operates as split phase for Factory Lane and Chamberlain Lane. The signal also has train preemption.

The intersections of Factory Lane with Colonial Springs Road and Terrace Spring Drive are controlled with two-way stop signs. Factory Lane is the primary route. There are dedicated left turn lanes on Factory Lane to Colonial Springs Road and to Terrace Spring Drive. There are no dedicated turn lanes on either Colonial Springs Road or Terrace Spring Drive.

The intersection with Old Henry Road is currently an all-way stop without dedicated turn lanes. Old Henry Road is scheduled for reconstruction beginning in the summer of 2016. The project will add a two-way left turn lane from the Bush Farm Road intersection to Ash Avenue (KY 362) in Oldham County. Old Henry Road will become the primary route (will not stop) and Factory Lane will be controlled with a stop sign. The eastbound Factory Lane approach will have a dedicated left turn lane. The design speed of Old Henry Road is 45 mph. There will be a sidewalk on the southeast side of the road and a multi-use path on the northwest side of the road.

The intersection of Arnold Palmer Boulevard/Hamilton Springs Drive is controlled with a two-way stop sign for the minor streets. The existing eastbound right turn lane on Old Henry Road will be removed. Neither Arnold Palmer Boulevard nor Hamilton Springs Drive have dedicated turn lanes.

The intersection of Old Henry Road with Bush Farm Road is controlled with a traffic signal. Eastbound Old Henry Road has a dedicated left turn lane and a dedicated right turn lane. Westbound Old Henry Road will have a dedicated left turn lane, a thru lane and a shared thru and right turn lane. Bush Farm Road will have dedicated left turn lane and a shared thru and right turn lane in both directions.

Jacobs Engineering Group Inc. obtained a.m. and p.m. peak hour traffic counts at the intersections. The full count data for each intersection are included in Appendix A. **Figure 2** illustrates the existing a.m. and p.m. peak hour traffic volumes for these intersections.

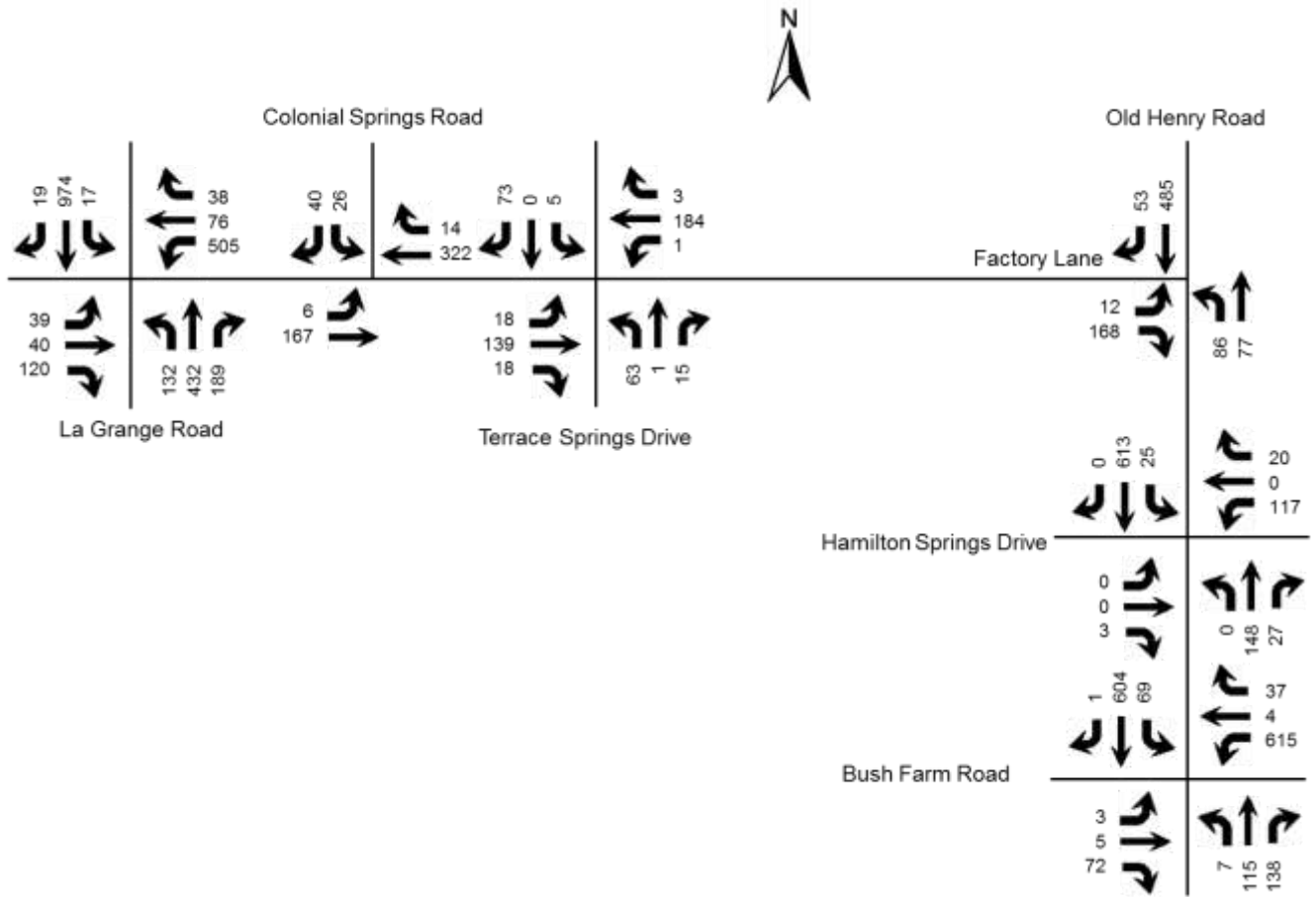


Figure 2 : A.M. Peak Hour Counts

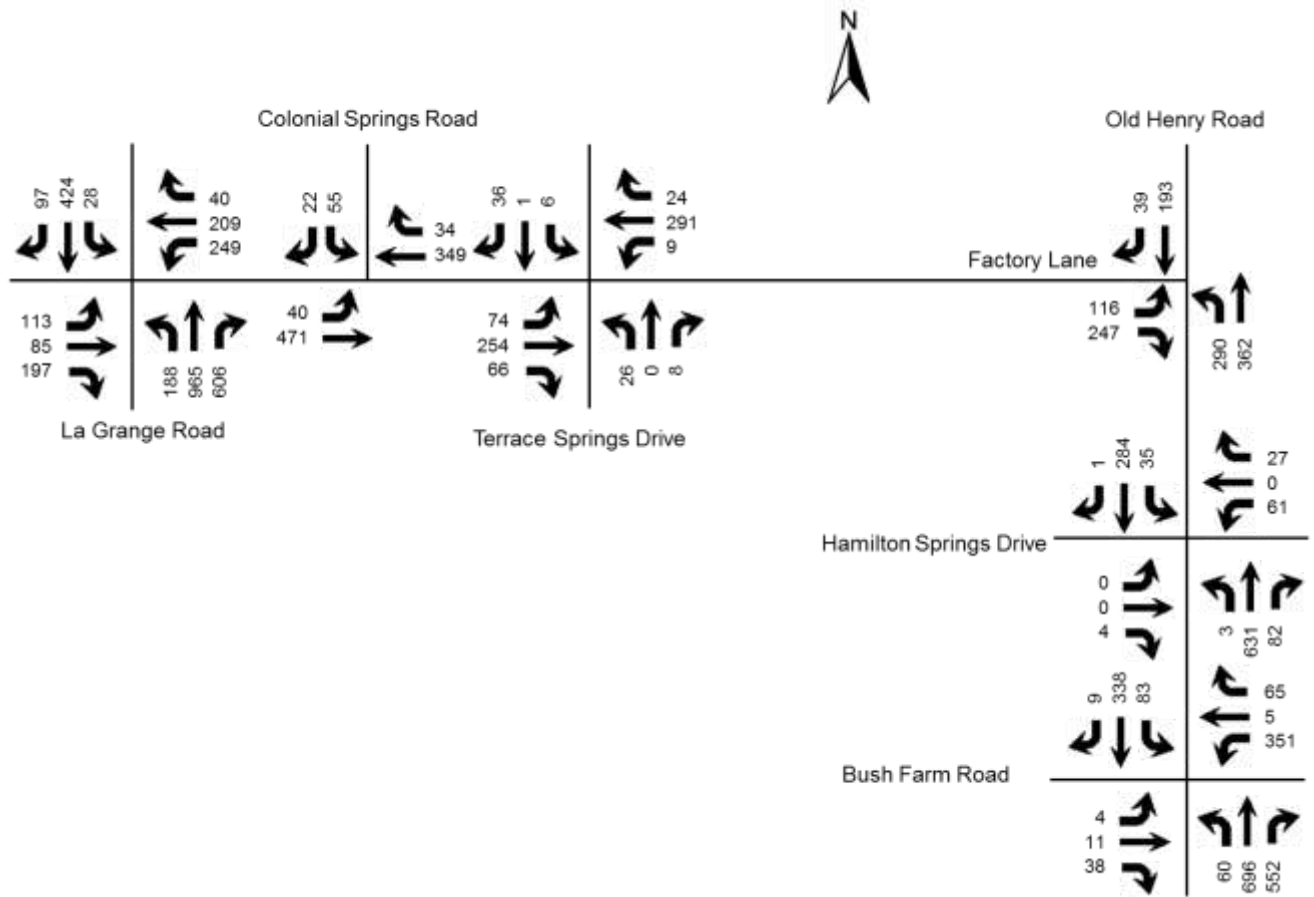


Figure 3 : P.M. Peak Hour Counts

3. Future Conditions

The projected completion year for this development is 2022, so the analysis year for this study is 2022. To predict traffic conditions in 2022, two and one third percent (2.33%) annual growth in traffic was added to Factory Lane and La Grange Road (KY 146). This growth is based upon a review of the traffic forecast for the Old Henry Road Extension dated June 16, 2011. Old Henry Road growth is taken from the forecast. Growth on Hamilton Springs Drive and Bush Farm Road is from the Old Henry Road Traffic Impact Study dated November 2014. **Figures 4 and 5** display the 2022 No Build peak hour volumes.

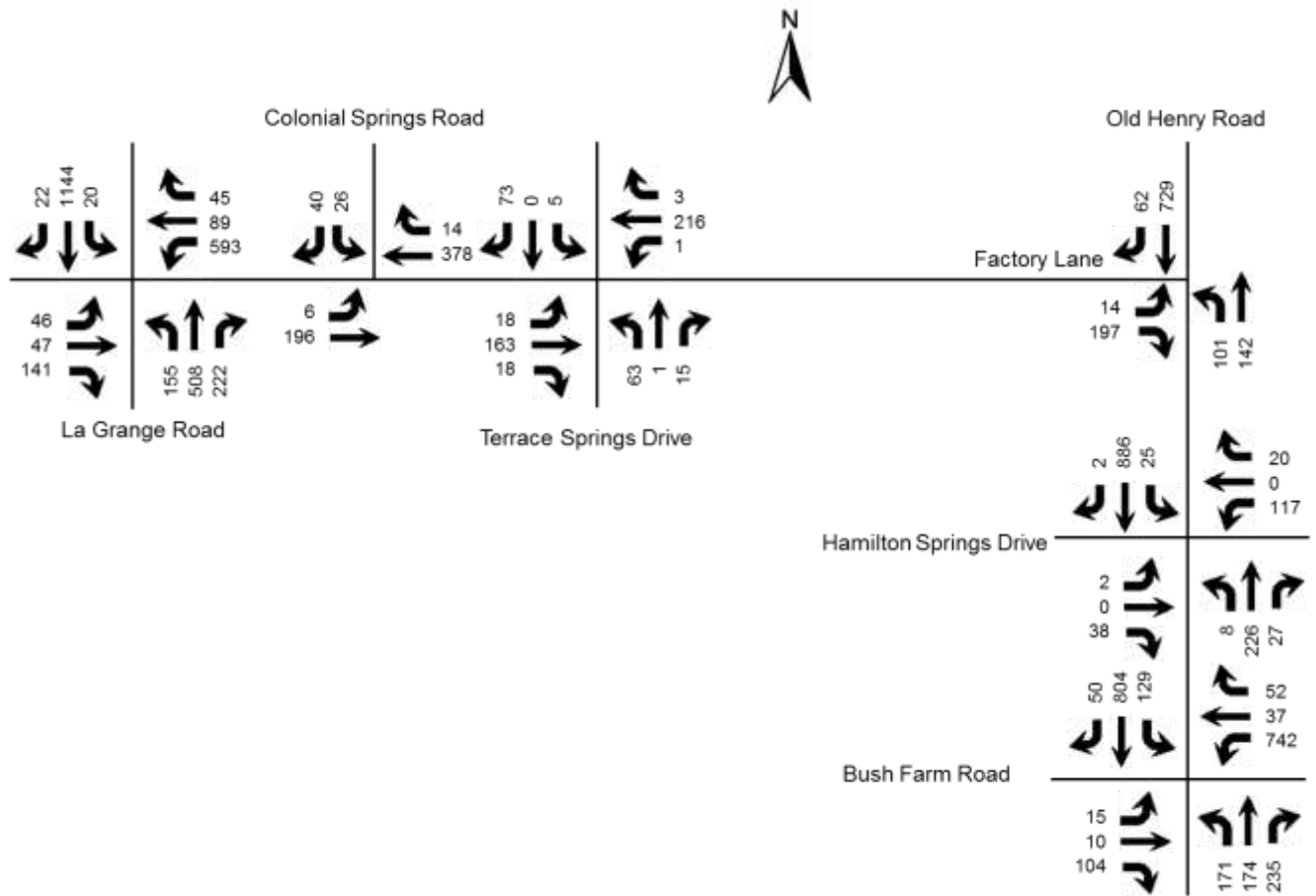


Figure 4 : No Build A.M. Peak Hour Volumes

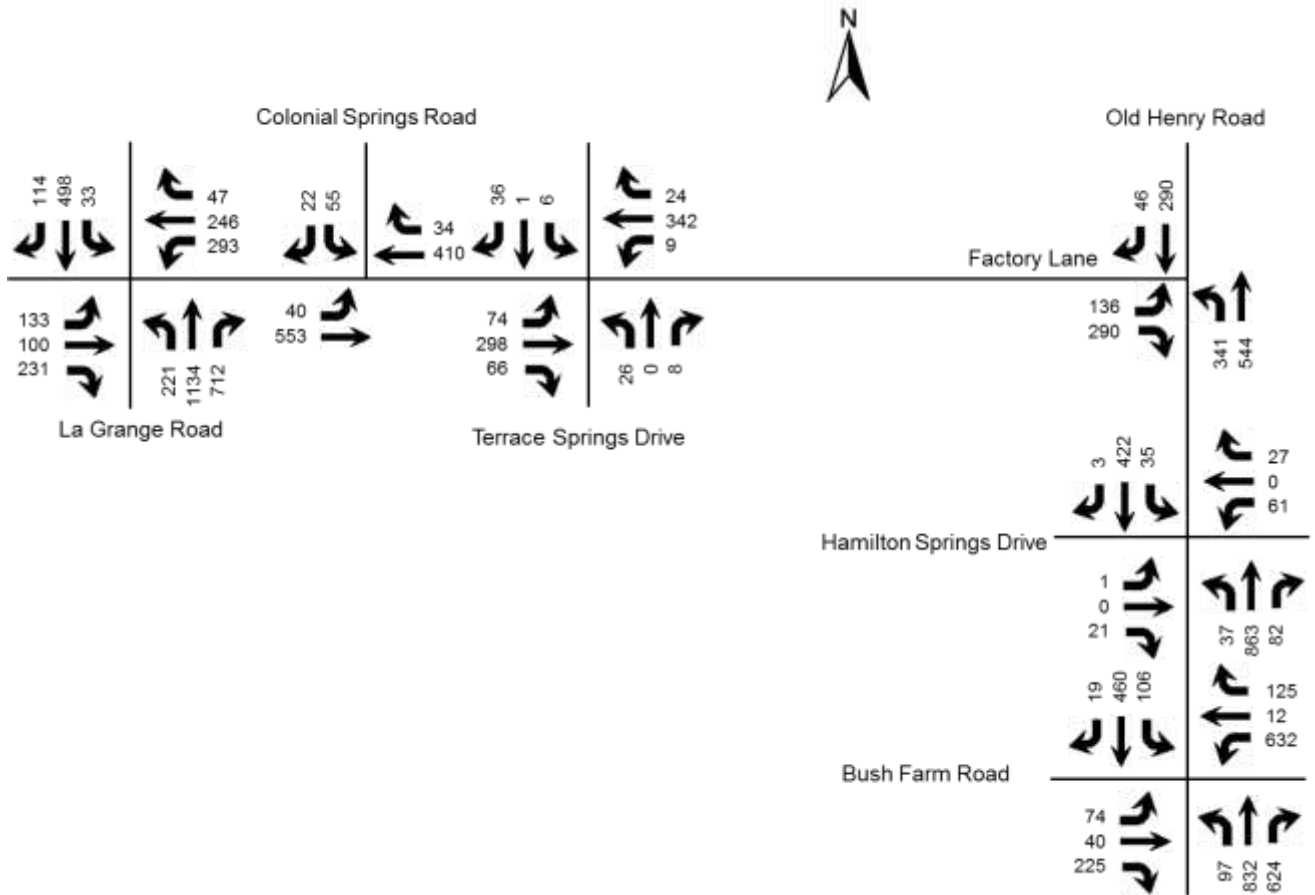


Figure 5 : No Build P.M. Peak Hour Volumes

4. Trip Generation and Distribution

The Institute of Transportation Engineers Trip Generation Manual, 9th Edition contains trip generation rates for a wide range of developments. The land use of “Single-Family Detached Housing (210)” and “Residential Condominium/Townhouse (230)” best describe this development. The trip generation results are listed in **Table 1**. The results of the trip generation analysis are that this additional development will generate 280 a.m. peak hour trips and 354 p.m. peak hour trips. The trips for the development were assigned to the highway network with percentages shown on **Figure 6**. **Figures 7 and 8** show the trips generated by this development and distributed throughout the road network for the year 2022 during the peak hours. **Figures 9 and 10** display the individual turning movements for the year 2022 for the peak hours when the development is completed.

Table 1. Trip Generation Results

	A.M.			P.M.		
	Total Trips	Entering	Exiting	Total Trips	Entering	Exiting
325 Detached lots (210)	237	59	178	304	192	112
80 Attached lots (230)	43	7	36	50	34	16
Total Peak Hour	280	66	214	354	226	128



Figure 6 : Site Trip Distribution Percentages

To simplify diagrams, the development is shown with a single access point in Figures 7 through 10. Figure 11 focuses on the turning movements for the entrances. Trip generation for 36 townhouses were assigned to the secondary entrance.

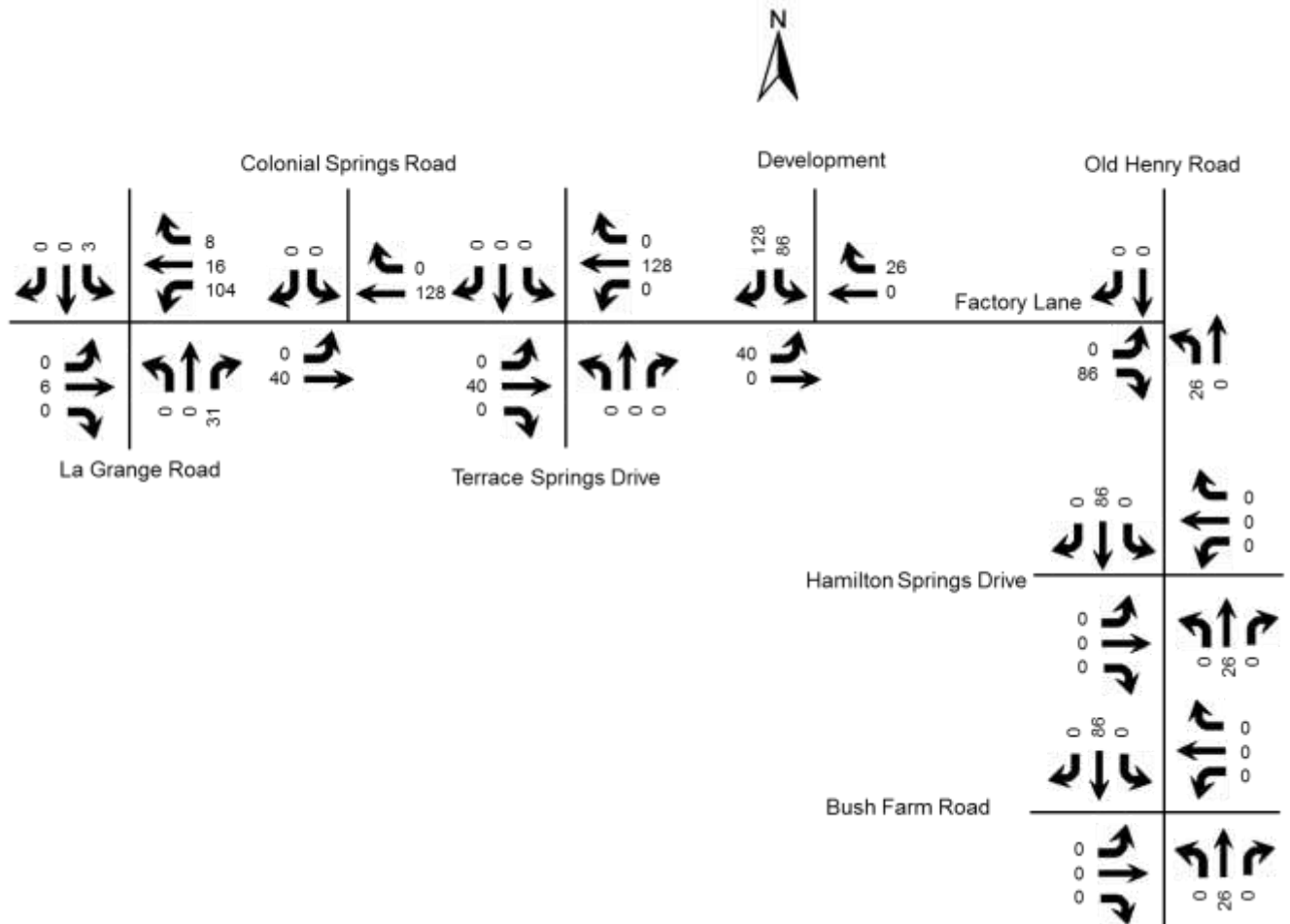


Figure 7 : Site Trip Distribution A.M. Peak Hour Volumes

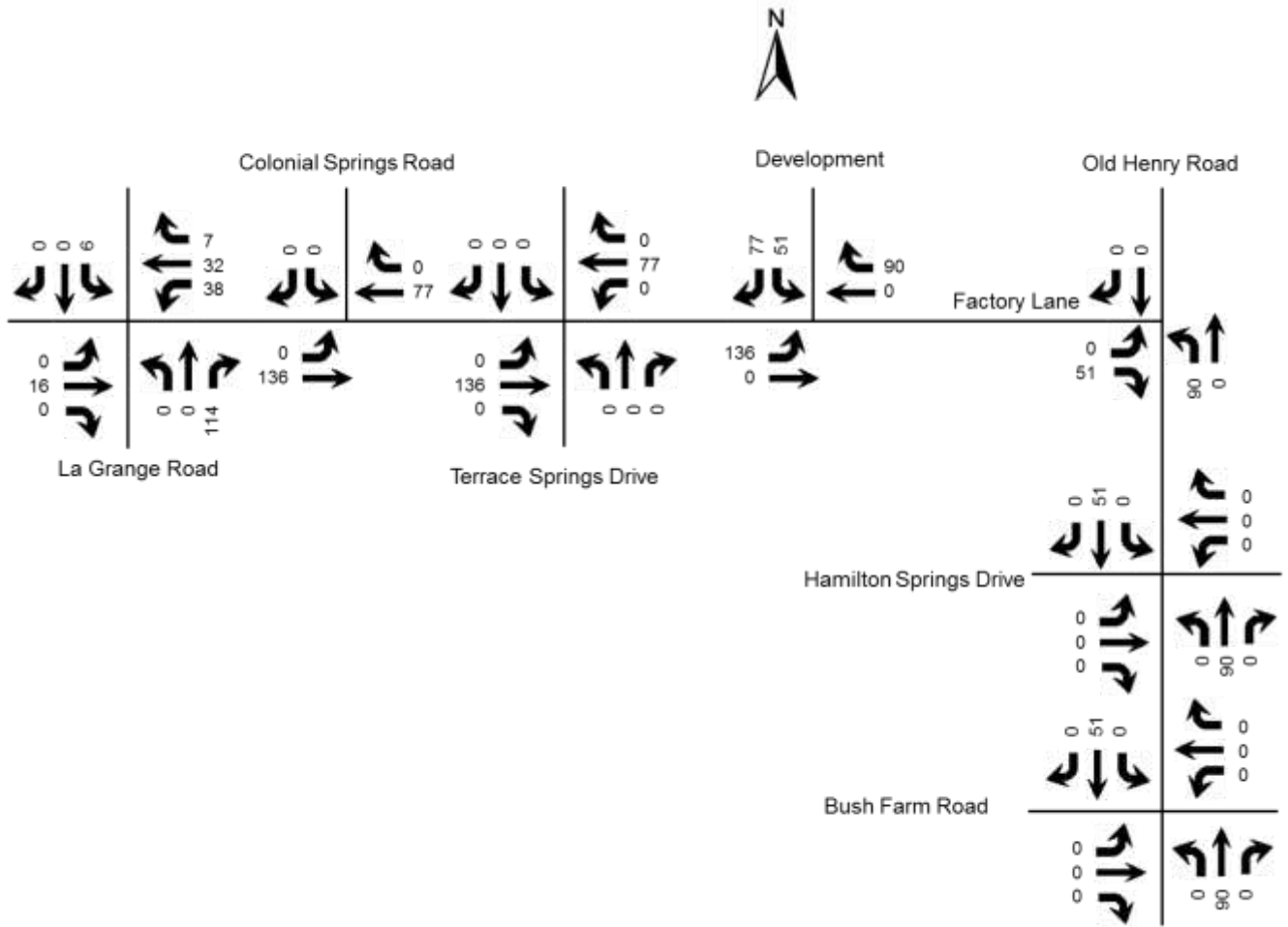


Figure 8 : Site Trip Distribution P.M. Peak Hour Volumes

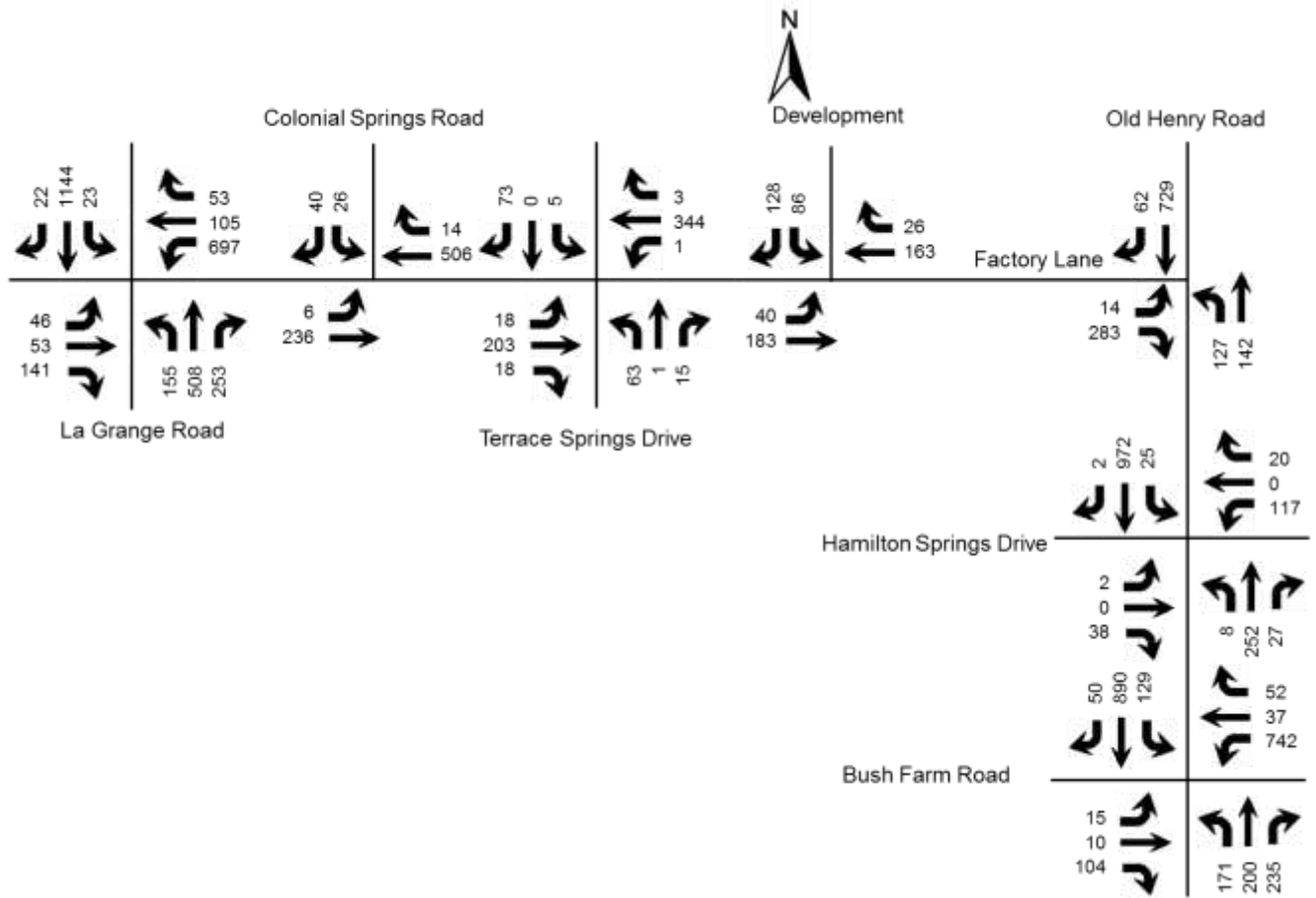


Figure 9 : Build A.M. Peak Hour Volumes

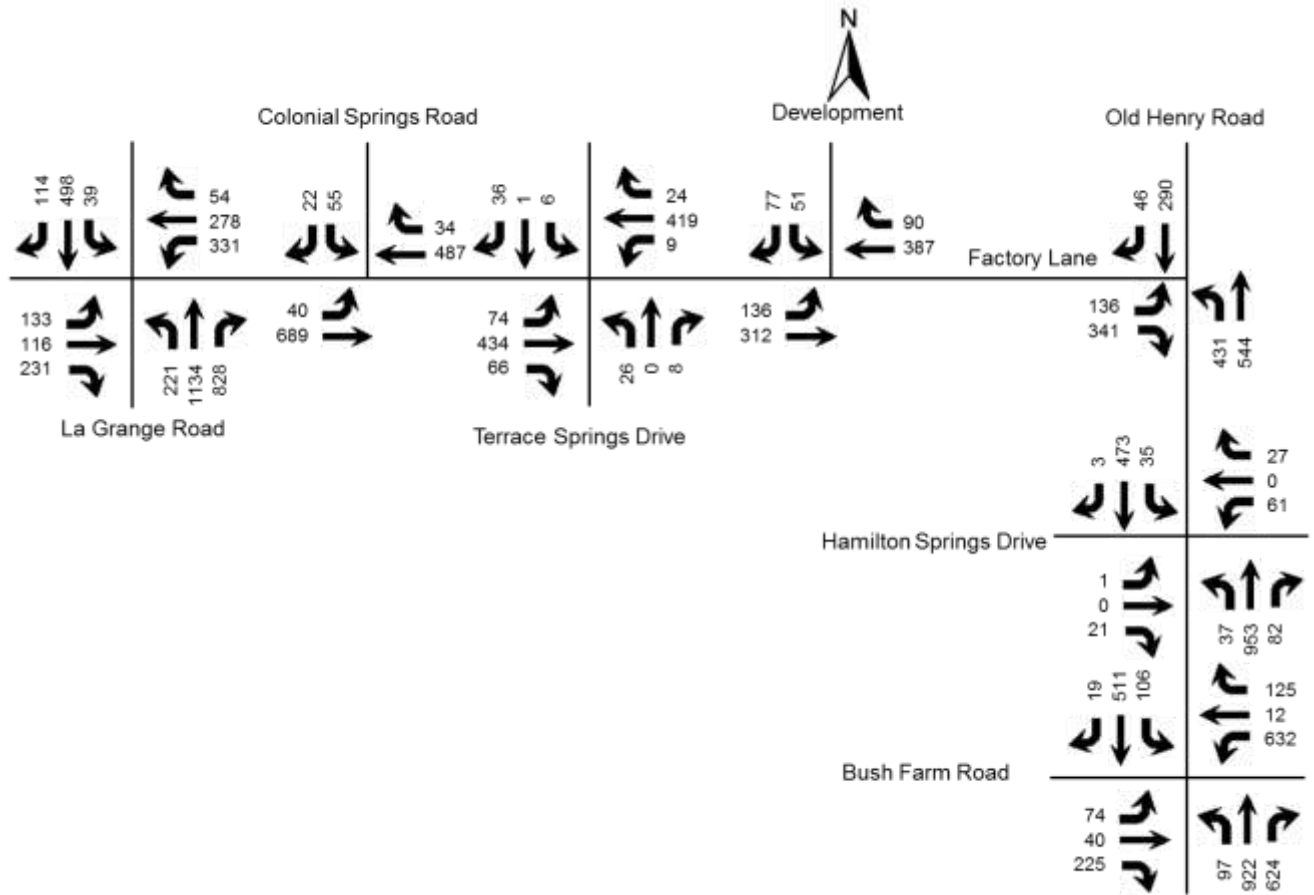


Figure 10 : Build P.M. Peak Hour Volumes

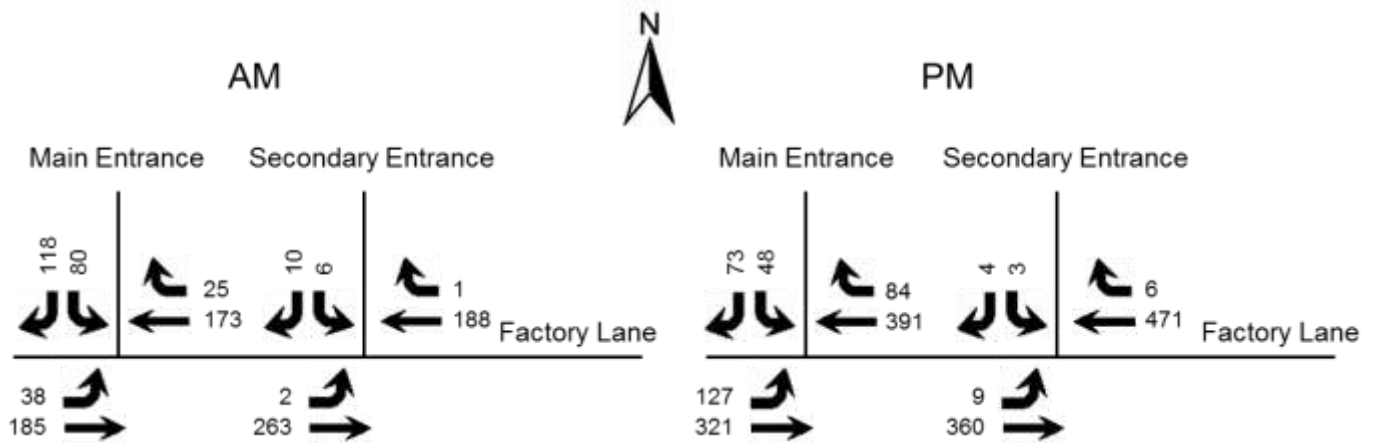


Figure 11 : Build Peak Hour Volumes at the Entrances

5. Analysis

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a “Level of Service” or LOS. Level of Service is a ranking scale from A through F with each level representing a range. LOS results depend upon the type of facility that is analyzed. In this case, the LOS is based upon the average vehicle delay each minor movement experiences at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersection were determined using procedures detailed in the Highway Capacity Manual, 2010 edition. Future delay and Level of Service were determined for the intersection using HCS 2010 Streets (version 6.65) and HCS+ (version 5.6) software. **Table 2** shows the results of the analysis for the three scenarios analyzed.

Metro Transportation Planning evaluates the need and length of auxiliary turn lanes using the Kentucky Transportation Cabinet Auxiliary Turn Lane Policy dated 7/20/2009. Using the volumes in **Figure 11**, an eastbound left turn lane is required at the main entrance. A westbound right turn lane is not required. An eastbound left turn lane is included in the results of the analysis in **Table 2**.

Table 2 : Level of Service Summary

Approach	A.M. Peak Hour			P.M. Peak Hour		
	2015	2022 No Build	2022 Build	2015	2022 No Build	2022 Build
La Grange Road at Factory Lane	D 39.7	E 70.5	F 102.2	C 26.9	D 35.4	D 46.0
La Grange Road Northbound	B 16.4	B 16.8	B 16.8	C 23.7	C 29.9	D 40.8
La Grange Road Southbound	C 27.9	C 31.1	C 31.1	C 27.5	C 31.1	C 32.2
Chamberlain Lane Eastbound	C 34.6	D 39.1	D 39.3	D 37.9	D 43.8	D 45.0
Factory Lane Westbound	F 88.8	F 210.4	F 310.3	C 28.5	D 52.8	E 77.6
Factory Lane at Colonial Springs Road						
Factory Lane Eastbound Left Turn	A 8.0	A 8.2	A 8.6	A 8.2	A 8.4	A 8.7
Colonial Springs Road Southbound	B 11.4	B 12.0	B 13.5	B 14.8	B 16.3	C 19.0
Factory Lane at Terrace Springs Drive						
Factory Lane Eastbound Left Turn	A 7.7	A 7.8	A 8.1	A 8.2	A 8.4	A 8.7
Factory Lane Westbound Left Turn	A 7.6	A 7.6	A 7.7	A 8.0	A 8.2	A 8.6
Terrace Springs Drive Northbound	B 11.6	B 12.1	B 13.7	C 15.1	C 16.4	C 19.7
Terrace Springs Drive Southbound	B 12.0	B 12.5	B 14.4	B 11.4	B 12.0	B 13.3

Approach	A.M. Peak Hour			P.M. Peak Hour		
	2015	2022 No Build	2022 Build	2015	2022 No Build	2022 Build
Factory Lane at Main Entrance						
Factory Lane Eastbound Left Turn			A 7.8			A 9.0
Main Entrance Southbound			B 12.4			C 16.8
Factory Lane at Secondary Entrance						
Factory Lane Eastbound Left Turn			A 7.6			A 8.5
Secondary Entrance Southbound			B 10.4			B 14.4
Old Henry Road at Factory Lane	(This intersection currently operates as an all-way stop. The construction project will create Old Henry as the major street.)					
Old Henry Road Northbound Left Turn	B 11.3	B 10.1	B 10.4	C 17.6	A 9.2	A 9.7
Old Henry Road Southbound (currently Westbound)	B 10.5			E 46.2		
Factory Lane Eastbound	C 24.9	C 24.0	E 38.1	B 14.8	E 42.4	F 74.2
Old Henry Road at Hamilton Springs						
Old Henry Road Northbound Left Turn	A 8.7	A 9.8	B 10.2	A 7.9	A 8.3	A 8.5
Old Henry Road Southbound Left Turn	A 7.6	A 7.8	A 7.9	A 9.9	B 12.2	B 13.5
Arnold Palmer Boulevard Westbound	D 25.2	E 38.5	E 49.9	D 32.4	E 44.2	F 64.2
Hamilton Springs Drive Eastbound	B 12.4	C 17.3	C 19.1	A 9.9	B 12.2	B 13.1
Old Henry Road at Bush Farm Road	C 31.4	E 77.2	E 76.9	C 20.1	F 116.5	F 124.8
Old Henry Road Northbound	B 17.8	C 23.3	C 25.3	B 18.6	D 50.4	E 74.7
Old Henry Road Southbound	C 28.4	B 19.1	B 19.8	B 16.4	E 76.5	E 71.9
Bush Farm Road Eastbound	B 13.7	C 21.7	C 22.0	B 19.3	C 21.1	C 21.1
Bush Farm Road Westbound	D 40.8	F 188.2	F 192.7	C 28.6	F 317.3	F 317.3

Key: Level of Service, Delay in seconds per vehicle

6. Conclusion

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2022, there will be impacts to the existing highway network. At the main entrance to the subdivision a left turn lane will be installed per Metro policy. Turn lanes are not required at the secondary entrance.

This report identifies capacity deficiencies at both signalized intersections in the study area. At the La Grange Road intersection with Factory Lane improvements should be realized with the installation of a signal at the intersection of Springs Station Road. This signal has been requested by the shopping center owner and is currently in the review process at KYTC. This proposed signal could divert as much as 30 percent of the shopping center traffic currently using Factory Lane.

At the Old Henry Road intersection with Bush Farm Road, the capacity deficiency is caused by the traffic forecast from the Old Henry Crossings development to utilize Bush Farm Road (westbound left turn). The proposed development does not add traffic to that movement. Once the proposed traffic signal at Terra Crossings is installed, the traffic volumes on Bush Farm Road should decrease.

Appendix A. Traffic Counts

Study Name LaGrange Rd & Chamberlain Ln

Start Date 02/24/2015

Start Time 7:00 AM

	LaGrange Road Southbound			Factory Lane Westbound			LaGrange Road Northbound				Chamberlain Lane Eastbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	178	3	118	16	8	25	77	33	0	16	3	27
7:15 AM	3	240	2	122	10	7	20	107	39	0	14	7	38
7:30 AM	2	286	5	118	21	12	30	108	50	0	8	5	32
7:45 AM	8	217	4	133	25	10	38	108	58	3	12	18	34
	13	921	14	491	72	37	113	400	180	3	50	33	131
8:00 AM	4	231	8	132	20	9	44	109	42	1	5	10	16
8:15 AM	7	194	11	111	21	8	36	95	47	0	8	10	33
8:30 AM	1	202	9	96	20	2	39	84	50	0	6	14	22
8:45 AM	9	155	11	97	18	13	25	81	55	0	9	15	26
	21	782	39	436	79	32	144	369	194	1	28	49	97
4:00 PM	8	115	10	64	27	12	29	180	113	0	36	35	99
4:15 PM	4	134	17	58	24	18	38	234	141	0	23	28	87
4:30 PM	1	120	12	55	36	8	48	210	142	0	30	38	86
4:45 PM	4	97	24	67	46	10	33	217	138	0	35	24	50
	17	466	63	244	133	48	148	841	534	0	124	125	322
5:00 PM	7	106	19	56	38	12	56	239	156	0	28	20	42
5:15 PM	8	110	24	68	63	7	34	245	170	0	27	19	59
5:30 PM	9	111	30	58	62	11	65	264	142	0	23	22	46
5:45 PM	4	99	15	52	40	15	32	241	134	0	24	23	46
	28	426	88	234	203	45	187	989	602	0	102	84	193

	LaGrange Road Southbound			Factory Lane Westbound			LaGrange Road Northbound				Chamberlain Lane Eastbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-Turn	Left	Thru	Right
7:15 AM	3	240	2	122	10	7	20	107	39	0	14	7	38
7:30 AM	2	286	5	118	21	12	30	108	50	0	8	5	32
7:45 AM	8	217	4	133	25	10	38	108	58	3	12	18	34
8:00 AM	4	231	8	132	20	9	44	109	42	1	5	10	16
	17	974	19	505	76	38	132	432	189	4	39	40	120

4:45 PM	4	97	24	67	46	10	33	217	138	0	35	24	50
5:00 PM	7	106	19	56	38	12	56	239	156	0	28	20	42
5:15 PM	8	110	24	68	63	7	34	245	170	0	27	19	59
5:30 PM	9	111	30	58	62	11	65	264	142	0	23	22	46
	28	424	97	249	209	40	188	965	606	0	113	85	197

JACOBS

11940 Highway 42, Suite 1
Goshen, KY 40026

Counted by: Andy Wolak

File Name : ColonialSpringsAM
Site Code : 00512151
Start Date : 5/12/2015
Page No : 1

Groups Printed- Unshifted

Start Time	Colonial Springs Rd From North				Factory Lane From East				From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	7	0	7	14	0	62	1	63	0	0	0	0	0	23	0	23	100
07:15 AM	9	0	11	20	0	75	0	75	0	0	0	0	0	22	0	22	117
07:30 AM	7	0	14	21	0	85	3	88	0	0	0	0	1	38	0	39	148
07:45 AM	13	0	9	22	0	90	4	94	0	0	0	0	3	39	0	42	158
Total	36	0	41	77	0	312	8	320	0	0	0	0	4	122	0	126	523
08:00 AM	4	0	9	13	0	79	4	83	0	0	0	0	2	50	0	52	148
08:15 AM	2	0	8	10	0	68	3	71	0	0	0	0	0	40	0	40	121
08:30 AM	5	0	4	9	0	69	3	72	0	0	0	0	3	31	0	34	115
08:45 AM	4	0	9	13	0	53	3	56	0	0	0	0	2	30	0	32	101
Total	15	0	30	45	0	269	13	282	0	0	0	0	7	151	0	158	485
Grand Total	51	0	71	122	0	581	21	602	0	0	0	0	11	273	0	284	1008
Apprch %	41.8	0	50.2		0	96.5	3.5		0	0	0	0	3.9	96.1	0		
Total %	5.1	0	7	12.1	0	57.6	2.1	59.7	0	0	0	0	1.1	27.1	0	28.2	

Start Time	Colonial Springs Rd From North				Factory Lane From East				From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	7	0	14	21	0	85	3	88	0	0	0	0	1	38	0	39	148
07:45 AM	13	0	9	22	0	90	4	94	0	0	0	0	3	39	0	42	158
08:00 AM	4	0	9	13	0	79	4	83	0	0	0	0	2	50	0	52	148
08:15 AM	2	0	8	10	0	68	3	71	0	0	0	0	0	40	0	40	121
Total Volume	26	0	40	66	0	322	14	336	0	0	0	0	6	167	0	173	575
% App. Total	39.4	0	60.6		0	95.8	4.2		0	0	0	0	3.5	96.5	0		
PHF	500	000	714	750	000	894	875	894	000	000	000	000	500	835	000	832	910

Groups Printed- Unshifted

Start Time	Colonial Springs Rd From North				Factory Lane From East				From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	12	0	4	16	0	59	5	64	0	0	0	0	6	73	0	79	159
04:15 PM	6	0	5	11	0	61	5	66	0	0	0	0	3	109	0	112	189
04:30 PM	8	0	4	12	0	79	5	84	0	0	0	0	8	132	0	140	236
04:45 PM	18	0	4	22	0	82	6	88	0	0	0	0	3	94	0	97	207
Total	44	0	17	61	0	281	21	302	0	0	0	0	20	408	0	428	791
05:00 PM	17	0	5	22	0	90	11	101	0	0	0	0	11	125	0	136	259
05:15 PM	17	0	6	23	0	93	8	101	0	0	0	0	8	125	0	133	257
05:30 PM	10	0	4	14	0	84	9	93	0	0	0	0	6	94	0	100	207
05:45 PM	11	0	7	18	0	82	6	88	0	0	0	0	15	127	0	142	248
Total	55	0	22	77	0	349	34	383	0	0	0	0	40	471	0	511	971
Grand Total	99	0	39	138	0	630	55	685	0	0	0	0	60	879	0	939	1762
Apprch %	71.7	0	26.3		0	92	8		0	0	0	0	6.4	93.6	0		
Total %	5.6	0	2.2	7.8	0	35.8	3.1	38.9	0	0	0	0	3.4	49.9	0	53.3	

Start Time	Colonial Springs Rd From North				Factory Lane From East				From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	17	0	5	22	0	90	11	101	0	0	0	0	11	125	0	136	259
05:15 PM	17	0	6	23	0	93	8	101	0	0	0	0	8	125	0	133	257
05:30 PM	10	0	4	14	0	84	9	93	0	0	0	0	6	94	0	100	207
05:45 PM	11	0	7	18	0	82	6	88	0	0	0	0	15	127	0	142	248
Total Volume	55	0	22	77	0	349	34	383	0	0	0	0	40	471	0	511	971
% App. Total	71.4	0	26.6		0	91.1	8.9		0	0	0	0	7.8	92.2	0		
PHF	809	000	786	837	000	938	773	948	000	000	000	000	667	927	000	900	937

JACOBS

11940 Highway 42, Suite 1
Goshen, KY 40026

Counted by: Andy Wolak

File Name : FactoryLnAM rot
Site Code : 00022515
Start Date : 2/25/2015
Page No : 1

Groups Printed- Unshifted

Start Time	Terrace Springs Drive From North				Factory Lane From East				Terrace Springs Drive From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	4	0	13	17	1	40	1	42	26	0	0	26	1	22	1	24	109
07:15 AM	3	0	18	21	0	31	0	31	15	0	0	16	4	18	5	27	95
07:30 AM	1	0	21	22	0	41	0	41	16	0	2	18	2	23	3	28	109
07:45 AM	3	0	19	22	0	50	0	50	18	1	3	22	6	42	5	53	147
Total	11	0	71	82	1	162	1	164	75	2	5	82	13	105	14	132	460
08:00 AM	1	0	13	14	1	53	2	56	13	0	3	16	4	42	5	51	137
08:15 AM	0	0	20	20	0	40	1	41	16	0	7	23	6	32	5	43	127
08:30 AM	1	0	16	17	0	30	2	32	15	0	3	18	4	30	2	36	103
08:45 AM	1	0	9	10	2	35	0	37	7	0	1	8	5	33	5	43	98
Total	3	0	58	61	3	158	5	166	51	0	14	65	19	137	17	173	465
Grand Total	14	0	129	143	4	320	6	330	126	2	19	147	32	242	31	305	925
Apprch %	9.8	0	90.2		1.2	97	1.8		85.7	1.4	12.9		10.5	79.3	10.2		
Total %	1.5	0	13.9	15.5	0.4	34.6	0.6	35.7	13.6	0.2	2.1	15.9	3.5	26.2	3.4	33	

Start Time	Terrace Springs Drive From North				Factory Lane From East				Terrace Springs Drive From South				Factory Lane From West				
Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	1	0	21	22	0	41	0	41	16	0	2	18	2	23	3	28	109
07:45 AM	3	0	19	22	0	50	0	50	18	1	3	22	6	42	5	53	147
08:00 AM	1	0	13	14	1	53	2	56	13	0	3	16	4	42	5	51	137
08:15 AM	0	0	20	20	0	40	1	41	16	0	7	23	6	32	5	43	127
Total Volume	5	0	73	78	1	184	3	188	63	1	15	79	18	139	18	175	520
% App. Total	6.4	0	93.6		0.5	97.9	1.6		79.7	1.3	19		10.3	79.4	10.3		
PHF	.417	.000	.869	.886	.250	.868	.375	.839	.875	.250	.536	.859	.750	.827	.900	.825	.884

Counted by: Andy Wolak

File Name : FactoryLnPM rot
Site Code : 00022415
Start Date : 2/24/2015
Page No : 1

Groups Printed- Unshifted

Start Time	Terrace Springs Drive From North				Factory Lane From East				Terrace Springs Drive From South				Factory Lane From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	1	0	6	7	2	44	3	49	11	0	1	12	17	54	12	83	151
04:15 PM	5	0	11	16	0	50	0	50	5	0	0	5	14	74	21	109	180
04:30 PM	1	0	7	8	1	37	4	42	6	0	1	7	20	65	11	96	153
04:45 PM	0	0	10	10	3	52	0	55	7	1	3	11	20	52	18	90	166
Total	7	0	34	41	6	183	7	196	29	1	5	35	71	245	62	378	650
05:00 PM	1	0	4	5	0	67	5	72	9	0	2	11	8	52	15	75	163
05:15 PM	3	1	13	17	4	75	8	87	6	0	1	7	26	72	16	114	225
05:30 PM	1	0	9	10	3	93	7	103	5	0	1	6	17	66	23	106	225
05:45 PM	1	0	10	11	2	56	4	62	6	0	4	10	23	64	12	99	182
Total	6	1	36	43	9	291	24	324	26	0	8	34	74	254	66	394	795
Grand Total	13	1	70	84	15	474	31	520	55	1	13	69	145	499	128	772	1445
Apprch %	15.5	1.2	83.3		2.9	91.2	6		79.7	1.4	18.8		18.8	64.6	16.6		
Total %	0.9	0.1	4.8	5.8	1	32.8	2.1	36	3.8	0.1	0.9	4.8	10	34.5	8.9	53.4	

Start Time	Terrace Springs Drive From North				Factory Lane From East				Terrace Springs Drive From South				Factory Lane From West				
Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	1	0	4	5	0	67	5	72	9	0	2	11	8	52	15	75	163
05:15 PM	3	1	13	17	4	75	8	87	6	0	1	7	26	72	16	114	225
05:30 PM	1	0	9	10	3	93	7	103	5	0	1	6	17	66	23	106	225
05:45 PM	1	0	10	11	2	56	4	62	6	0	4	10	23	64	12	99	182
Total Volume	6	1	36	43	9	291	24	324	26	0	8	34	74	254	66	394	795
% App. Total	14	2.3	83.7		2.8	89.8	7.4		76.5	0	23.5		18.8	64.5	16.8		
PHF	.500	.250	.692	.632	.563	.782	.750	.786	.722	.000	.500	.773	.712	.882	.717	.864	.883



11940 Highway 42, Suite 1
Goshen, KY 40026

Counted by: Andy Wolak

File Name : FactoryLnOldHenryAM
Site Code : 05121522
Start Date : 5/13/2015
Page No : 1

Groups Printed- Unshifted

Start Time	Old Henry Road From North					Old Henry Road From East					From South					Factory Lane From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
07:00 AM	122	0	9	0	130	0	16	10	0	26	0	0	0	0	0	3	27	0	0	30	188
07:15 AM	135	0	17	0	152	0	10	20	0	30	0	0	0	0	0	1	45	0	0	46	231
07:30 AM	133	0	13	0	146	0	19	16	0	35	0	0	0	0	0	4	36	0	0	40	221
07:45 AM	105	0	12	0	117	0	39	16	0	55	0	0	0	0	0	5	42	0	0	47	219
Total	465	0	50	0	545	0	84	62	0	146	0	0	0	0	0	13	153	0	0	166	867
08:00 AM	112	0	11	0	123	0	18	25	0	43	0	0	0	0	0	2	42	0	0	44	210
08:15 AM	90	0	10	0	100	0	23	15	0	38	0	0	0	0	0	5	44	0	0	49	188
08:30 AM	84	0	13	0	97	0	14	23	0	37	0	0	0	0	0	4	31	0	0	35	169
08:45 AM	84	0	14	0	98	0	21	23	0	44	0	0	0	0	0	4	39	0	0	43	185
Total	370	0	48	0	418	0	76	87	0	163	0	0	0	0	0	15	158	0	0	171	782
Grand Total	865	0	98	0	963	0	160	149	0	309	0	0	0	0	0	28	309	0	0	337	1609
Approach %	89.8	0	10.2	0		0	51.8	48.2	0		0	0	0	0	0	8.3	91.7	0	0		
Total %	53.8	0	6.1	0	59.9	0	9.9	9.3	0	19.2	0	0	0	0	0	1.7	19.2	0	0	20.9	

Start Time	Old Henry Road From North					Old Henry Road From East					From South					Factory Lane From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:15 AM																					
07:15 AM	135	0	17	0	152	0	10	20	0	30	0	0	0	0	0	1	48	0	0	49	231
07:30 AM	133	0	13	0	146	0	19	16	0	35	0	0	0	0	0	4	36	0	0	40	221
07:45 AM	105	0	12	0	117	0	39	16	0	55	0	0	0	0	0	5	42	0	0	47	219
08:00 AM	112	0	11	0	123	0	18	25	0	43	0	0	0	0	0	2	42	0	0	44	210
Total Volume	485	0	53	0	538	0	86	77	0	163	0	0	0	0	0	12	168	0	0	180	861
% App. Total	90.1	0	9.9	0		0	52.8	47.2	0		0	0	0	0	0	6.7	93.3	0	0		
PHF	898	0.000	779	0.000	885	0.000	551	770	0.000	741	0.000	0.000	0.000	0.000	0.000	600	875	0.000	0.000	918	953

Counted by: Andy Wolak

File Name : FactoryLnOldHenryPM
Site Code : 05111511
Start Date : 5/12/2015
Page No : 1

Groups Printed- Unshifted

Start Time	Old Henry Road From North					Old Henry Road From East					From South					Factory Lane From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
04:00 PM	48	0	9	0	55	0	39	74	0	113	0	0	0	0	0	22	27	0	0	49	217
04:15 PM	43	0	9	0	50	0	33	71	0	104	0	0	0	0	0	11	42	0	0	53	209
04:30 PM	51	0	14	0	65	0	38	77	0	112	0	0	0	0	0	21	57	0	0	78	266
04:45 PM	47	0	8	0	52	0	41	75	0	116	0	0	0	0	0	18	55	0	0	71	226
Total	187	0	37	0	224	0	148	297	0	445	0	0	0	0	0	70	181	0	0	251	920
05:00 PM	46	0	12	0	58	0	50	88	0	136	0	0	0	0	0	30	58	0	0	88	279
05:15 PM	55	0	7	0	62	0	84	91	0	175	0	0	0	0	0	28	61	0	0	89	326
05:30 PM	34	0	9	0	43	0	85	100	0	185	0	0	0	0	0	25	65	0	0	90	318
05:45 PM	58	0	11	0	69	0	71	85	0	156	0	0	0	0	0	33	66	0	0	99	324
Total	193	0	39	0	232	0	295	382	0	652	0	0	0	0	0	116	247	0	0	363	1247
Grand Total	380	0	76	0	456	0	438	658	0	1097	0	0	0	0	0	186	426	0	0	614	2167
Approach %	83.3	0	16.7	0		0	39.9	60.1	0		0	0	0	0	0	30.3	89.7	0	0		
Total %	17.5	0	3.6	0	21	0	20.2	30.4	0	50.6	0	0	0	0	0	8.5	19.9	0	0	28.5	

Start Time	Old Henry Road From North					Old Henry Road From East					From South					Factory Lane From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 05:00 PM																					
05:00 PM	46	0	12	0	58	0	50	86	0	136	0	0	0	0	0	30	55	0	0	85	279
05:15 PM	55	0	7	0	62	0	84	91	0	175	0	0	0	0	0	28	61	0	0	89	326
05:30 PM	34	0	9	0	43	0	85	100	0	185	0	0	0	0	0	25	65	0	0	90	318
05:45 PM	58	0	11	0	69	0	71	85	0	156	0	0	0	0	0	33	66	0	0	99	324
Total Volume	193	0	39	0	232	0	290	362	0	652	0	0	0	0	0	116	247	0	0	363	1247
% App. Total	83.2	0	16.8	0		0	44.5	55.5	0		0	0	0	0	0	32	68	0	0		
PHF	832	0.000	813	0.000	841	0.000	853	905	0.000	881	0.000	0.000	0.000	0.000	0.000	879	936	0.000	0.000	917	956

TABLE 8
WEEKDAY PEAK HOUR COUNTS
 Old Henry Road @ Arnold Palmer Boulevard/Hamilton Springs Drive
 Old Henry Crossing Traffic Study

Nov-14 Time Interval	Old Henry Parkway						Arnold Palmer Blvd			Hamilton Springs Dr			Totals	
	EB Left	EB Through	EB Right	WB Left	WB Through	WB Right	EB Left	EB Through	EB Right	EB Left	EB Through	EB Right	Quarter Hour	Hourly
7:00-7:15 AM	0	13	1	2	160	0	29	0	2	0	0	0	207	1
7:15-7:30 AM	0	18	4	8	186	0	25	0	3	0	0	0	244	1
7:30-7:45 AM	0	45	5	3	147	0	34	0	7	0	0	1	242	1
7:45-8:00 AM	0	39	6	8	149	0	29	0	6	0	0	2	239	932
8:00-8:15 AM	0	46	12	6	131	0	29	0	4	0	0	0	228	953
8:15-8:30 AM	1	36	10	3	109	0	17	0	3	0	0	2	181	890
8:30-8:45 AM	0	19	13	3	125	0	22	0	3	0	0	0	185	833
8:45-9:00 AM	1	39	12	5	101	0	26	0	11	1	0	0	196	790
TOTAL	2	255	63	38	1108	0	211	0	39	1	0	5	1722	---
2014 A.M. PEAK HR	0	148	27	25	613	0	117	0	20	0	0	3		
2016 A.M. PEAK HR	0	154	28	26	638	0	122	0	21	0	0	3		
2018 A.M. PEAK HR	0	160	29	27	664	0	127	0	22	0	0	3		
4:00-4:15 AM	0	88	17	9	61	0	18	1	3	0	0	2	199	---
4:15-4:30 AM	1	114	16	3	59	0	18	0	3	1	0	0	215	---
4:30-4:45 AM	0	90	32	6	77	0	19	0	4	0	0	0	228	---
4:45-5:00 AM	1	140	17	8	74	0	11	0	10	0	0	1	262	904
5:00-5:15 AM	1	179	20	9	67	0	17	0	10	0	0	1	304	1009
5:15-5:30 AM	1	163	23	9	80	1	15	0	5	0	0	1	298	1092
5:30-5:45 AM	0	149	22	9	63	0	18	0	2	0	0	1	264	1128
5:45-6:00 AM	1	137	18	3	67	0	17	0	9	0	0	0	252	1118
TOTAL	5	1060	165	56	548	1	133	1	46	1	0	6	2022	---
2014 P.M. PEAK HR	3	631	82	35	284	1	61	0	27	0	0	4		
2016 P.M. PEAK HR	3	656	85	36	295	1	63	0	28	0	0	4		
2018 P.M. PEAK HR	3	683	89	38	307	1	66	0	29	0	0	4		

JACOBS
 11940 Highway 42, Suite 1
 Goshen, KY 40026

Counted by: Andy Wolak

File Name : OldHenryBushFarmAM
 Site Code : 00062151
 Start Date : 6/2/2015
 Page No : 1

Groups Printed- Unshifted

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	10	142	0	152	121	0	5	126	2	16	16	34	0	1	10	11	323
07:15 AM	8	169	1	178	119	0	9	128	2	12	36	50	0	0	9	9	365
07:30 AM	8	150	0	158	155	0	12	167	2	33	34	69	0	3	19	22	416
07:45 AM	14	158	0	172	161	1	9	171	1	20	42	63	2	1	14	17	423
Total	40	619	1	660	556	1	35	592	7	81	128	216	2	5	52	59	1527
08:00 AM	24	147	1	172	153	2	11	166	2	34	29	65	1	1	16	18	421
08:15 AM	23	149	0	172	146	1	5	152	2	28	33	63	0	0	23	23	410
08:30 AM	14	139	0	153	127	0	7	134	3	25	62	90	0	2	12	14	391
08:45 AM	46	126	2	174	149	1	13	163	4	31	74	109	1	3	11	15	461
Total	107	561	3	671	575	4	36	615	11	118	198	327	2	6	62	70	1683
Grand Total	147	1180	4	1331	1131	5	71	1207	18	199	326	543	4	11	114	129	3210
Apprch %	11	88.7	0.3		93.7	0.4	5.9		3.3	36.6	60		3.1	8.5	88.4		
Total %	4.6	36.8	0.1	41.5	35.2	0.2	2.2	37.6	0.6	6.2	10.2	16.9	0.1	0.3	3.6		4

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 08:00 AM																	
08:00 AM	24	147	1	172	153	2	11	166	2	34	29	65	1	1	16	18	421
08:15 AM	23	149	0	172	146	1	5	152	2	28	33	63	0	0	23	23	410
08:30 AM	14	139	0	153	127	0	7	134	3	25	62	90	0	2	12	14	391
08:45 AM	46	126	2	174	149	1	13	163	4	31	74	109	1	3	11	15	461
Total Volume	107	561	3	671	575	4	36	615	11	118	198	327	2	6	62	70	1683
% App. Total	15.9	83.6	0.4		93.5	0.7	5.9		3.4	36.1	60.6		2.9	8.6	88.6		
PHF	582	941	375	964	940	500	692	926	688	868	669	750	500	500	674	761	913

Groups Printed- Unshifted

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	19	57	1	77	97	4	21	122	13	99	105	217	0	1	4	5	421
04:15 PM	12	65	1	78	89	3	11	103	5	93	108	206	0	0	7	7	394
04:30 PM	20	65	1	86	67	0	15	82	15	126	99	240	0	1	7	8	416
04:45 PM	32	83	2	117	78	0	15	93	10	134	123	267	2	0	9	11	488
Total	83	270	5	358	331	7	62	400	43	452	435	930	2	2	27	31	1719
05:00 PM	27	91	2	120	107	1	11	119	25	175	145	345	0	3	6	9	593
05:15 PM	16	86	3	105	81	1	31	113	15	185	141	341	1	3	8	12	571
05:30 PM	18	81	2	101	77	1	12	90	13	175	131	319	2	3	12	17	527
05:45 PM	22	80	2	104	86	2	11	99	7	161	135	303	1	2	12	15	521
Total	83	338	9	430	351	5	65	421	60	696	552	1308	4	11	38	53	2212
Grand Total	166	608	14	788	682	12	127	821	103	1148	987	2238	6	13	65	84	3931
Apprch %	21.1	77.2	1.8		83.1	1.5	15.5		4.6	51.3	44.1		7.1	15.5	77.4		
Total %	4.2	15.5	0.4	20	17.3	0.3	3.2	20.9	2.6	29.2	25.1	56.9	0.2	0.3	1.7	2.1	

Start Time	Old Henry Road From North				Bush Farm Road From East				Old Henry Road From South				Bush Farm Road From West				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	27	91	2	120	107	1	11	119	25	175	145	345	0	3	6	9	593
05:15 PM	16	86	3	105	81	1	31	113	15	185	141	341	1	3	8	12	571
05:30 PM	18	81	2	101	77	1	12	90	13	175	131	319	2	3	12	17	527
05:45 PM	22	80	2	104	86	2	11	99	7	161	135	303	1	2	12	15	521
Total Volume	83	338	9	430	351	5	65	421	60	696	552	1308	4	11	38	53	2212
% App. Total	19.3	78.6	2.1		83.4	1.2	15.4		4.6	53.2	42.2		7.5	20.8	71.7		
PHF	769	929	750	896	820	625	524	884	600	941	952	948	500	917	792	779	933

Appendix B. Highway Capacity Software Printouts

HCS 2010 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency	Jacobs					Duration, h	0.25								
Analyst	D Zimmerman		Analysis Date	Jul 21, 2015		Area Type	Other								
Jurisdiction			Time Period	AM Peak		PHF	0.95								
Intersection	Factory Lane		Analysis Year	2015		Analysis Period	1 > 7:00								
File Name	Factory AM 15.xus														
Project Description	Ball Homes														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				39	40	120	505	76	38	132	432	189	17	974	19
Signal Information															
Cycle, s	88.1	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	On	Green	1.4	5.0	30.6	15.0	10.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	4.3	4.0	4.0	0.0					
				Red	3.0	0.0	1.6	2.2	3.5	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4	3	8	1	6	5	2				
Case Number					7.3	1.0	3.0	1.1	3.0	1.1	4.0				
Phase Duration, s					17.5	21.2	38.7	13.0	41.5	7.9	36.5				
Change Period, (Y+R _c), s					7.5	6.2	7.5	6.5	5.9	6.5	5.9				
Max Allow Headway (MAH), s					4.4	4.0	4.4	4.0	3.7	4.0	3.7				
Queue Clearance Time (q _s), s					8.2	17.0	4.5	6.3	9.6	2.6	24.3				
Green Extension Time (g _e), s					1.4	0.0	1.4	0.4	6.5	0.0	6.3				
Phase Call Probability					1.00	1.00	1.00	0.97	1.00	0.35	1.00				
Max Out Probability					0.00	1.00	0.00	0.00	0.01	0.00	0.04				
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	1	6	16	5	2	12
Adjusted Flow Rate (v), veh/h				83	126	532	80	40	139	455	199	18	524	521	
Adjusted Saturation Flow Rate (s), veh/h/ln				1623	1594	1792	1881	1594	1792	1791	1594	1792	1881	1868	
Queue Service Time (g _s), s				2.0	6.2	15.0	2.5	1.4	4.3	7.6	5.4	0.6	22.3	22.3	
Cycle Queue Clearance Time (g _c), s				4.0	6.2	15.0	2.5	1.4	4.3	7.6	5.4	0.6	22.3	22.3	
Green Ratio (g/C)				0.11	0.19	0.31	0.35	0.37	0.43	0.40	0.57	0.36	0.35	0.35	
Capacity (c), veh/h				245	298	480	666	590	265	1448	916	389	653	648	
Volume-to-Capacity Ratio (X)				0.339	0.424	1.107	0.120	0.068	0.525	0.314	0.217	0.046	0.803	0.803	
Available Capacity (c _a), veh/h				776	840	480	854	749	946	2234	1266	767	1174	1166	
Back of Queue (Q), veh/ln (50th percentile)				1.6	2.3	12.4	1.0	0.5	1.7	2.9	1.6	0.2	9.4	9.3	
Queue Storage Ratio (RQ) (50th percentile)				0.08	0.12	1.49	0.12	0.07	0.10	0.07	0.13	0.01	0.24	0.24	
Uniform Delay (d ₁), s/veh				36.3	31.7	31.1	19.2	17.9	19.7	17.9	9.1	18.2	26.1	26.1	
Incremental Delay (d ₂), s/veh				1.0	1.2	73.5	0.1	0.0	1.6	0.1	0.1	0.0	2.0	2.0	
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh				37.3	32.8	104.6	19.3	18.0	21.3	18.0	9.2	18.2	28.1	28.1	
Level of Service (LOS)				D	C	F	B	B	C	B	A	B	C	C	
Approach Delay, s/veh / LOS				34.6	C	88.8	F	16.4	B	27.9	C				
Intersection Delay, s/veh / LOS				39.7						D					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				3.0	C	2.8	C	2.4	B	2.3	B				
Bicycle LOS Score / LOS				0.8	A	1.6	A	1.1	A	1.4	A				

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information												
Agency	Jacobs			Duration, h	0.25											
Analyst	D Zimmerman		Analysis Date	Jul 21, 2015												
Jurisdiction		Time Period	AM Peak													
Intersection	Factory Lane		Analysis Year	2022 No Build												
File Name	Factory AM 22 NB.xus			Analysis Period	1> 7:00											
Project Description	Ball Homes															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				46	47	141	593	89	45	155	508	222	20	1144	22	
Signal Information																
Cycle, s	99.7	Reference Phase	2													
Offset, s	0	Reference Point	End	Green	1.8	5.8	39.1	15.0	11.9	0.0						
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	0.0	4.3	4.0	4.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	0.0	1.6	2.2	3.5	0.0						
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4	3	8	1	6	5	2					
Case Number					7.3	1.0	3.0	1.1	3.0	1.1	4.0					
Phase Duration, s					19.4	21.2	40.6	14.1	50.9	8.3	45.0					
Change Period, (Y+Rc), s					7.5	6.2	7.5	6.5	5.9	6.5	5.9					
Max Allow Headway (MAH), s					4.4	4.0	4.4	4.0	3.7	4.0	3.7					
Queue Clearance Time (gs), s					10.2	17.0	5.5	7.2	11.6	2.7	31.5					
Green Extension Time (ge), s					1.6	0.0	1.6	0.5	8.4	0.0	7.6					
Phase Call Probability					1.00	1.00	1.00	0.99	1.00	0.44	1.00					
Max Out Probability					0.00	1.00	0.00	0.00	0.03	0.00	0.17					
Movement Group Results				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement				7	4	14	3	8	18	1	6	16	5	2	12	
Adjusted Flow Rate (v), veh/h				98	148	624	94	47	163	535	234	21	616	612		
Adjusted Saturation Flow Rate (s), veh/h/ln				1603	1594	1792	1881	1594	1792	1791	1594	1792	1881	1868		
Queue Service Time (gs), s				3.5	8.2	15.0	3.5	2.0	5.2	9.6	6.8	0.7	29.5	29.5		
Cycle Queue Clearance Time (gc), s				5.5	8.2	15.0	3.5	2.0	5.2	9.6	6.8	0.7	29.5	29.5		
Green Ratio (g/C)				0.12	0.20	0.29	0.33	0.35	0.48	0.45	0.60	0.41	0.39	0.39		
Capacity (c), veh/h				245	312	429	624	557	253	1616	959	398	738	733		
Volume-to-Capacity Ratio (X)				0.400	0.476	1.454	0.150	0.085	0.644	0.331	0.244	0.053	0.834	0.834		
Available Capacity (ca), veh/h				681	761	429	755	668	835	1975	1119	725	1038	1030		
Back of Queue (Q), veh/ln (50th percentile)				2.2	3.2	28.8	1.5	0.7	2.1	3.7	2.1	0.3	12.9	12.9		
Queue Storage Ratio (RQ) (50th percentile)				0.11	0.16	3.45	0.18	0.10	0.13	0.09	0.17	0.02	0.33	0.32		
Uniform Delay (d1), s/veh				41.0	35.6	35.8	23.4	21.8	21.6	17.7	9.3	17.7	27.4	27.4		
Incremental Delay (d2), s/veh				1.3	1.4	216.9	0.1	0.1	2.7	0.1	0.1	0.1	3.9	4.0		
Initial Queue Delay (d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay (d), s/veh				42.3	37.0	252.7	23.6	21.8	24.3	17.8	9.4	17.8	31.3	31.3		
Level of Service (LOS)				D	D	F	C	C	C	B	A	B	C	C		
Approach Delay, s/veh / LOS				39.1	D	210.4	F		16.8	B		31.1	C			
Intersection Delay, s/veh / LOS				70.5						E						
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS				3.0	C	2.8	C		2.4	B		2.3	B			
Bicycle LOS Score / LOS				0.9	A	1.8	A		1.3	A		1.5	A			

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information												
Agency	Jacobs			Duration, h	0.25											
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015	Area Type	Other											
Jurisdiction		Time Period	AM Peak	PHF	0.95											
Intersection	Factory Lane	Analysis Year	2022 Build	Analysis Period	1> 7:00											
File Name	Factory AM 22 B.xus															
Project Description	Ball Homes															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				46	53	141	697	105	53	155	508	253	23	1144	22	
Signal Information																
Cycle, s	100.1	Reference Phase	2													
Offset, s	0	Reference Point	End													
Uncoordinated	Yes	Simult. Gap E/W	On	Green	2.0	5.7	39.4	15.0	12.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	0.0	4.3	4.0	4.0	0.0						
				Red	3.0	0.0	1.6	2.2	3.5	0.0						
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4	3	8	1	6	5	2					
Case Number					7.3	1.0	3.0	1.1	3.0	1.1	4.0					
Phase Duration, s					19.5	21.2	40.7	14.1	50.9	8.5	45.3					
Change Period, (Y+R _c), s					7.5	6.2	7.5	6.5	5.9	6.5	5.9					
Max Allow Headway (MAH), s					4.4	4.0	4.4	4.0	3.7	4.0	3.7					
Queue Clearance Time (g _s), s					10.3	17.0	6.2	7.2	11.7	2.8	31.6					
Green Extension Time (g _e), s					1.7	0.0	1.8	0.5	8.7	0.0	7.7					
Phase Call Probability					1.00	1.00	1.00	0.99	1.00	0.49	1.00					
Max Out Probability					0.00	1.00	0.00	0.00	0.03	0.00	0.18					
Movement Group Results				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement				7	4	14	3	8	18	1	6	16	5	2	12	
Adjusted Flow Rate (v), veh/h					104	148	734	111	56	163	535	266	24	616	612	
Adjusted Saturation Flow Rate (s), veh/h/ln					1607	1594	1792	1881	1594	1792	1791	1594	1792	1881	1868	
Queue Service Time (g _s), s					3.9	8.3	15.0	4.2	2.4	5.2	9.7	8.0	0.8	29.6	29.6	
Cycle Queue Clearance Time (g _c), s					5.9	8.3	15.0	4.2	2.4	5.2	9.7	8.0	0.8	29.6	29.6	
Green Ratio (g/C)					0.12	0.20	0.29	0.33	0.35	0.48	0.45	0.60	0.41	0.39	0.39	
Capacity (c), veh/h					245	313	423	624	560	254	1612	956	400	740	735	
Volume-to-Capacity Ratio (X)					0.425	0.474	1.733	0.177	0.100	0.643	0.332	0.279	0.061	0.832	0.833	
Available Capacity (c _a), veh/h					679	759	423	751	668	832	1967	1114	722	1033	1026	
Back of Queue (Q), veh/ln (50th percentile)					2.4	3.2	42.8	1.8	0.8	2.1	3.7	2.4	0.3	13.0	12.9	
Queue Storage Ratio (RQ) (50th percentile)					0.12	0.16	5.13	0.21	0.12	0.13	0.09	0.21	0.02	0.33	0.33	
Uniform Delay (d ₁), s/veh					41.2	35.7	35.8	23.8	21.8	21.7	17.8	9.6	17.7	27.4	27.4	
Incremental Delay (d ₂), s/veh					1.4	1.3	339.5	0.1	0.1	2.7	0.1	0.1	0.1	3.9	4.0	
Initial Queue Delay (d ₃), s/veh					0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh					42.7	37.0	375.3	23.9	21.9	24.4	17.9	9.8	17.7	31.3	31.4	
Level of Service (LOS)					D	D	F	C	C	C	B	A	B	C	C	
Approach Delay, s/veh / LOS				39.3	D	310.3	F	16.8	B	31.1	C					
Intersection Delay, s/veh / LOS				102.2						F						
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS				3.0	C	2.8	C	2.4	B	2.3	B					
Bicycle LOS Score / LOS				0.9	A	2.0	A	1.3	A	1.5	A					

HCS 2010 Signalized Intersection Results Summary

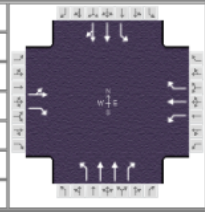
General Information				Intersection Information												
Agency	Jacobs			Duration, h	0.25											
Analyst	D Zimmerman			Analysis Date	Jul 21, 2015											
Jurisdiction				Time Period	PM Peak											
Intersection	Factory Lane			Analysis Year	2015											
File Name	Factory PM 15.xus			Analysis Period	1> 7:00											
Project Description	Ball Homes															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				113	85	197	249	209	40	188	965	606	28	424	97	
Signal Information																
Cycle, s	105.5	Reference Phase	2													
Offset, s	0	Reference Point	End													
Uncoordinated	Yes	Simult. Gap E/W	On	Green	2.3	1.0	35.6	14.1	19.8	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	4.0	4.0	0.0						
				Red	3.0	3.0	1.6	2.2	3.5	0.0						
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4	3	8	1	6	5	2					
Case Number					7.3	1.0	3.0	1.1	3.0	1.1	4.0					
Phase Duration, s					27.3	20.3	47.6	16.3	49.1	8.8	41.5					
Change Period, (Y+R _c), s					7.5	6.2	7.5	6.5	5.9	6.5	5.9					
Max Allow Headway (MAH), s					4.5	4.0	4.5	4.0	3.7	4.0	3.7					
Queue Clearance Time (g _s), s					16.9	13.9	10.7	9.2	34.2	3.1	14.5					
Green Extension Time (g _e), s					2.5	0.1	3.0	0.6	8.9	0.0	10.8					
Phase Call Probability					1.00	1.00	1.00	1.00	1.00	0.58	1.00					
Max Out Probability					0.01	1.00	0.00	0.00	0.30	0.00	0.07					
Movement Group Results				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement				7	4	14	3	8	18	1	6	16	5	2	12	
Adjusted Flow Rate (v), veh/h				208	207	262	220	42	198	1016	638	29	282	267		
Adjusted Saturation Flow Rate (s), veh/h/ln				1402	1594	1792	1881	1594	1792	1791	1594	1792	1881	1760		
Queue Service Time (g _e), s				14.7	11.3	11.9	8.7	1.7	7.2	24.7	32.2	1.1	12.3	12.5		
Cycle Queue Clearance Time (g _c), s				14.9	11.3	11.9	8.7	1.7	7.2	24.7	32.2	1.1	12.3	12.5		
Green Ratio (g/C)				0.19	0.28	0.34	0.38	0.40	0.45	0.41	0.54	0.36	0.34	0.34		
Capacity (c), veh/h				317	449	368	715	641	426	1466	865	195	636	595		
Volume-to-Capacity Ratio (X)				0.657	0.462	0.712	0.308	0.066	0.465	0.693	0.737	0.151	0.443	0.448		
Available Capacity (c _a), veh/h				583	753	384	715	641	937	1866	1043	495	980	917		
Back of Queue (Q), veh/ln (50th percentile)				5.2	4.3	5.4	3.7	0.6	2.9	9.9	11.0	0.5	5.3	5.1		
Queue Storage Ratio (RQ) (50th percentile)				0.26	0.22	0.65	0.44	0.09	0.17	0.25	0.92	0.03	0.13	0.13		
Uniform Delay (d ₁), s/veh				40.8	31.3	28.6	23.0	19.4	19.2	25.7	18.4	23.8	27.2	27.3		
Incremental Delay (d ₂), s/veh				2.8	0.9	5.8	0.2	0.0	0.8	0.7	2.1	0.4	0.4	0.5		
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh				43.6	32.2	34.4	23.2	19.4	20.0	26.4	20.5	24.2	27.6	27.7		
Level of Service (LOS)				D	C	C	C	B	C	C	C	C	C	C		
Approach Delay, s/veh / LOS				37.9	D	28.5	C	23.7	C	27.5	C					
Intersection Delay, s/veh / LOS				26.9						C						
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS				3.0	C	2.8	C	2.4	B	2.3	B					
Bicycle LOS Score / LOS				1.2	A	1.4	A	2.0	B	1.0	A					

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information												
Agency	Jacobs			Duration, h	0.25											
Analyst	D Zimmerman		Analysis Date	Jul 21, 2015		Area Type	Other									
Jurisdiction				Time Period	PM Peak		PHF				0.95					
Intersection	Factory Lane		Analysis Year	2022 No Build		Analysis Period	1> 7:00									
File Name	Factory PM 22 NB.xus															
Project Description	Ball Homes															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				133	100	231	293	246	47	221	1134	712	33	498	114	
Signal Information																
Cycle, s	123.5	Reference Phase	2													
Offset, s	0	Reference Point	End	Green	2.8	3.1	44.4	15.0	25.5	0.0						
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	3.5	4.3	4.0	4.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.0	3.0	1.6	2.2	3.5	0.0						
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Assigned Phase					4	3	8	1	6	5	2					
Case Number					7.3	1.0	3.0	1.1	3.0	1.1	4.0					
Phase Duration, s					33.0	21.2	54.2	18.9	59.9	9.3	50.3					
Change Period, (Y+R _c), s					7.5	6.2	7.5	6.5	5.9	6.5	5.9					
Max Allow Headway (MAH), s					4.5	4.0	4.5	4.0	3.7	4.0	3.7					
Queue Clearance Time (g _s), s					23.5	17.0	14.2	11.6	50.3	3.5	19.1					
Green Extension Time (g _e), s					2.0	0.0	3.6	0.8	3.7	0.0	14.0					
Phase Call Probability					1.00	1.00	1.00	1.00	1.00	0.70	1.00					
Max Out Probability					0.07	1.00	0.01	0.00	0.96	0.00	0.22					
Movement Group Results				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Assigned Movement				7	4	14	3	8	18	1	6	16	5	2	12	
Adjusted Flow Rate (v), veh/h				245	243	308	259	49	233	1194	749	35	332	313		
Adjusted Saturation Flow Rate (s), veh/h/ln				1361	1594	1792	1881	1594	1792	1791	1594	1792	1881	1760		
Queue Service Time (g _s), s				21.5	15.4	15.0	12.2	2.4	9.6	34.7	48.3	1.5	16.9	17.1		
Cycle Queue Clearance Time (g _c), s				21.5	15.4	15.0	12.2	2.4	9.6	34.7	48.3	1.5	16.9	17.1		
Green Ratio (g/C)				0.21	0.31	0.34	0.38	0.40	0.48	0.44	0.56	0.38	0.36	0.36		
Capacity (c), veh/h				328	491	319	712	640	413	1567	891	165	676	633		
Volume-to-Capacity Ratio (X)				0.749	0.496	0.968	0.364	0.077	0.563	0.762	0.841	0.211	0.490	0.494		
Available Capacity (c _a), veh/h				487	677	319	712	640	813	1595	904	414	838	784		
Back of Queue (Q), veh/ln (50th percentile)				7.4	5.9	10.7	5.4	0.9	4.0	14.5	18.1	0.7	7.5	7.1		
Queue Storage Ratio (RQ) (50th percentile)				0.38	0.30	1.29	0.65	0.13	0.24	0.37	1.52	0.04	0.19	0.18		
Uniform Delay (d ₁), s/veh				47.4	34.9	36.7	27.6	22.8	21.5	29.3	22.7	27.3	30.8	30.8		
Incremental Delay (d ₂), s/veh				4.2	0.9	41.7	0.3	0.1	1.2	2.1	7.0	0.6	0.5	0.5		
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh				51.6	35.9	78.5	28.0	22.9	22.8	31.4	29.7	27.9	31.2	31.3		
Level of Service (LOS)				D	D	E	C	C	C	C	C	C	C	C		
Approach Delay, s/veh / LOS				43.8	D	52.8	D		29.9	C		31.1	C			
Intersection Delay, s/veh / LOS				35.4						D						
Multimodal Results				EB			WB			NB			SB			
Pedestrian LOS Score / LOS				3.0	C	2.8	C		2.4	B		2.3	B			
Bicycle LOS Score / LOS				1.3	A	1.5	A		2.3	B		1.0	A			

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	Jacobs			Duration, h	0.25		
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015	Area Type	Other		
Jurisdiction		Time Period	PM Peak	PHF	0.95		
Intersection	Factory Lane	Analysis Year	2022 Build	Analysis Period	1> 7:00		
File Name	Factory PM 22 B.xus						
Project Description	Ball Homes						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	133	116	231	331	278	54	221	1134	828	39	498	114

Signal Information				Signal Timing (s)									
Cycle, s	126.9	Reference Phase	2										
Offset, s	0	Reference Point	End										
Uncoordinated	Yes	Simult. Gap E/W	On	Green	3.1	3.2	45.3	15.0	27.7	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.5	3.5	4.3	4.0	4.0	0.0			
				Red	3.0	3.0	1.6	2.2	3.5	0.0			

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4	3	8	1	6	5	2
Case Number		7.3	1.0	3.0	1.1	3.0	1.1	4.0
Phase Duration, s		35.2	21.2	56.4	19.2	60.9	9.6	51.2
Change Period, (Y+R _c), s		7.5	6.2	7.5	6.5	5.9	6.5	5.9
Max Allow Headway (MAH), s		4.5	4.0	4.5	4.0	3.8	4.0	3.8
Queue Clearance Time (g _s), s		25.7	17.0	16.4	12.0	57.0	3.8	19.6
Green Extension Time (g _e), s		2.0	0.0	3.8	0.8	0.0	0.1	15.4
Phase Call Probability		1.00	1.00	1.00	1.00	1.00	0.76	1.00
Max Out Probability		0.14	1.00	0.02	0.00	1.00	0.00	0.28

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Assigned Movement	7	4	14	3	8	18	1	6	16	5	2	12
Adjusted Flow Rate (v), veh/h		262	243	348	293	57	233	1194	872	41	332	313
Adjusted Saturation Flow Rate (s), veh/h/ln		1358	1594	1792	1881	1594	1792	1791	1594	1792	1881	1760
Queue Service Time (g _s), s		23.7	15.6	15.0	14.4	2.8	10.0	35.9	55.0	1.8	17.5	17.6
Cycle Queue Clearance Time (g _c), s		23.7	15.6	15.0	14.4	2.8	10.0	35.9	55.0	1.8	17.5	17.6
Green Ratio (g/C)		0.22	0.32	0.35	0.39	0.41	0.47	0.43	0.55	0.38	0.36	0.36
Capacity (c), veh/h		340	509	309	725	653	409	1553	880	163	672	628
Volume-to-Capacity Ratio (X)		0.771	0.478	1.127	0.403	0.087	0.569	0.769	0.991	0.252	0.494	0.497
Available Capacity (c _a), veh/h		472	663	309	725	653	794	1553	880	402	815	763
Back of Queue (Q), veh/ln (50th percentile)		8.3	6.0	14.9	6.3	1.0	4.1	15.2	30.4	0.8	7.8	7.3
Queue Storage Ratio (RQ) (50th percentile)		0.42	0.30	1.79	0.76	0.15	0.25	0.38	2.55	0.05	0.20	0.19
Uniform Delay (d ₁), s/veh		48.0	34.7	37.7	28.4	22.9	22.4	30.5	28.1	28.3	31.8	31.9
Incremental Delay (d ₂), s/veh		5.8	0.8	89.9	0.4	0.1	1.2	2.3	28.0	0.8	0.5	0.5
Initial Queue Delay (d ₃), s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh		53.8	35.6	127.6	28.7	23.0	23.6	32.9	56.1	29.1	32.3	32.4
Level of Service (LOS)		D	D	F	C	C	C	C	E	C	C	C
Approach Delay, s/veh / LOS	45.0		D	77.6		E	40.8		D	32.2		C
Intersection Delay, s/veh / LOS	46.0						D					

Multimodal Results	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	3.0		C	2.8		C	2.4		B	2.3		B
Bicycle LOS Score / LOS	1.3		A	1.6		A	2.4		B	1.1		A

TWO-WAY STOP CONTROL SUMMARY								
General Information			Site Information					
Analyst	<i>D Zimmerman</i>		Intersection					
Agency/Co.	<i>Jacobs</i>		Jurisdiction					
Date Performed	<i>7/21/2015</i>		Analysis Year		<i>2015</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>			North/South Street: <i>Colonial Springs</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	<i>6</i>	<i>167</i>			<i>322</i>	<i>14</i>		
Peak-Hour Factor, PHF	<i>0.91</i>	<i>0.91</i>	<i>1.00</i>	<i>1.00</i>	<i>0.91</i>	<i>0.91</i>		
Hourly Flow Rate, HFR (veh/h)	<i>6</i>	<i>183</i>	<i>0</i>	<i>0</i>	<i>353</i>	<i>15</i>		
Percent Heavy Vehicles	<i>1</i>	--	--	<i>0</i>	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>		
Configuration	<i>L</i>	<i>T</i>				<i>TR</i>		
Upstream Signal		<i>0</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				<i>26</i>		<i>40</i>		
Peak-Hour Factor, PHF	<i>1.00</i>	<i>1.00</i>	<i>1.00</i>	<i>0.91</i>	<i>1.00</i>	<i>0.91</i>		
Hourly Flow Rate, HFR (veh/h)	<i>0</i>	<i>0</i>	<i>0</i>	<i>28</i>	<i>0</i>	<i>43</i>		
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>1</i>		
Percent Grade (%)	<i>0</i>			<i>0</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>						<i>LR</i>	
v (veh/h)	<i>6</i>						<i>71</i>	
C (m) (veh/h)	<i>1196</i>						<i>636</i>	
v/c	<i>0.01</i>						<i>0.11</i>	
95% queue length	<i>0.02</i>						<i>0.38</i>	
Control Delay (s/veh)	<i>8.0</i>						<i>11.4</i>	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--				<i>11.4</i>		
Approach LOS	--	--				<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>AM Peak</i>			<i>2022 No Build</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Colonial Springs</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	6	196			378	14		
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.91	0.91		
Hourly Flow Rate, HFR (veh/h)	6	215	0	0	415	15		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				26		40		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.91	1.00	0.91		
Hourly Flow Rate, HFR (veh/h)	0	0	0	28	0	43		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	6						71	
C (m) (veh/h)	1135						587	
v/c	0.01						0.12	
95% queue length	0.02						0.41	
Control Delay (s/veh)	8.2						12.0	
LOS	A						B	
Approach Delay (s/veh)	--	--				12.0		
Approach LOS	--	--				B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Colonial Springs</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	6	236			506	14		
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.91	0.91		
Hourly Flow Rate, HFR (veh/h)	6	259	0	0	556	15		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				26		40		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.91	1.00	0.91		
Hourly Flow Rate, HFR (veh/h)	0	0	0	28	0	43		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	6						71	
C (m) (veh/h)	1007						492	
v/c	0.01						0.14	
95% queue length	0.02						0.50	
Control Delay (s/veh)	8.6						13.5	
LOS	A						B	
Approach Delay (s/veh)	--	--					13.5	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>PM Peak</i>			<i>2015</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Colonial Springs</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	40	471			349	34		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.94	0.94		
Hourly Flow Rate, HFR (veh/h)	42	501	0	0	371	36		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0				0	
Lanes	1	1	0	0	1	0		
Configuration	L	T					TR	
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				55		22		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94		
Hourly Flow Rate, HFR (veh/h)	0	0	0	58	0	23		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	42						81	
C (m) (veh/h)	1157						447	
v/c	0.04						0.18	
95% queue length	0.11						0.65	
Control Delay (s/veh)	8.2						14.8	
LOS	A						B	
Approach Delay (s/veh)	--	--					14.8	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>PM Peak</i>			<i>2022 No Build</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Colonial Springs</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	40	553			410	34		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.94	0.94		
Hourly Flow Rate, HFR (veh/h)	42	588	0	0	436	36		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0				0	
Lanes	1	1	0	0	1	0		
Configuration	L	T					TR	
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				55		22		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94		
Hourly Flow Rate, HFR (veh/h)	0	0	0	58	0	23		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	42						81	
C (m) (veh/h)	1095						398	
v/c	0.04						0.20	
95% queue length	0.12						0.75	
Control Delay (s/veh)	8.4						16.3	
LOS	A						C	
Approach Delay (s/veh)	--	--				16.3		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Colonial Springs</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	40	689			487	34		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.94	0.94		
Hourly Flow Rate, HFR (veh/h)	42	732	0	0	518	36		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				55		22		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94		
Hourly Flow Rate, HFR (veh/h)	0	0	0	58	0	23		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	42						81	
C (m) (veh/h)	1021						338	
v/c	0.04						0.24	
95% queue length	0.13						0.92	
Control Delay (s/veh)	8.7						19.0	
LOS	A						C	
Approach Delay (s/veh)	--	--				19.0		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	D Zimmerman			Intersection				
Agency/Co.	Jacobs			Jurisdiction				
Date Performed	7/21/2015			Analysis Year	2015			
Analysis Time Period	AM Peak							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	18	139	18	1	184	3		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	20	157	20	1	209	3		
Percent Heavy Vehicles	1	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	63	1	15	73	0	5		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	71	1	17	82	0	5		
Percent Heavy Vehicles	1	1	1	1	1	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	20	1	89			87		
C (m) (veh/h)	1364	1405	635			602		
v/c	0.01	0.00	0.14			0.14		
95% queue length	0.04	0.00	0.49			0.50		
Control Delay (s/veh)	7.7	7.6	11.6			12.0		
LOS	A	A	B			B		
Approach Delay (s/veh)	--	--	11.6			12.0		
Approach LOS	--	--	B			B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 No Build</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	<i>18</i>	<i>163</i>	<i>18</i>	<i>1</i>	<i>216</i>	<i>3</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>20</i>	<i>185</i>	<i>20</i>	<i>1</i>	<i>245</i>	<i>3</i>		
Percent Heavy Vehicles	<i>1</i>	--	--	<i>1</i>	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>1</i>	<i>1</i>	<i>0</i>		
Configuration	<i>L</i>		<i>TR</i>	<i>L</i>		<i>TR</i>		
Upstream Signal		<i>0</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	<i>63</i>	<i>1</i>	<i>15</i>	<i>73</i>	<i>0</i>	<i>5</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>71</i>	<i>1</i>	<i>17</i>	<i>82</i>	<i>0</i>	<i>5</i>		
Percent Heavy Vehicles	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>		
Percent Grade (%)	<i>0</i>			<i>0</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>		
Configuration		<i>LTR</i>			<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>	<i>L</i>	<i>LTR</i>			<i>LTR</i>		
v (veh/h)	<i>20</i>	<i>1</i>	<i>89</i>			<i>87</i>		
C (m) (veh/h)	<i>1324</i>	<i>1372</i>	<i>598</i>			<i>566</i>		
v/c	<i>0.02</i>	<i>0.00</i>	<i>0.15</i>			<i>0.15</i>		
95% queue length	<i>0.05</i>	<i>0.00</i>	<i>0.52</i>			<i>0.54</i>		
Control Delay (s/veh)	<i>7.8</i>	<i>7.6</i>	<i>12.1</i>			<i>12.5</i>		
LOS	<i>A</i>	<i>A</i>	<i>B</i>			<i>B</i>		
Approach Delay (s/veh)	--	--	<i>12.1</i>			<i>12.5</i>		
Approach LOS	--	--	<i>B</i>			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	<i>18</i>	<i>203</i>	<i>18</i>	<i>1</i>	<i>344</i>	<i>3</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>20</i>	<i>230</i>	<i>20</i>	<i>1</i>	<i>390</i>	<i>3</i>		
Percent Heavy Vehicles	<i>1</i>	--	--	<i>1</i>	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>1</i>	<i>1</i>	<i>0</i>		
Configuration	<i>L</i>		<i>TR</i>	<i>L</i>		<i>TR</i>		
Upstream Signal		<i>0</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	<i>63</i>	<i>1</i>	<i>15</i>	<i>73</i>	<i>0</i>	<i>5</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>71</i>	<i>1</i>	<i>17</i>	<i>82</i>	<i>0</i>	<i>5</i>		
Percent Heavy Vehicles	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>		
Percent Grade (%)	<i>0</i>			<i>0</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>		
Configuration		<i>LTR</i>			<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>	<i>L</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	<i>20</i>	<i>1</i>		<i>89</i>			<i>87</i>	
C (m) (veh/h)	<i>1171</i>	<i>1321</i>		<i>501</i>			<i>470</i>	
v/c	<i>0.02</i>	<i>0.00</i>		<i>0.18</i>			<i>0.19</i>	
95% queue length	<i>0.05</i>	<i>0.00</i>		<i>0.64</i>			<i>0.67</i>	
Control Delay (s/veh)	<i>8.1</i>	<i>7.7</i>		<i>13.7</i>			<i>14.4</i>	
LOS	<i>A</i>	<i>A</i>		<i>B</i>			<i>B</i>	
Approach Delay (s/veh)	--	--	<i>13.7</i>			<i>14.4</i>		
Approach LOS	--	--	<i>B</i>			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	D Zimmerman			Intersection				
Agency/Co.	Jacobs			Jurisdiction				
Date Performed	7/21/2015			Analysis Year	2015			
Analysis Time Period	PM Peak							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	74	254	66	9	291	24		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	84	288	75	10	330	27		
Percent Heavy Vehicles	1	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	26	0	8	6	1	36		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	29	0	9	6	1	40		
Percent Heavy Vehicles	1	1	1	1	1	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	84	10	38			47		
C (m) (veh/h)	1207	1201	393			610		
v/c	0.07	0.01	0.10			0.08		
95% queue length	0.22	0.03	0.32			0.25		
Control Delay (s/veh)	8.2	8.0	15.1			11.4		
LOS	A	A	C			B		
Approach Delay (s/veh)	--	--	15.1			11.4		
Approach LOS	--	--	C			B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 No Build</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	74	298	66	9	342	24		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	84	338	75	10	388	27		
Percent Heavy Vehicles	1	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	26	0	8	6	1	36		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	29	0	9	6	1	40		
Percent Heavy Vehicles	1	1	1	1	1	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	84	10	38			47		
C (m) (veh/h)	1149	1151	355			561		
v/c	0.07	0.01	0.11			0.08		
95% queue length	0.24	0.03	0.36			0.27		
Control Delay (s/veh)	8.4	8.2	16.4			12.0		
LOS	A	A	C			B		
Approach Delay (s/veh)	--	--	16.4			12.0		
Approach LOS	--	--	C			B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Terrace Springs Drive</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	<i>74</i>	<i>434</i>	<i>66</i>	<i>9</i>	<i>419</i>	<i>24</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>84</i>	<i>493</i>	<i>75</i>	<i>10</i>	<i>476</i>	<i>27</i>		
Percent Heavy Vehicles	<i>1</i>	--	--	<i>1</i>	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>1</i>	<i>1</i>	<i>0</i>		
Configuration	<i>L</i>		<i>TR</i>	<i>L</i>		<i>TR</i>		
Upstream Signal		<i>0</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	<i>26</i>	<i>0</i>	<i>8</i>	<i>6</i>	<i>1</i>	<i>36</i>		
Peak-Hour Factor, PHF	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>	<i>0.88</i>		
Hourly Flow Rate, HFR (veh/h)	<i>29</i>	<i>0</i>	<i>9</i>	<i>6</i>	<i>1</i>	<i>40</i>		
Percent Heavy Vehicles	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>	<i>1</i>		
Percent Grade (%)	<i>0</i>			<i>0</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>		
Configuration		<i>LTR</i>			<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>	<i>L</i>	<i>LTR</i>			<i>LTR</i>		
v (veh/h)	<i>84</i>	<i>10</i>	<i>38</i>			<i>47</i>		
C (m) (veh/h)	<i>1067</i>	<i>1009</i>	<i>283</i>			<i>481</i>		
v/c	<i>0.08</i>	<i>0.01</i>	<i>0.13</i>			<i>0.10</i>		
95% queue length	<i>0.26</i>	<i>0.03</i>	<i>0.46</i>			<i>0.32</i>		
Control Delay (s/veh)	<i>8.7</i>	<i>8.6</i>	<i>19.7</i>			<i>13.3</i>		
LOS	<i>A</i>	<i>A</i>	<i>C</i>			<i>B</i>		
Approach Delay (s/veh)	--	--	<i>19.7</i>			<i>13.3</i>		
Approach LOS	--	--	<i>C</i>			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>AM Peak</i>			<i>2022</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Main Entrance</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	38	185			173	25		
Peak-Hour Factor, PHF	0.88	0.88	1.00	1.00	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	43	210	0	0	196	28		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Raised curb</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				80		118		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.88	1.00	0.88		
Hourly Flow Rate, HFR (veh/h)	0	0	0	90	0	134		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	43						224	
C (m) (veh/h)	1351						712	
v/c	0.03						0.31	
95% queue length	0.10						1.35	
Control Delay (s/veh)	7.8						12.4	
LOS	A						B	
Approach Delay (s/veh)	--	--				12.4		
Approach LOS	--	--				B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	D Zimmerman			Intersection				
Agency/Co.	Jacobs			Jurisdiction				
Date Performed	7/21/2015			Analysis Year				
Analysis Time Period	PM Peak			2022				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Main Entrance</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	127	321			391	84		
Peak-Hour Factor, PHF	0.88	0.88	1.00	1.00	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	144	364	0	0	444	95		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				48		73		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.88	1.00	0.88		
Hourly Flow Rate, HFR (veh/h)	0	0	0	54	0	82		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	144						136	
C (m) (veh/h)	1034						439	
v/c	0.14						0.31	
95% queue length	0.48						1.30	
Control Delay (s/veh)	9.0						16.8	
LOS	A						C	
Approach Delay (s/veh)	--	--				16.8		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	2022			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Secondary Entrance</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	2	263			188	1		
Peak-Hour Factor, PHF	0.88	0.88	1.00	1.00	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	2	298	0	0	213	1		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	<i>LT</i>			<i>TR</i>				
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				6		10		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.88	1.00	0.88		
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	11		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	2						17	
C (m) (veh/h)	1362						685	
v/c	0.00						0.02	
95% queue length	0.00						0.08	
Control Delay (s/veh)	7.6						10.4	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--				10.4		
Approach LOS	--	--				<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	D Zimmerman			Intersection				
Agency/Co.	Jacobs			Jurisdiction				
Date Performed	7/21/2015			Analysis Year	2022			
Analysis Time Period	PM Peak							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Secondary Entrance</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	9	360			471	6		
Peak-Hour Factor, PHF	0.88	0.88	1.00	1.00	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	10	409	0	0	535	6		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT			TR				
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				3		4		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.88	1.00	0.88		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	4		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT					LR		
v (veh/h)	10					7		
C (m) (veh/h)	1033					388		
v/c	0.01					0.02		
95% queue length	0.03					0.06		
Control Delay (s/veh)	8.5					14.4		
LOS	A					B		
Approach Delay (s/veh)	--	--				14.4		
Approach LOS	--	--				B		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	D Zimmerman				Intersection				
Agency/Co.	Jacobs				Jurisdiction				
Date Performed	7/21/2015				Analysis Year	2015			
Analysis Time Period	AM Peak								
Project ID <i>Bail Homes Factory Lane</i>									
East/West Street: <i>Factory Lane</i>					North/South Street: <i>Old Henry Road</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R			
Volume (veh/h)	12	168	0	0	86	77			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R			
Volume (veh/h)	0	0	0	485	0	53			
%Thrus Left Lane									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR				LR		
PHF	0.95		0.95				0.95		
Flow Rate (veh/h)	188		171				565		
% Heavy Vehicles	1		1				1		
No. Lanes	1		1		0		1		
Geometry Group	1		1				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.1		0.0				0.9		
Prop. Right-Turns	0.0		0.5				0.1		
Prop. Heavy Vehicle	0.0		0.0				0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.0		-0.3				0.1		
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20		
x, initial	0.17		0.15				0.50		
hd, final value (s)	5.81		5.55				5.08		
x, final value	0.30		0.26				0.80		
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	3.8		3.6				3.1		
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	438		421				699		
Delay (s/veh)	11.32		10.53				24.92		
LOS	B		B				C		
Approach: Delay (s/veh)	11.32		10.53				24.92		
LOS	B		B				C		
Intersection Delay (s/veh)	19.49								
Intersection LOS	C								

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jaccobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>AM Peak</i>			<i>2022 No Build</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	<i>101</i>	<i>142</i>			<i>729</i>	<i>62</i>		
Peak-Hour Factor, PHF	<i>0.95</i>	<i>0.95</i>	<i>1.00</i>	<i>1.00</i>	<i>0.95</i>	<i>0.95</i>		
Hourly Flow Rate, HFR (veh/h)	<i>106</i>	<i>149</i>	<i>0</i>	<i>0</i>	<i>767</i>	<i>65</i>		
Percent Heavy Vehicles	<i>1</i>	--	--	<i>0</i>	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>		
Configuration	<i>L</i>	<i>T</i>					<i>TR</i>	
Upstream Signal		<i>0</i>			<i>0</i>			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	<i>14</i>		<i>197</i>					
Peak-Hour Factor, PHF	<i>0.95</i>	<i>1.00</i>	<i>0.95</i>	<i>1.00</i>	<i>1.00</i>	<i>1.00</i>		
Hourly Flow Rate, HFR (veh/h)	<i>14</i>	<i>0</i>	<i>207</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Heavy Vehicles	<i>1</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Grade (%)	<i>0</i>			<i>0</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration	<i>L</i>		<i>R</i>					
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>					<i>L</i>		<i>R</i>
v (veh/h)	<i>106</i>					<i>14</i>		<i>207</i>
C (m) (veh/h)	<i>805</i>					<i>318</i>		<i>387</i>
v/c	<i>0.13</i>					<i>0.04</i>		<i>0.53</i>
95% queue length	<i>0.45</i>					<i>0.14</i>		<i>3.04</i>
Control Delay (s/veh)	<i>10.1</i>					<i>16.8</i>		<i>24.5</i>
LOS	<i>B</i>					<i>C</i>		<i>C</i>
Approach Delay (s/veh)	--	--				<i>24.0</i>		
Approach LOS	--	--				<i>C</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jaccobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	127	142			729	62		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	133	149	0	0	767	65		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	14		283					
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	14	0	297	0	0	0		
Percent Heavy Vehicles	1	0	1	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	133					14		297
C (m) (veh/h)	805					301		387
v/c	0.17					0.05		0.77
95% queue length	0.59					0.15		6.33
Control Delay (s/veh)	10.4					17.5		39.1
LOS	B					C		E
Approach Delay (s/veh)	--	--				38.1		
Approach LOS	--	--				E		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	D Zimmerman				Intersection			
Agency/Co.	Jacobs				Jurisdiction			
Date Performed	7/21/2015				Analysis Year	2015		
Analysis Time Period	PM Peak							
Project ID <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>					North/South Street: <i>Old Henry Road</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound			Westbound				
Movement	L	T	R	L	T	R		
Volume (veh/h)	116	247	0	0	290	362		
%Thrus Left Lane								
Approach	Northbound			Southbound				
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	0	0	193	0	39		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR				LR	
PHF	0.96		0.96				0.96	
Flow Rate (veh/h)	377		679				241	
% Heavy Vehicles	1		1				1	
No. Lanes	1		1		0		1	
Geometry Group	1		1				1	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.3		0.0				0.8	
Prop. Right-Turns	0.0		0.6				0.2	
Prop. Heavy Vehicle	0.0		0.0				0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7
hadj, computed	0.1		-0.3				0.1	
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20				3.20	
x, initial	0.34		0.60				0.21	
hd, final value (s)	5.83		5.08				6.63	
x, final value	0.61		0.96				0.44	
Move-up time, m (s)	2.0		2.0				2.0	
Service Time, t _s (s)	3.8		3.1				4.6	
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	603		707				491	
Delay (s/veh)	17.56		46.20				14.82	
LOS	C		E				B	
Approach: Delay (s/veh)	17.56		46.20				14.82	
LOS	C		E				B	
Intersection Delay (s/veh)	32.04							
Intersection LOS	D							

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jaccobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 No Build</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	341	544			290	46		
Peak-Hour Factor, PHF	0.96	0.96	1.00	1.00	0.96	0.96		
Hourly Flow Rate, HFR (veh/h)	355	566	0	0	302	47		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	136		290					
Peak-Hour Factor, PHF	0.96	1.00	0.96	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	141	0	302	0	0	0		
Percent Heavy Vehicles	1	0	1	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	
Lane Configuration	L					L	R	
v (veh/h)	355					141	302	
C (m) (veh/h)	1215					157	718	
v/c	0.29					0.90	0.42	
95% queue length	1.22					6.34	2.09	
Control Delay (s/veh)	9.2					104.0	13.6	
LOS	A					F	B	
Approach Delay (s/veh)	--	--					42.4	
Approach LOS	--	--					E	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>PM Peak</i>			<i>2022 Build</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Factory Lane</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	431	544			290	46		
Peak-Hour Factor, PHF	0.96	0.96	1.00	1.00	0.96	0.96		
Hourly Flow Rate, HFR (veh/h)	448	566	0	0	302	47		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0				0	
Lanes	1	1	0	0	1	0		
Configuration	L	T					TR	
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	136		341					
Peak-Hour Factor, PHF	0.96	1.00	0.96	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	141	0	355	0	0	0		
Percent Heavy Vehicles	1	0	1	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	448					141		355
C (m) (veh/h)	1215					116		718
v/c	0.37					1.22		0.49
95% queue length	1.72					9.00		2.77
Control Delay (s/veh)	9.7					223.5		14.8
LOS	A					F		B
Approach Delay (s/veh)	--	--				74.2		
Approach LOS	--	--				F		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	<i>D Zimmerman</i>			Intersection			
Agency/Co.	<i>Jacobs</i>			Jurisdiction			
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2015</i>		
Analysis Time Period	<i>AM Peak</i>						
Project Description <i>Ball Homes Factory Lane</i>							
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>				North/South Street: <i>Old Henry Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	0	148	27	25	613	0	
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	
Hourly Flow Rate, HFR (veh/h)	0	151	27	25	625	0	
Percent Heavy Vehicles	0	--	--	1	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	0	1		0
Configuration	LT		R	LTR			
Upstream Signal		1			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	0	0	3	117	0	20	
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	
Hourly Flow Rate, HFR (veh/h)	0	0	3	119	0	20	
Percent Heavy Vehicles	0	0	0	1	0	0	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LT	LTR		LTR			LTR
v (veh/h)	0	25		139			3
C (m) (veh/h)	966	1405		315			488
v/c	0.00	0.02		0.44			0.01
95% queue length	0.00	0.05		2.16			0.02
Control Delay (s/veh)	8.7	7.6		25.2			12.4
LOS	A	A		D			B
Approach Delay (s/veh)	--	--		25.2			12.4
Approach LOS	--	--		D			B

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	2022			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	8	226	27	25	886	2		
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	8	230	27	25	904	2		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		1			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	2	0	38	117	0	20		
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	2	0	38	119	0	20		
Percent Heavy Vehicles	0	0	0	1	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	8	25	139			40		
C (m) (veh/h)	759	1318	241			332		
v/c	0.01	0.02	0.58			0.12		
95% queue length	0.03	0.06	3.26			0.41		
Control Delay (s/veh)	9.8	7.8	38.5			17.3		
LOS	A	A	E			C		
Approach Delay (s/veh)	--	--	38.5			17.3		
Approach LOS	--	--	E			C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>AM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	8	252	27	25	972	2		
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	8	257	27	25	991	2		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		1			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	2	0	38	117	0	20		
Peak-Hour Factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	2	0	38	119	0	20		
Percent Heavy Vehicles	0	0	0	1	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	8	25	139			40		
C (m) (veh/h)	704	1288	211			296		
v/c	0.01	0.02	0.66			0.14		
95% queue length	0.03	0.06	4.01			0.46		
Control Delay (s/veh)	10.2	7.9	49.9			19.1		
LOS	B	A	E			C		
Approach Delay (s/veh)	--	--	49.9			19.1		
Approach LOS	--	--	E			C		

TWO-WAY STOP CONTROL SUMMARY								
General Information			Site Information					
Analyst	<i>D Zimmerman</i>		Intersection					
Agency/Co.	<i>Jacobs</i>		Jurisdiction					
Date Performed	<i>7/21/2015</i>		Analysis Year		<i>2015</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>			North/South Street: <i>Old Henry Road</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	3	631	82	35	284	1		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	3	678	88	37	305	1		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>	<i>LTR</i>				
Upstream Signal		1			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	0	0	4	61	0	27		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	0	0	4	65	0	29		
Percent Heavy Vehicles	0	0	0	1	0	0		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		<i>LTR</i>			<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LTR</i>	<i>LTR</i>			<i>LTR</i>		
v (veh/h)	3	37	94			4		
C (m) (veh/h)	1266	776	223			739		
v/c	0.00	0.05	0.42			0.01		
95% queue length	0.01	0.15	1.95			0.02		
Control Delay (s/veh)	7.9	9.9	32.4			9.9		
LOS	<i>A</i>	<i>A</i>	<i>D</i>			<i>A</i>		
Approach Delay (s/veh)	--	--	32.4			9.9		
Approach LOS	--	--	<i>D</i>			<i>A</i>		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year				
Analysis Time Period	<i>PM Peak</i>			<i>2022 No Build</i>				
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	37	863	82	35	422	3		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	39	927	88	37	453	3		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0			0		
Lanes	1	1	0	1	1	0		
Configuration	L		TR	L		TR		
Upstream Signal		1			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	1	0	21	61	0	27		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	1	0	22	65	0	29		
Percent Heavy Vehicles	0	0	0	1	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	39	37	94			23		
C (m) (veh/h)	1115	535	182			524		
v/c	0.03	0.07	0.52			0.04		
95% queue length	0.11	0.22	2.59			0.14		
Control Delay (s/veh)	8.3	12.2	44.2			12.2		
LOS	A	B	E			B		
Approach Delay (s/veh)	--	--	44.2			12.2		
Approach LOS	--	--	E			B		

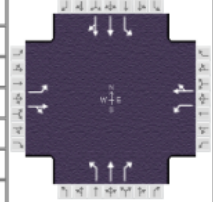
TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	<i>D Zimmerman</i>			Intersection				
Agency/Co.	<i>Jacobs</i>			Jurisdiction				
Date Performed	<i>7/21/2015</i>			Analysis Year	<i>2022 Build</i>			
Analysis Time Period	<i>PM Peak</i>							
Project Description <i>Ball Homes Factory Lane</i>								
East/West Street: <i>Hamilton Springs/Arnold Palmer</i>				North/South Street: <i>Old Henry Road</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	37	953	82	35	473	3		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	39	1024	88	37	508	3		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Two Way Left Turn Lane</i>							
RT Channelized			0				0	
Lanes	1	1	0	1	1		0	
Configuration	L		TR	L			TR	
Upstream Signal		1			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	1	0	21	61	0	27		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	1	0	22	65	0	29		
Percent Heavy Vehicles	0	0	0	1	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	1	0	0	1		0	
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LTR			LTR		
v (veh/h)	39	37	94			23		
C (m) (veh/h)	1065	462	148			467		
v/c	0.04	0.08	0.64			0.05		
95% queue length	0.11	0.26	3.45			0.15		
Control Delay (s/veh)	8.5	13.5	64.2			13.1		
LOS	A	B	F			B		
Approach Delay (s/veh)	--	--	64.2			13.1		
Approach LOS	--	--	F			B		

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	Jacobs			Duration, h	0.25										
Analyst	D Zimmerman			Analysis Date	Jul 21, 2015										
Jurisdiction				Time Period	AM Peak										
Intersection	Bush Farm Road			Analysis Year	2015										
File Name	Old Henry at Bush 15 AM.xus			Analysis Period	1 > 7:30										
Project Description	Ball Homes Factory Lane														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				3	5	72	615	4	37	7	115	138	69	604	1
Signal Information															
Cycle, s	85.0	Reference Phase	2												
Offset, s	0	Reference Point	End	Green	32.8	40.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8		2		6				
Case Number					6.0		6.0		5.0		6.0				
Phase Duration, s					46.6		46.6		38.4		38.4				
Change Period, (Y+R _c), s					6.6		6.6		5.6		5.6				
Max Allow Headway (MAH), s					4.8		4.8		5.0		5.0				
Queue Clearance Time (g _s), s					42.0		42.0		27.8		27.1				
Green Extension Time (g _e), s					0.0		0.0		4.9		5.0				
Phase Call Probability					1.00		1.00		1.00		1.00				
Max Out Probability					1.00		1.00		0.17		0.15				
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				3	78		621	41		7	116	79	70	611	
Adjusted Saturation Flow Rate (s), veh/h/ln				1387	1610		1329	1618		823	1863	1579	1283	1881	
Queue Service Time (g _s), s				0.0	2.3		37.7	1.2		0.7	3.5	2.7	3.2	25.1	
Cycle Queue Clearance Time (g _c), s				40.0	2.3		40.0	1.2		25.8	3.5	2.7	6.7	25.1	
Green Ratio (g/C)				0.47	0.47		0.47	0.47		0.39	0.39	0.39	0.39	0.39	
Capacity (c), veh/h				85	758		675	762		159	719	609	527	725	
Volume-to-Capacity Ratio (X)				0.036	0.103		0.921	0.054		0.045	0.162	0.129	0.132	0.842	
Available Capacity (c _a), veh/h				85	758		675	762		277	986	836	712	996	
Back of Queue (Q), veh/ln (50th percentile)				0.1	0.8		15.1	0.4		0.1	1.4	0.9	0.9	11.0	
Queue Storage Ratio (RQ) (50th percentile)				0.01	0.04		0.76	0.01		0.02	0.04	0.02	0.23	0.28	
Uniform Delay (d ₁), s/veh				42.5	12.5		24.6	12.2		35.5	17.1	16.9	19.3	23.7	
Incremental Delay (d ₂), s/veh				0.2	0.1		18.2	0.0		0.2	0.1	0.1	0.2	5.7	
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh				42.7	12.6		42.7	12.3		35.7	17.2	17.0	19.5	29.5	
Level of Service (LOS)				D	B		D	B		D	B	B	B	C	
Approach Delay, s/veh / LOS				13.7		B	40.8		D	17.8		B	28.4		C
Intersection Delay, s/veh / LOS				31.4						C					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.5		B	2.3		B	2.3		B	2.3		B
Bicycle LOS Score / LOS				0.6		A	1.6		A	0.8		A	1.6		A

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	Jacobs			Duration, h	0.25		
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015	Area Type	Other		
Jurisdiction		Time Period	AM Peak	PHF	0.99		
Intersection	Bush Farm Road	Analysis Year	2022 No Build	Analysis Period	1 > 7:30		
File Name	Old Henry at Bush 22 AM NB.xus						
Project Description	Ball Homes Factory Lane						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	15	10	104	742	37	52	171	174	235	129	804	50

Signal Information																		
Cycle, s	96.6	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	Yes	Simult. Gap E/W	On	Green	44.4	40.0	0.0	0.0	0.0	0.0	1		2		3		4	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	5		6		7		8	
				Red	1.3	3.0	0.0	0.0	0.0	0.0								

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		6.0		6.0		5.0		6.0
Phase Duration, s		46.6		46.6		50.0		50.0
Change Period, (Y+R _c), s		6.6		6.6		5.6		5.6
Max Allow Headway (MAH), s		4.8		4.8		5.3		5.3
Queue Clearance Time (g _s), s		42.0		42.0		42.3		17.7
Green Extension Time (g _e), s		0.0		0.0		2.1		11.7
Phase Call Probability		1.00		1.00		1.00		1.00
Max Out Probability		1.00		1.00		1.00		0.31

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement	7	4	14	3	8	18	5	2	12	1	6	16
Assigned Movement												
Adjusted Flow Rate (v), veh/h	15	115		749	90		173	176	177	130	436	427
Adjusted Saturation Flow Rate (s), veh/h/ln	1328	1616		1285	1702		651	1863	1579	1216	1881	1842
Queue Service Time (g _s), s	0.0	4.3		35.7	3.2		24.5	5.4	6.6	6.9	15.7	15.7
Cycle Queue Clearance Time (g _c), s	40.0	4.3		40.0	3.2		40.3	5.4	6.6	12.4	15.7	15.7
Green Ratio (g/C)	0.41	0.41		0.41	0.41		0.46	0.46	0.46	0.46	0.46	0.46
Capacity (c), veh/h	75	669		549	705		268	856	725	565	864	846
Volume-to-Capacity Ratio (X)	0.203	0.172		1.365	0.127		0.646	0.205	0.244	0.231	0.504	0.504
Available Capacity (c _a), veh/h	75	669		549	705		272	868	736	573	877	858
Back of Queue (Q), veh/ln (50th percentile)	0.4	1.6		39.8	1.2		4.0	2.2	2.2	1.9	6.3	6.2
Queue Storage Ratio (RQ) (50th percentile)	0.05	0.08		2.01	0.03		0.50	0.05	0.06	0.47	0.16	0.16
Uniform Delay (d ₁), s/veh	48.3	17.8		32.9	17.5		32.5	15.6	15.9	19.3	18.4	18.4
Incremental Delay (d ₂), s/veh	1.6	0.1		175.8	0.1		5.9	0.2	0.2	0.3	0.7	0.7
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	49.9	18.0		208.7	17.6		38.4	15.7	16.1	19.6	19.0	19.0
Level of Service (LOS)	D	B		F	B		D	B	B	B	B	B
Approach Delay, s/veh / LOS	21.7		C	188.2		F	23.3		C	19.1		B
Intersection Delay, s/veh / LOS	77.2						E					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.9	C	2.4	B	2.3	B	2.3	B
Bicycle LOS Score / LOS	0.7	A	1.9	A	1.4	A	1.3	A

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	Jacobs			Duration, h	0.25										
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015			Area Type	Other								
Jurisdiction		Time Period	AM Peak			PHF	0.99								
Intersection	Bush Farm Road	Analysis Year	2022 Build			Analysis Period	1 > 7:30								
File Name	Old Henry at Bush 22 AM B.xus														
Project Description	Ball Homes Factory Lane														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	15	10	104	742	37	52	171	200	235	129	890	50			
Signal Information															
Cycle, s	97.2	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	On	Green	45.0	40.0	0.0	0.0	0.0	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0			
				Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0			
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8		2		6				
Case Number					6.0		6.0		5.0		6.0				
Phase Duration, s					46.6		46.6		50.6		50.6				
Change Period, (Y+R _c), s					6.6		6.6		5.6		5.6				
Max Allow Headway (MAH), s					4.8		4.8		5.4		5.4				
Queue Clearance Time (g _s), s					42.0		42.0		47.0		19.8				
Green Extension Time (g _e), s					0.0		0.0		0.0		12.4				
Phase Call Probability					1.00		1.00		1.00		1.00				
Max Out Probability					1.00		1.00		1.00		0.41				
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	15	115		749	90		173	202	177	130	479	470			
Adjusted Saturation Flow Rate (s), veh/h/ln	1328	1616		1285	1702		600	1863	1579	1187	1881	1846			
Queue Service Time (g _s), s	0.0	4.4		35.6	3.2		27.2	6.3	6.6	7.2	17.8	17.8			
Cycle Queue Clearance Time (g _c), s	40.0	4.4		40.0	3.2		45.0	6.3	6.6	13.6	17.8	17.8			
Green Ratio (g/C)	0.41	0.41		0.41	0.41		0.46	0.46	0.46	0.46	0.46	0.46			
Capacity (c), veh/h	74	665		545	700		242	862	731	546	871	854			
Volume-to-Capacity Ratio (X)	0.204	0.173		1.376	0.128		0.715	0.234	0.242	0.239	0.550	0.550			
Available Capacity (c _a), veh/h	74	665		545	700		242	862	731	546	871	854			
Back of Queue (Q), veh/ln (50th percentile)	0.4	1.6		40.3	1.2		4.6	2.5	2.2	1.9	7.3	7.1			
Queue Storage Ratio (RQ) (50th percentile)	0.05	0.08		2.03	0.03		0.57	0.06	0.06	0.48	0.18	0.18			
Uniform Delay (d ₁), s/veh	48.6	18.1		33.2	17.8		35.4	15.7	15.8	19.8	18.8	18.8			
Incremental Delay (d ₂), s/veh	1.6	0.1		180.5	0.1		10.4	0.2	0.2	0.3	1.0	1.0			
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	50.2	18.3		213.7	17.9		45.9	15.9	16.0	20.1	19.8	19.8			
Level of Service (LOS)	D	B		F	B		D	B	B	C	B	B			
Approach Delay, s/veh / LOS	22.0		C	192.7		F	25.3		C	19.8		B			
Intersection Delay, s/veh / LOS	76.9						E								
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.9	C		2.4	B		2.3	B		2.3	B				
Bicycle LOS Score / LOS	0.7	A		1.9	A		1.4	A		1.4	A				

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	Jacobs			Duration, h	0.25										
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015			Area Type	Other								
Jurisdiction		Time Period	PM Peak			PHF	0.93								
Intersection	Bush Farm Road		Analysis Year	2015		Analysis Period	1 > 5:00								
File Name	Old Henry at Bush 15 PM.xus														
Project Description	Ball Homes Factory Lane														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	4	11	38	351	5	65	60	696	552	83	338	9			
Signal Information															
Cycle, s	83.9	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	On	Green	43.6	28.1	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0					
				Red	1.3	3.0	0.0	0.0	0.0	0.0					
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8		2		6				
Case Number					6.0		6.0		5.0		6.0				
Phase Duration, s					34.7		34.7		49.2		49.2				
Change Period, (Y+R _c), s					6.6		6.6		5.6		5.6				
Max Allow Headway (MAH), s					4.7		4.7		5.2		5.2				
Queue Clearance Time (g _s), s					4.9		26.0		29.1		38.8				
Green Extension Time (g _e), s					2.7		2.1		10.3		4.9				
Phase Call Probability					1.00		1.00		1.00		1.00				
Max Out Probability					0.00		0.06		0.68		0.96				
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	4	53		377	75		65	748	529	89	373				
Adjusted Saturation Flow Rate (s), veh/h/ln	1345	1651		1359	1612		1025	1863	1579	717	1872				
Queue Service Time (g _s), s	0.2	1.8		22.1	2.7		3.4	27.1	20.3	9.6	10.0				
Cycle Queue Clearance Time (g _c), s	2.9	1.8		24.0	2.7		13.5	27.1	20.3	36.8	10.0				
Green Ratio (g/C)	0.34	0.34		0.34	0.34		0.52	0.52	0.52	0.52	0.52				
Capacity (c), veh/h	494	554		512	541		495	967	820	226	972				
Volume-to-Capacity Ratio (X)	0.009	0.095		0.737	0.139		0.130	0.774	0.645	0.395	0.384				
Available Capacity (c _a), veh/h	683	786		704	768		512	998	846	238	1004				
Back of Queue (Q), veh/ln (50th percentile)	0.1	0.7		7.2	1.0		0.7	10.6	6.5	1.6	3.7				
Queue Storage Ratio (RQ) (50th percentile)	0.01	0.03		0.36	0.03		0.09	0.27	0.17	0.41	0.09				
Uniform Delay (d ₁), s/veh	20.4	19.1		27.4	19.4		16.2	16.2	14.6	31.0	12.1				
Incremental Delay (d ₂), s/veh	0.0	0.1		3.0	0.1		0.2	4.0	1.9	1.6	0.4				
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0				
Control Delay (d), s/veh	20.5	19.2		30.4	19.6		16.4	20.2	16.5	32.6	12.5				
Level of Service (LOS)	C	B		C	B		B	C	B	C	B				
Approach Delay, s/veh / LOS	19.3		B	28.6		C	18.6		B	16.4		B			
Intersection Delay, s/veh / LOS	20.1						C								
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.5		B	2.3		B	2.2		B	2.2		B			
Bicycle LOS Score / LOS	0.6		A	1.2		A	2.7		B	1.3		A			

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information																							
Agency	Jacobs			Duration, h	0.25																						
Analyst	D Zimmerman			Analysis Date	Jul 21, 2015																						
Jurisdiction				Time Period	PM Peak																						
Intersection	Bush Farm Road			Analysis Year	2022 No Build																						
File Name	Old Henry at Bush 22 PM NB.xus			Analysis Period	1> 5:00																						
Project Description	Ball Homes Factory Lane																										
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				74	40	225	632	12	125	97	832	624	106	460	19												
Signal Information																											
Cycle, s	97.2	Reference Phase	2																								
Offset, s	0	Reference Point	End																								
Uncoordinated	Yes	Simult. Gap E/W	On	Green	45.0	40.0	0.0	0.0	0.0	0.0	0.0																
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0																	
				Red	1.3	3.0	0.0	0.0	0.0	0.0																	
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase							4						8						2						6		
Case Number							6.0						6.0						5.0						6.0		
Phase Duration, s							46.6						46.6						50.6						50.6		
Change Period, (Y+R _c), s							6.6						6.6						5.6						5.6		
Max Allow Headway (MAH), s							5.1						5.1						5.2						5.2		
Queue Clearance Time (g _s), s							14.1						42.0						47.0						47.0		
Green Extension Time (g _e), s							8.5						0.0						0.0						0.0		
Phase Call Probability							1.00						1.00						1.00						1.00		
Max Out Probability							0.16						1.00						1.00						1.00		
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16												
Adjusted Flow Rate (v), veh/h				80	285		680	147		104	895	606	114	259	256												
Adjusted Saturation Flow Rate (s), veh/h/ln				1260	1632		1101	1616		900	1863	1579	626	1881	1855												
Queue Service Time (g _s), s				4.2	12.1		27.9	5.7		7.9	45.0	32.6	0.0	8.3	8.4												
Cycle Queue Clearance Time (g _c), s				10.0	12.1		40.0	5.7		16.3	45.0	32.6	45.0	8.3	8.4												
Green Ratio (g/C)				0.41	0.41		0.41	0.41		0.46	0.46	0.46	0.46	0.46	0.46												
Capacity (c), veh/h				518	672		390	665		413	862	731	74	871	859												
Volume-to-Capacity Ratio (X)				0.154	0.424		1.743	0.222		0.252	1.037	0.830	1.539	0.297	0.298												
Available Capacity (c _a), veh/h				518	672		390	665		413	862	731	74	871	859												
Back of Queue (Q), veh/ln (50th percentile)				1.2	4.5		46.7	2.1		1.6	27.3	12.4	7.9	3.3	3.3												
Queue Storage Ratio (RQ) (50th percentile)				0.16	0.23		2.35	0.05		0.20	0.69	0.32	1.99	0.08	0.08												
Uniform Delay (d ₁), s/veh				21.7	20.4		37.3	18.5		21.3	26.1	22.8	48.6	16.3	16.3												
Incremental Delay (d ₂), s/veh				0.2	0.5		344.7	0.2		0.5	40.7	8.3	298.7	0.3	0.3												
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				21.9	20.9		382.0	18.7		21.8	66.8	31.0	347.3	16.5	16.5												
Level of Service (LOS)				C	C		F	B		C	F	C	F	B	B												
Approach Delay, s/veh / LOS				21.1	C		317.3	F		50.4	D		76.5	E													
Intersection Delay, s/veh / LOS				116.5						F																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				2.9	C		2.4	B		2.3	B		2.3	B													
Bicycle LOS Score / LOS				1.1	A		1.9	A		3.1	C		1.0	A													

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	Jacobs			Duration, h	0.25										
Analyst	D Zimmerman	Analysis Date	Jul 21, 2015			Area Type	Other								
Jurisdiction		Time Period	PM Peak			PHF	0.93								
Intersection	Bush Farm Road		Analysis Year	2022 Build		Analysis Period	1 > 5:00								
File Name	Old Henry at Bush 22 PM B.xus														
Project Description	Ball Homes Factory Lane														
Demand Information				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h	74	40	225	632	12	125	97	922	624	106	511	19			
Signal Information															
Cycle, s	97.2	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	Yes	Simult. Gap E/W	On	Green	45.0	40.0	0.0	0.0	0.0	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.3	3.6	0.0	0.0	0.0	0.0	0.0	0.0			
				Red	1.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0			
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8		2		6				
Case Number					6.0		6.0		5.0		6.0				
Phase Duration, s					46.6		46.6		50.6		50.6				
Change Period, (Y+R _c), s					6.6		6.6		5.6		5.6				
Max Allow Headway (MAH), s					5.1		5.1		5.2		5.2				
Queue Clearance Time (g _s), s					14.1		42.0		47.0		47.0				
Green Extension Time (g _e), s					8.5		0.0		0.0		0.0				
Phase Call Probability					1.00		1.00		1.00		1.00				
Max Out Probability					0.16		1.00		1.00		1.00				
Movement Group Results				EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16			
Adjusted Flow Rate (v), veh/h	80	285		680	147		104	991	606	114	286	283			
Adjusted Saturation Flow Rate (s), veh/h/ln	1260	1632		1101	1616		855	1863	1579	571	1881	1857			
Queue Service Time (g _s), s	4.2	12.1		27.9	5.7		8.6	45.0	32.6	0.0	9.4	9.4			
Cycle Queue Clearance Time (g _c), s	10.0	12.1		40.0	5.7		18.0	45.0	32.6	45.0	9.4	9.4			
Green Ratio (g/C)	0.41	0.41		0.41	0.41		0.46	0.46	0.46	0.46	0.46	0.46			
Capacity (c), veh/h	518	672		390	665		387	862	731	74	871	860			
Volume-to-Capacity Ratio (X)	0.154	0.424		1.743	0.222		0.269	1.150	0.830	1.539	0.329	0.330			
Available Capacity (c _a), veh/h	518	672		390	665		387	862	731	74	871	860			
Back of Queue (Q), veh/ln (50th percentile)	1.2	4.5		46.7	2.1		1.7	36.9	12.4	7.9	3.8	3.7			
Queue Storage Ratio (RQ) (50th percentile)	0.16	0.23		2.35	0.05		0.21	0.94	0.32	1.99	0.09	0.09			
Uniform Delay (d ₁), s/veh	21.7	20.4		37.3	18.5		22.2	26.1	22.8	48.6	16.5	16.5			
Incremental Delay (d ₂), s/veh	0.2	0.5		344.7	0.2		0.5	80.7	8.3	298.7	0.3	0.3			
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay (d), s/veh	21.9	20.9		382.0	18.7		22.8	106.8	31.0	347.3	16.8	16.9			
Level of Service (LOS)	C	C		F	B		C	F	C	F	B	B			
Approach Delay, s/veh / LOS	21.1		C	317.3			F	74.7			E	71.9		E	
Intersection Delay, s/veh / LOS				124.8						F					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.9	C		2.4	B		2.3	B		2.3	B				
Bicycle LOS Score / LOS	1.1	A		1.9	A		3.3	C		1.1	A				

Tab 10

Statutory/Case Law regarding contents
of subdivision regulations and review
of subdivisions

KRS § 100.281 -

Contents of subdivision regulations

- Subdivision regulations shall be based on the comprehensive plan, in those counties which have adopted a comprehensive plan, and all subdivision regulations shall contain:

(1) The procedure for the submission and approval of preliminary and final plat and the recordation of final plats. The commission may delegate to its secretary or any other officer or employee the power to approve plats in accordance with the commission's adopted requirements, but all plats, preliminary and final, shall be approved or disapproved within ninety (90) days;

- “note that an administrative agency's interpretation of its own regulations is entitled to substantial deference”. Commonwealth of Kentucky, Cabinet for Health Services v. Family Home Health Care, Inc., 98 S.W.3d 524 (Ky. App. 2003) citing Camera Center, Inc. v. Revenue Cabinet, Ky., 34 S.W.3d 39 (2000). A reviewing court is not free to substitute its judgment as to the proper interpretation of the agency's regulations as long as that interpretation is compatible and consistent with the statute under which it was promulgated and is not otherwise defective as arbitrary or capricious.

Directory v. Advisory provisions of regulations (meaning specific standards):

- Subdivision regulations shall contain rules and regulations that constitute specific standards to be applied. Snyder v. Owensboro, 528, S.W.2d 663, 664 (Ky. 1975).

Directory v. Advisory provisions of regulations (meaning specific standards):

- As respects subdivision applications “there must be rules and regulations constituting specific standards to be applied in determining whether approval is to be granted.” Wolf Pen Preservation Ass'n, Inc. v. Louisville & Jefferson County Planning Com'n, Canfield-Knopf Properties, Inc., 942 S.W.2d 310, 312 (Ky. App. 1997).

Directory v. Advisory provisions of regulations (meaning specific standards):

- The legislative scheme must be essentially complete on its face, leaving to regulatory authority administrative rather than policy decisions. Louisville and Jefferson County Planning Commission v. Schmidt, 83 S.W.3d. 449 (Ky. 2001) (citing Diemer v. Com. Transp. Cabinet, 786 S.W.2d 861, 865 (1990), citing Holsclaw v. Stephens, Ky., 507 S.W.2d 462, 471 (1974).

Tab 11

Written justification, in addition to Conservation Plan and exhibits, demonstrating compliance with LDC Section 7.11.4.B.9 requirement

- a. Describe how the existing natural features of the site are being preserved and incorporated into the layout.

The existing natural features on the site are being preserved and incorporated into the subdivision layout. There is a 200 ft perennial stream buffer area between the townhomes and the single family lots. The exiting tree mass on the west side of the property is being preserved along with preservation along the property perimeter on the north and east sides, which is approximately 42.69 acres of open space. The open space to the rear of the proposed subdivision also abuts a wooded portion of the adjoining single family neighborhood, adding to the aesthetics of the area and thus protecting neighbor properties that might involve larger subdivision lots. Over 28.5 acres of tree canopy will be preserved and most intermittent and ephemeral streams channels on site will be maintained.

This application for a conservation subdivision is located on 122 acres off Factory Lane between La Grange Road and Old Henry Road, nestled between the Forest Springs and Woodmont Subdivisions. Townhomes have been clustered towards the front of the development along Factory Lane where over 2.5 acres separate the development from Factory Lane. These will allow for a mix of residential home types in the area which already includes apartments, patio homes and single family homes.

- b. Explain how clustering of dwelling units will:

- i. Minimize disturbance of woodlands, wetlands, grasslands, mature trees and steep slopes

Lots have been located to preserve existing stream corridors as much as possible. Stream corridors overlap many of the wooded areas, steep slopes and wetlands on site. This is demonstrated on the accompanying “Existing Resources & Site Analysis Plan” and as described above.

- ii. Prevent downstream impacts due to runoff through storm water techniques including minimizing impermeable areas, using bio swales, rain gardens, permeable pavements, small-scale, infiltration and green roofs.

Conceptual storm-water strategies include a combination of bio-swales, a green dry basin and water quality units for the treatment of storm-water runoff. Final designs will be determined at the time of construction plan approval and will meet all MSD requirements.

- iii. Protect views of open land from existing adjacent roadways through practices such as orienting structures to align with topographic character of land, tucking structures behind tree lines or knolls, using vegetation as a backdrop to reduce prominence of the structures, varying setbacks, setting aside required conservation land as a visual amenity into and within the development site, or any combination of these practices.

The frontage of the property includes an open space lot with a supplemental setback of some 30’ from the Factory Lane right-of-way. Backs of homes will not face Factory Lane, and the building setback will vary across this frontage.

Large open spaces flank the entrance and will allow for a landscaped signature entrance. This is demonstrated on the accompanying “Existing Resources & Site Analysis Plan” in combination with the submitted Conservation Subdivision Plan and as described above.

- iv. Protect archaeological site and existing historic buildings or incorporate them through adaptive reuse.

No known archaeological features or historic structures exist on this site.

- v. Avoid encroaching on sensitive areas such as rare plant communities, high quality habitats, or endangered species habitats identified by the Kentucky Department for Natural Resources.

The delineation summary report prepared by Redwing Environmental Services on November 20, 2014 indicates approximately 4 acres of suitable habitat for the federally endangered Indiana Bat. Over 28.5 acres of tree canopy is being preserved on site. No other endangered species or rare plants have been identified. This is demonstrated on the accompanying “Existing Resources & Site Analysis Plan” in combination with the submitted Conservation Subdivision Plan and as described above.

- c. Explain how the design and location of buildable lots will ensure compatibility with existing adjacent development.

This is demonstrated in the PowerPoint presentation presented at the required neighborhood meeting which accompanied the application, as supplemented at this public hearing.

- d. Justification must be provided for any cases where proposed open space areas within the development will not abut existing open space areas on adjoining parcels.

To the extent that open space exists on adjoining properties, open spaces shown on the submitted Conservation Subdivision Plan will connect.