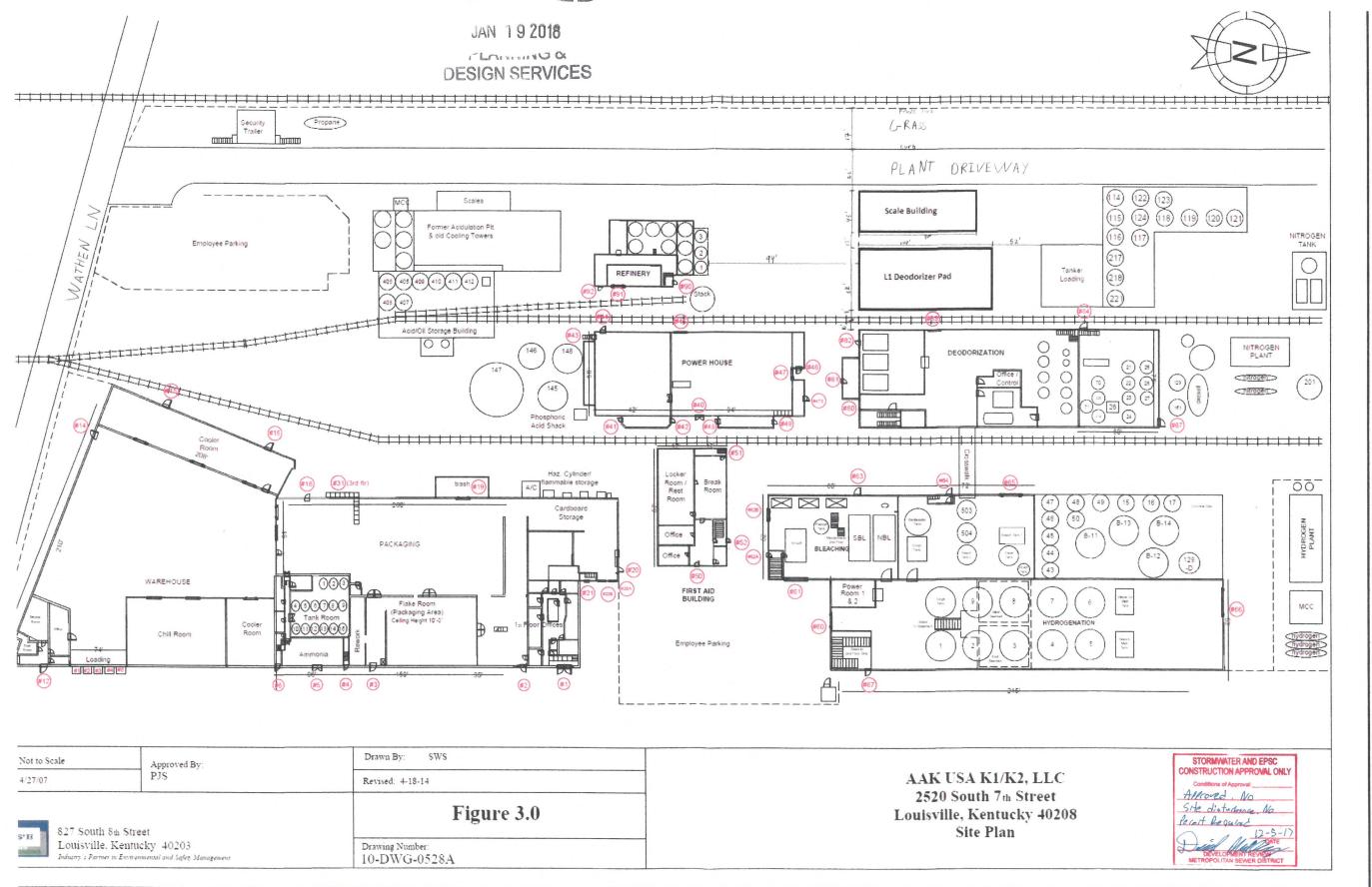
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GENERAL

- PERFORM WORK SHOWN ON DRAWINGS IN ACCORDANCE WITH UGS (UNITED GROUP SERVICES, INC.) PACKAGE AND ALL REFERENCED SPECIFICATIONS
- REFERENCED CODES, SPECIFICATIONS, AND STANDARDS REFER TO THE LATEST EDITIONS, UNLESS NOTED OTHERWISE
- VERIFY EXISTING CONDITIONS AND DIMENSIONS IN THE FIFLD PRIOR TO INSTALL-ATION OF ANY MATERIALS. NOTIFY OWNER IMMEDIATELY WHERE EXISTING CON-DITIONS ARE SUBSTANTIALLY DIFFERENT FROM THOSE SHOWN ON THE DRAWING
- DIMENSIONS SHALL NOT BE SCALED OFF DRAWINGS.
- ALL PROPRIETARY MATERIALS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN RECOMMENDATIONS.
- IF CONFLICTS EXIST BETWEEN THE SPECIFICATIONS AND GENERAL NOTES, THE
- CONTRACTOR IS RESPONSIBLE FOR ANY PILING REQUIRED TO KEEP RAILROAD TRACKS FROM SLIDING INTO EXCAVATION.
- ANY DRAIN PIPE ENCOUNTERED DURING EXCAVATION & INSTALLATION IS CONSIDERED TO BE ABANDONED IN PLACE.
- ALL PILE CAPS HAVE GREEN CONDUIT NEXT TO PIER FOR GROUNDING, EXCEPT PILE CAP TYPE 3 & PILE CAP TYPE 4.
- PAINT EQUIPMENT NUMBERS ON CO-INCIDING EQUIPMENT PAD (ALFA LAVAL & AAK BOTH)

SITE PREPARATION & EARTHWORK

- SITE PREPARATION AND EARTHWORK SHALL BE IN ACCORDANCE WITH TERRACON SOILS REPORT AND ASSOCIATED SPECIFICATIONS.
- EXCAVATION SHALL CONFORM TO OSHA EXCAVATION REGULATIONS 29-CFR PART
- VERIFY LOCATIONS. OF ALL "IN SERVICE" UNDERGROUND LINES SHOWN ON DRAWINGS PRIOR TO STARTING WORK.
- OPEN EXCAVATIONS SHALL BE PROTECTED FROM DISTURBANCE AND RAIN. SURFACE RUNOFF WATER SHALL BE DRAINED AWAY FROM ALL EXCAVATIONS AND
- EXCESS EXCAVATED MATERIAL AND/OR UNUSED BACKFILL MATERIALS SHALL BE REMOVED AND HAULED TO AN AREA DESIGNATED BY THE OWNER WITHIN THE
- SOIL SUBGRADE SHALL BE COMPACTED TO 95% OF DRY DENSITY IN ACCORDANCE
- AGGREGATE BASE MATERIAL SHALL BE DGA COMPACTED TO 100% OF DRY DENSITY IN ACCORDANCE WITH THE STANDARD PROCTOR TEST ASTM D698. MAXIMUM LIFTS
- ALL DRAIN PIPING SHOWN ON THE DRAWING SHALL BE LINE CLASS K1UG SCH. 40 CARBON STEEL PIPE. A106 GR B.
- NEW UNDERGROUND PIPING, GROUNDING AND ELECTRICAL CONDUIT SHALL BE INSTALLED AFTER PILE INSTALLATION AND PRIOR TO CONCRETE PLACEMENT.
- UPON COMPLETION OF WORK, RETURN SITE ROADS AND GRADES TO ORIGINAL CONDITIONS OR AS SHOWN ON DRAWINGS.

ANCHOR BOLTS

- ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 36.
- NUTS SHALL BE HEAVY HEX IN ACCORDANCE WITH ASTM A563 GRADE A. WASHERS SHALL BE HARDENED STEEL IN ACCORDANCE WITH ASTM F436, TYPE 1. ACHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM
- THREADS SHALL BE UNC AMERICAN STANDARD COURSE THREAD SERIES CLASS 2A FIT FOR EXTERNAL AND CLASS 2B FOR INTERNAL. OVERSIZE TAPPING OF NUTS SHALL BE PERFORMED AFTER GALVANIZING, IN ACCORDANCE WITH ASTM A385.
- ANCHOR BOLTS SHALL BE SUBMITTED COMPLETE WITH NUTS AND WASHERS, AS SHOWN. EACH COMPLETED ANCHOR BOLT ASSEMBLY SHALL BE MARKED OR TAGGED
- ASSEMBLIES MARKED SAME SHALL BE PACKED TOGETHER. CLEARLY MARK ALL PACKAGES WITH THEIR CONTENTS. (e.g. ANCHOR BOLTS - 12 PIECES, R6530 - AB -11.)

PILES

AUGER CAST CONCRETE PILES:

- CONTRACTOR SHALL NOTIFY OWNER, PRIOR TO START OF PILE INSTALLATION.
- CONTRACTOR SHALL REVIEW GEOTECHNICAL REPORTS PERFORMED ON THE SITE BY TERRACON 13050 EASTGATE PARKWAY, SUITE 101 LOUISVILLE, KY 40223 (502) 365-9734. CONTRACTOR IS RESPONSIBLE FOR BEING FULLY KNOWLEDGEABLE OF CONDITIONS SHOWN ON MOST UP TO DATE DOCUMENTS.
- PILING CONTRACTOR SHALL BE AWARE OF EXISTING PILES AND UNDERGROUND OBSTRUCTIONS IN THE AREA
- PILES SHALL BE CAREFULLY LOCATED AND INSTALLED TO THE POSITION AS INDICATED ON THE DRAWINGS.
- THE MAXIMUM DEVIATION FROM PLAN LOCATION SHALL BE 2 INCHES. THE MAXIMUM DEVIATION FROM PLUMB SHALL BE 1/4" PER FOOT
- ALL PILES SHALL BE INSTALLED TO DEPTH INDICATED ON DRAWINGS.
- GROUT USED IN AUGER CAST PILES SHALL ATTAIN A COMPRESSIVE STRENGTH OF 4000 P.S.I. AT 28 DAYS.
- PUMPING PRESSURES FOR GROUT SHALL BE MEASURED AND SHALL BE MAIN-TAINED HIGH ENOUGH AT ALL TIMES TO OFFSET HYDROSTATIC AND LATERAL EARTH PRESSURE. THE GROUT PUMP SHOULD BE PROVIDED WITH A PRESSURE GAUGE IN CLEAR VIEW OF THE EQUIPMENT OPERATOR AND THE ENGINEER OR INSPECTOR.
- THE GROUT PUMP SHOULD BE CALIBRATED AT THE BEGINNING OF THE WORK TO DETERMINE THE VOLUME OF GROUT PUMPED PER STROKE. A POSITIVE METHOD OF COUNTING GROUT PUMP STROKE SHOULD BE PROVIDED BY THE PILE CONTRACTOR
- ALL OIL OR OTHER RUST INHIBITORS SHOULD BE REMOVED FROM MIXING DRUMES AND PUMPS PRIOR TO USING. ALL MATERIALS SHOULD BE SUCH AS TO PROPE AN HOMOGENOUS GROUT OF THE DESIRED CONSISTENCY. USE OF A FLOW IS RECOMMENDED FOR CONTROLLING GROUT CONSISTENCY.
- GROUT VOLUMES SHOULD BE MEASURED TO ENSURE THAT THE VOLUME OF GROUT PLACED IN EACH PILE IS EQUAL TO OR GREATER THAN THE THEORETICAL VOLUME OF THE HOLE CREATED BY THE AUGER, OTHERWISE, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY. GROUT USED IN PILE RESULTS SHOULD BE SUBMITTED TO ENGINEER FOR REVIEW.
- THE AUGER FLIGHTING SHOULD BE UNIFORM IN DIAMETER THROUGHOUT ITS LENGTH AND SHOULD BE THE DIAMETER SPECIFIED FOR THE PILES WITHIN 3 PERCENT. THE DISCHARGE HOLE FOR THE GROUT SHOULD BE LOCATED AT THE BOTTOM OF THE AUGER BIT, BELOW THE CUTTING TEETH. AUGERS OVER 40 FEET IN LENGTH SHOULD CONTAIN A MIDDLE GUIDER.
- THE AUGER LEADS SHOULD BE PREVENTED FROM ROTATING BY A STABILIZING ARM OR BY OTHER ACCEPTABLE MEANS. LEADS SHOULD BE MARKED AT 1 FOOT INTERVALS TO FACILITATE MEASUREMENT OF AUGER PENETRATION AND RATES OF
- AUGER WITHDRAWAL RATES FOR AUGERED PRESSURE GROUTED PILES SHOULD BE CORRELATED WITH PUMPING RATES, VOLUMES, SHAFT DIAMETERS, AND SITE SPECIFIC DETAILS TO ASSURE THAT THE PROPER QUANTITY OF GROUT IS INSTALLED IN THE PILE SHAFT
- THE AUGER SHOULD NOT BE ALLOWED TO JUMP, JERK, OR RAPIDLY ACCELERATE
- AUGERED CAST-IN-PLACE PILES SHOULD NOT BE INSTALLED WITHIN 6 PILE DIAMETERS, CENTER TO CENTER, OF A PILE FILLED WITH CONCRETE LESS THAN 12 HOURS OLD. IF THE CONCRETE LEVEL IN ANY COMPLETE PILE DROPS DURING INSTALLATION OF AN ADJACENT PILE, THE PILE SHOULD BE REPLACED.
- PILE LOAD TEST IS REQUIRED.

CAST-IN-PLACE CONCRETE

- CAST—IN—PLACE CONCRETE WORK SHALL CONFORM TO THE LATEST ACL CODE AND STANDARDS. ACI 301—89 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" IS HEREBY MADE A PART OF THESE DRAWINGS. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301—89, EXCEPT AS MODIFIED HEREIN.
- ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE IN 28 DAYS SHALL BE: 4000 PSI - ALL CONCRETE
- REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60 DEFORMED BARS.
- WELDED WIRE FARRIC: ASTM A185
- USE AN AIR-ENTRAINING ADMIXTURE CONTAINING VINSOL RESIN IN ALL CONCRETE EXPOSED TO THE WEATHER OR IN A LOCATION VULNERABLE TO DEICERS. THE AMOUNT OF ENTRAINED AIR SHALL BE $6\%\pm1\%$.
- PROVIDE CORROSION RESISTANT ACCESSORIES SUCH AS GRAY PLASTIC CHAIRS OR CHAIRS WITH GRAY PLASTIC COATED TIPS, IN ALL EXPOSED CONCRETE CONSTRUCTION. PRECAST CONCRETE CUBES (MASONRY BRICK NOT ALLOWED) OR SAND PLATE CHAIRS SHALL BE USED FOR THE SUPPORT OF REINFORCING WAS AND PLATE CHAIRS SHALL BE USED FOR THE SUPPORT OF REINFORCING WAS AND PLATE CHAIRS SHALL BE USED FOR THE SUPPORT OF REINFORCING WAS AND PLATE.
- REINFORCING BARS REQUIRED FOR PROPER SUPPORT OF PRINCIPAL REINFORCING SHALL BE DETAILED AND SUPPLIED BY THE CONTRACTOR WHETHER OR NOT THEY ARE INDICATED ON THE DRAWINGS. THE MINIMUM BAR SIZE SHALL BE #4 AND MAXIMUM SPACING SHALL BE 36" ON CENTER FOR ALL BARS THAT NEED SUPPORT. WELDED WIRE FABRIC SHALL NOT BE USED FOR THE SUPPORT OF PRINCIPAL REINFORCING.
- ALL CONCRETE SHALL BE CURED USING A LIQUID MEMBRANE CURING COMPOUND WITH A MAXIMUM UNIT MOISTURE LOSS OF 0.039 GR./SQ.CM @ 72 HOURS AND APPLIED AT A MAXIMUM COVERAGE RATE OF 200 SQ.FT/GAL. CURING COMPOUND SHALL BE APPLIED WITHIN 1 HOUR AFTER FINAL TROWELING OR FORM REMOVAL. ALL CONCRETE SHALL BE CURED FOR NOT LESS THAN 7 DAYS.
- CHAMFER EXPOSED EDGES OF CONCRETE 1", UNLESS NOTED OTHERWISE.
- PROVIDE CONTROL JOINTS IN ALL SLABS ON GRADE AS SHOWN ON THE DRAWINGS.
- PROVIDE 1/2" PREMOLDED JOINT BETWEEN PIERS AND CONCRETE SLAB.

12. DETERMINE SIZE AND LOCATION OF MECHANICAL EQUIPMENT, AND MAKE PROVISIONS FOR BOLITS, SIFFURS, PADS, FIRST, IN AGCORDANCE WITH THE LACE AND LACE

SONGRETE MIX DESIGN: TO BE SUMMITTED COLORAGE BY CONCRETE CONTRACTOR OF KEN CONSTRUCTION SHOULD BE TESTED IN ACCORDANCE WITH ASTM STANDARDS. TEATBETOVER AS NOTED, based on information shown. This approval is subjectly as sometiments of the subject inspaction by the Fire Department.

COUNTATION AZARNOOSH 5 ASHRAFZADEH

Fire Hydrani(s) required as notations

L1-C-0008 - PILE CAP SECTIONS & DETAILS L1-C-0007 - PILE CAP SECTIONS & DETAILS L1-C-0008 - DEODORIZER COLUMN ANCHOR BOLT LAYOUT Lewisville Ordinance

FDC required as noted on plans. Size / Type;

The above approvals shall meet the outsent adop

Ordinances Date reviewed: NOVEMBESE

ISSUE FOR CONSTRUCTION

ISSUED FOR CONSTRUCTION

 KAK
 07/13/17
 SEC
 AA
 JB

 DRAWN BY
 DATE REVISED
 CHECKED BY
 PROJECT ENG.
 APPROVED BY

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AAK

DATE ISSUED 07/13/17

LOUISVILLE, KY 40208

29306 CENSED

ONAL ENGIN

PROJECT 21305 DEODORIZER

NOTES SHT-1 OF 2 LOUISVILLE DRAWN BY K.KEATON SCALE: AS NOTED CHECKED BY S.CLAY PROJECT ENG. A.ASHRAFZADEH L1-C-0001 APPROVED BY J. BECK

17 DEVPLAN 1225

0

STRUCTURAL STEEL GENERAL NOTES A. CONTRACTOR SHALL VERIFY ALL ELEVATION AND DIMENSIONS AND INTERFERENCES PRIOR TO COMMENCING FABRICATION, REPORT ANY DISCREPANCIES TO OWNERS ENGINEER. IN THE EVENT OF CONFLICT BETWEEN THE DRAWINGS AND THE OWNERS ENGINEERING STANDARDS. THE ENGINEER SHALL BE CONSULTED. 1.1 FNGINFFRING STANDARDS CODES & SPECIFICATION: DIVISION 05-METALS-SECTION 05010 STRUCTURAL STEEL DIVISION 05-METALS-SECTION 05500 METAL FABRICATION DIVISION 07-THERMAL & MOISTURE PROTECTION SECTION 07812 SPRAYED FIRE RESISTIVE MATERIALS. DIVISION 07-THERMAL & MOISTURE PROTECTION SECTION 07841 THROUGH PENETRATION FIRESTOP SYSTEM. KENTUCKY BUILDING CODE 2013 4TH ED. JUNE 22, 2016. IBC 2012 ASCE 7-10. CODES & SPECIFICATIONS: A. STRUCTURAL STEEL SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE CODES AND SPECIFICATIONS LISTED BELOW. 1. "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" ANSI/AISC 360 "STEEL CONSTRUCTION MANUAL" 13TH EDITION 2. CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AISC 303 2010 3. "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" RCSC 2004 B. ALL WORK SHALL CONFORM TO THE IBC-2012 C. 2013 KENTUCKY BUILDING CODE 3RD EDDITION AUGUST 2014 CONNECTIONS: A. ALL BEAM CONNECTIONS TO BE IN ACCORDANCE WITH TABLE 10-1 - 10-3 FROM PART 10 OF THE A.I.S.C. STEEL CONSTRUCTION MANUAL, 360-ASD. CONNECTIONS TO BE SIZED FOR THE REACTIONS POSTED ON THE DRAWINGS. IF NO REACTION IS POSTED, USE A.I.S.C. MANUAL PART 3, "DESIGN OF FLEXURAL MEMBERS". BEAM CONNECTIONS TO BE SIZED FOR THE LESSER OF: HALF THE TOTAL ALLOWABLE UNIFORM LOAD FOR THE APPLICABLE BEAM LENGTH OR "V" (SEE NOTE 6.D.). MINIMUM 2 BOLTS. B. SHOP CONNECTIONS TO BE FABRICATED WITH A SHOP WELDED & FIELD BOLTED APPROACHED UNLESS NOTED ON DESIGN DRAWINGS. C. FIELD CONNECTIONS TO BE BOLTED UNLESS NOTED. D. ALL POSTED LOADS ARE THE LOADS TO BE USED FOR CONNECTION. E. ALL MOMENT CONNECTIONS (NOTED M.C. OR MOM. CONN. ON PLANS) TO BE DESIGNED FOR POSTED REACTION. WHERE NO MOMENT IS POSTED, CONNECTION TO BE DESIGNED FOR THE FULL CAPACITY OF THE BEAM. (MIN. 6 BOLTS)

GRATING EQUIPMENT CONCRETE SLAB B. LIVE LOADS C. GROUND SNOW LOAD D. WIND LOAD E. RUSIMONE

THEY ARE NOT TO BE FURTHER FACTORED. F. ALL COLUMN BASE AND CAP PLATES TO BE 1/2" MINIMUM THICKNESS WITH (4) A325 BOLTS UNLESS NOTED OTHERWISE. G. ALL COLUMN SPLICES TO BE DESIGNED TO DEVELOP 50 PERCENT OF THE NOME 50 PERCENT OF THE SHEAR CAPACITIES OF THE COLUMN UNLESS SHOWN OTHERWISE WHERE TWO DIFFERENT COLUMNS ARE SPLICED, USE THE SMALLER COLUMN FOR CONNECTION DESIGN. THE MINIMUM COLUMN SPLICE CONNECTION TO BE 2 ROWS OF 2 BOLTS EACH SIDE OF SPLICE CENTERLINE. BOLTS TO BE IN DOUBLE SHEAR WIT SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3903 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3903 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE ENDS OF THE BOLTED COLUMNS \$ \$3000 811 / BUIDING BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THICK THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES A MINIMUM OF 3/8" THE BOLDED WELL SPLICE PLATES

BE FINISHED AND IN BEARING. H. ALL BRACING MEMBERS WITHOUT A LOAD DESIGNATED ARE TO HAVE A MINIMUM 2 BOLT CONNECTION, FOR WT'S MIN. 2 BOLTS WHEN CONNECTING TO WEB & 4 BOLTS WHEN CONNECTION TO FLG.

I. ALL GUSSET PLATES TO BE 3/8" THICK MINIMUM UNLESS NOTED OTHERWISE.

ASTM-A325

ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS INDICATED BELOW TEGN ASZINIE ALL W SHAPES, (U.N.O.) ASTM A992

BASE PLATES, CONNECTIONS PLATES, ANGLES, ASTM A-36 CHANNELS AND MISCELLANEOUS. HSS SHAPES ASTM500 GR B

HS BOITS

STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED BY AN APPROVED AND LICENSED FABRICATOR IN ACCORDANCE WITH THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS LATEST EDITION.

STRUCTURAL STEEL GENERAL NOTES CONT.

THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS TO THE ENGINEER OF ALL STEEL FOR STRUCTURAL ENGINEER'S REVIEW BEFORE FABRICATION.

NO OPENINGS SHALL BE CUT IN STRUCTURAL STEEL MEMBERS UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.

HOLES IN STEEL SHALL BE 1/16" LARGER DIAMETER THAN NOMINAL SIZE OF BOLT USED,

ALL STRUCTURAL STEEL SURFACES THAT ARE ENCASED IN CONCRETE MASONRY OR SPRAY ON FIREPROOFING, OR ARE ENCASED BY BUILDING FINISH, SHALL BE LEFT UNPAINTED.

A. ALL WELDING SHALL BE IN ACCORDANCE WITH A.W.S. D1.1, LATEST EDITION B. E70XX ELECTRODES SHALL BE LOW HYDROGEN AND EQUIVALENT TO BASE MATERIAL.

ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS USING E70XX ELECTRODES (U.N.O.). ALL WELDS SHALL BE IN CONFORMITY WITH THE PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D1.1 LATEST REVISION) OF THE AMERICAN WEIDING SOCIETY

WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.

12. PAINTING:

A. STRUCTURAL STEEL TO BE PRIMERED PRIOR TO ARRIVAL TO JOB SITE.

B. OWNER RESPONSIBLE FOR ANY SECONDARY PAINTING.

ALL GRATING PER DIVISION 05-METALS-SECTION 05500 METAL FABRICATION.

ALL HANDRAIL PER DIVISION 05-METAILS-SECTION 05521 PIPE & TUBE RAILINGS

ALL STAIRS PER DIVISION 05-METALS-SECTION 05511 METAL STAIRS.

ALL LADDERS PER DIVISION 05-METALS-SECTION 05515 STEEL LADDERS

DESIGN CRITERIA:

A DEAD LOADS: 30 PSF PER VENDOR 75 PSF 75 PSF 15 PSF 115 MPH (ULTIMATE)

Late reviewed Water S.

The above approvals shall have

FBG required as noted on plans.

LININ

Jefferson County Suburban Fire Service



6'-7" SECTIONS

STRUCTURAL STEEL FABRICATION AND DETAILING NOTES

(UNITED GROUP SERVICES, INC) DRAWING REVIEW IS FOR GENERAL COMPLIANCE WITH CONTRACT DOCUMENTS NO RESPONSIBILITY IS ASSUMED FOR CORRECTNESS OF DIMENSIONS, DETAILS OR DETAILS DESIGN

SHOP FABRICATION DETAILS ARE TO BE MADE FROM ISSUED FOR CONSTRUCTION DESIGN DRAWINGS ONLY.

FABRICATOR IS RESPONSIBLE FOR THE FOLLOWING:

A. CHECKED SHOP DETAILS.

B. CONNECTIONS NOT SHOWN ON THE DESIGN DWG.

C. PROVIDING FINISHED SHIPPING WEIGHTS (EITHER UNDER THE PIECE OR IN THE BILL/MATERIAL

D. FIELD BOLT LIST FOR STRUCTURAL STEEL TO STRUCTURAL STEEL FIELD CONNECTIONS, INCLUDING BOLT SUMMARY

E. CHECKING BEAM WEBS WITH DEEP COPE CUTS FOR WEB SHEAR, AND PROVIDING NECESSARY REINFORCEMENT, IF REQUIRED.

F. PAINTING TO BE CONSISTENT WITH FABRICATION SPECIFICATION, WHEN REQUIRED. FIELD PAINTING TO BE MINIMIZED WHILE NOT AFFECTING DESIGN OF BOLTED CONNECTIONS. (SEE GALVANIZING)

G FABRICATOR IS RESPONSIBLE TO COORDINATE ANY AND ALL OF THE DETAILING SUB-CONTRACTOR'S APPROVAL PACKAGE SUBMITTALS.

AREAS MARKED "HOLD" ARE NOT TO BE DETAILED.

FABRICATOR TO SUBMIT ERECTION DRAWINGS CONTAINING ALL SUBMITTED DETAILED MEMBER PIECE MARKS WHEN REQUESTING APPROVAL OF DETAILS.

ALL SHOP DETAILS ARE TO BE CHECKED PRIOR TO SUBMITTAL FOR APPROVAL. DRAWINGS MUST BE MARKED AS CHECKED WITH THE CHECKERS INITIALS OR NAME. UNCHECKED DRAWINGS WILL BE RETURNED UN-REVIEWED & REJECTED.

WHEN UGS DESIGN DRAWINGS ARE USED AS ERECTION DRAWINGS THE ERECTION E-NUMBER IS TO BE SUPERSEDED BY THE DESIGN DRAWING NUMBER

ALL BRACING, BOTH HORIZONTAL AND VERTICAL, THAT INTERSECTS BEAM/BEAM OR BEAM/COLUMN, SHALL BE CONNECTED TO BOTH UNLESS NOTED

FABRICATOR IS TO PROVIDE ALL FIELD BOLTS INCLUDING NUTS AND WASHER (FLAT, BEVELED OR CLIPPED).

ALL ORIGINAL SHOP FABRICATION DETAIL DRAWINGS. BILLS OF MATERIAL FIELD BOLT LIST (IF SEPARATE) AND ALL ERECTION DRAWINGS WILL BE SURRENDERED TO AAK.

DETAIL SHEETS SHALL HAVE A TITLE BLOCK INDICATING PROJECT NAME, FABRICATOR OR DETAILERS NAME, LOCATION, DRAWN BY, CHECK BY & DRAWING OR SHEET NO. ALSO INDICATE ERECTION DRAWING REFERENCE

ISSUED FOR CONSTRUCTION

OF KEN

AZARNOOSH

ASHRAFZADEH:

29306

SONAL ENG

11/9/1-

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APPROVED BY DATE ISSUED 09/28/17

AAK, USA, LV 2520 7th STREET RD. LOUISVILLE, KY 40208

PROJECT 21305 L1 BUILDING STRUCTURAL NOTES

LOUISVILLE DRAWN BY A.TUFTS SCALE: AS NOTED CHECKED BY S.CLAY PROJECT ENG. A.ASHRAFZADEH L1-S-0002 APPROVED BY A RAMEY