REPORT

Fern Valley Commerce Center 4500 Fern Valley Road Louisville, KY

Traffic Impact Study

Louisville Metro Planning Kentucky Transportation Cabinet

February 9, 2016



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Introduction

Summit Construction is proposing a warehouse on Fern Valley Road (KY 1747) in Louisville, KY. The building is proposed as 332,500 square foot warehouse. **Figure 1** displays a map of the site. Access to the site will be from an entrance on Fern Valley Road and on Jefferson Boulevard. The purpose of this study is to examine the traffic impacts of the proposed development upon the adjacent highway system. For this study the impact area was defined to be the intersections of Fern Valley Road and Jefferson Boulevard.

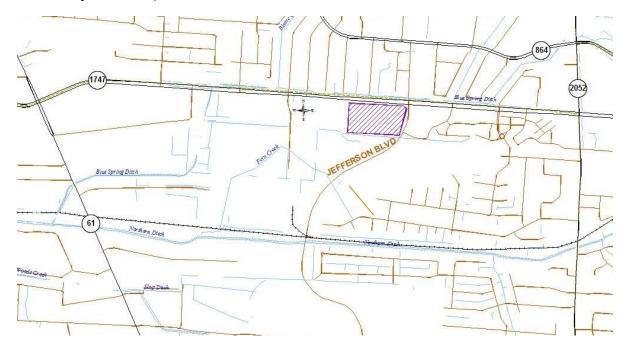


Figure 1
Site Location

Existing Conditions

Fern Valley Road, KY 1747, is maintained by the Kentucky Transportation Cabinet (KYTC) with an estimated 2016 ADT of 32,400 vehicles per day between I 65 and Preston Highway (KY 61), as estimated from the Kentucky Transportation Cabinet count at station 606. The road is a six-lane road with twelve-foot lanes and curbs. The posted speed limit is 45 mph. There is a sidewalk on the north side. The intersection with Jefferson Boulevard is controlled with a traffic signal. All four approaches have dual left turn lanes. Three of the approaches have right turn lanes; westbound Fern Valley does not.

Jefferson Boulevard is maintained by Metro Louisville with an estimated 2016 ADT of 11,000 vehicles per day, as estimated from the turning movement count. The road is a four-lane road with twelve-foot lanes, curb and gutter. The posted speed limit is 35 mph. There are sidewalks on both sides.



A.m. and p.m. peak hour traffic counts were obtained at the intersection on January 7, 2016. The a.m. peak hour occurred between 7:15 and 8:15 and the p.m. peak hour occurred between 4:45 and 5:45 p.m. **Figure 2** illustrates the existing peak hour traffic volumes.

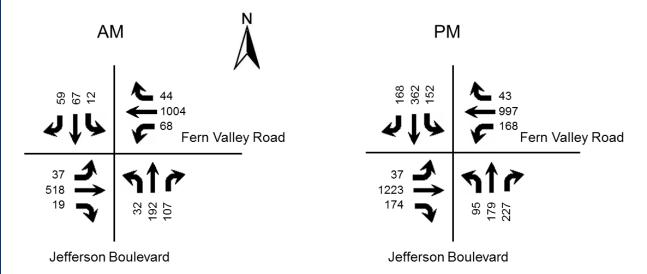


Figure 2 2016 Peak Hour Counts

Future Conditions

The projected completion year for this development is 2017, so the analysis year for this study is 2017. To predict traffic conditions in 2017, one percent annual growth in traffic was added as determined by reviewing historical trends. **Figure 3** displays the 2017 No Build volumes.

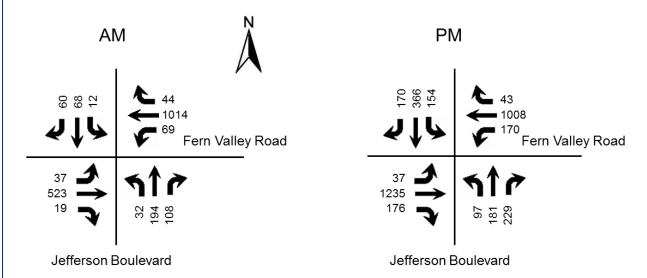


Figure 3 2017 No Build Peak Hour Volumes



Trip Generation

The Institute of Transportation Engineers <u>Trip Generation Manual</u>, 9th Edition contains trip generation rates for a wide range of developments. The land use of "Warehouse (150)" best describes this development. The access point on Fern Valley Road (KY 1747) will be a shared driveway. For this analysis, the adjacent property was assumed to be "Business Park (770)" with 58,250 square feet. The trip generation results are listed in **Table 1**. The results of the trip generation analysis are that this development will generate 244 a.m. peak hour trips and 220 p.m. peak hour trips. The trips were assigned to the highway network with 40 percent to/from the west, 20 percent to/from the east, 20 percent to/from the north and 20 percent to/from the south. **Figure 5** displays the individual turning movements for the year 2017 for the peak hours when the development is completed.

Table 1 - Trip Generation

	AM	Peak Ho	ur	PM	Peak Ho	ur
	Total	Enter	Exit	Total	Enter	Exit
Warehouse (332,500 sq. ft.)	160	126	34	129	32	97
Business Park (58,250 sq. ft.)	84	71	13	91	24	67
TOTAL	244	197	47	220	56	164

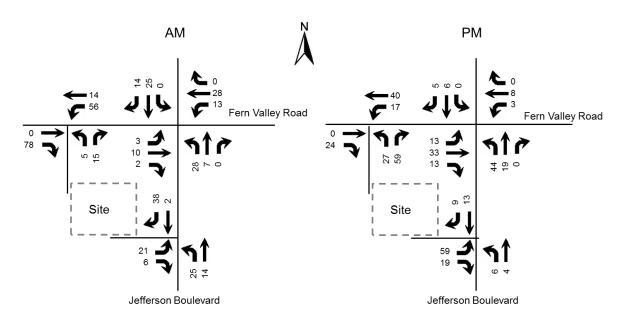


Figure 4
Trip Distribution for Site



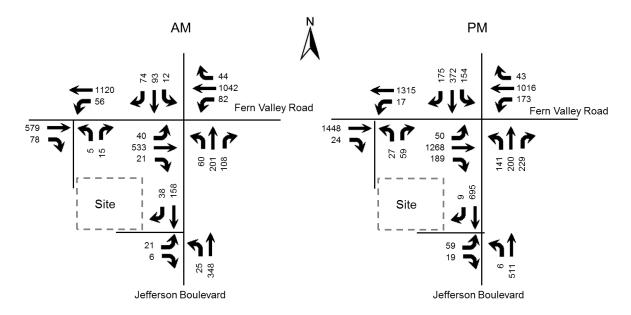


Figure 5 2017 Build Peak Hour Volumes

Analysis

The qualitative measure of operation for a roadway facility or intersection is evaluated by assigning a "Level of Service" or LOS. Level of Service is a ranking scale from A through F with each level representing a range. LOS results depend upon the type of facility that is analyzed. In this case, the LOS is based upon the average vehicle delay each movement experiences at an intersection.

To evaluate the impact of the proposed development, the vehicle delays at the intersection were determined using procedures detailed in the <u>Highway Capacity Manual</u>, 2010 edition. Future delay and Level of Service were determined for the intersection using HCS 2010 Streets and TWSC (version 6.70) and software. **Table 2** shows the results of the analysis for the three scenarios analyzed. The full printouts are included in Appendix B.

Using the Kentucky Transportation Cabinet <u>Auxiliary Turn Lane</u> Policy dated 7/20/2009 and the volumes in **Figure 5**, the volumes do not meet the warrants for an eastbound right turn lane on Fern Valley Road.



Table 2 - Level of Service Results

	Α	M Peak Ho	ur	PI	M Peak Hou	r
	2016 Existing	2017 No Build	2017 Build	2016 Existing	2017 No Build	2017 Build
Fern Valley Road at	С	С	С	D	D	D
Jefferson Boulevard	27.4	27.5	29.4	38.9	39.8	40.8
Fern Valley Road	В	В	В	D	D	D
Eastbound	19.5	19.6	21.7	37.0	38.3	39.3
Fern Valley Road	С	С	С	D	D	D
Westbound	20.3	20.4	23.1	33.4	34.2	35.7
Jefferson Boulevard	D	D	D	D	D	D
Northbound	53.7	53.6	49.8	47.9	48.1	48.5
Jefferson Boulevard	D	D	D	D	D	D
Southbound	54.5	54.5	54.0	46.5	46.7	46.8
Fern Valley Road at						
Entrance						
Fern Valley Road			В			С
Westbound			12.6			24.3
Entrance Northbound			В			D
			13.6			34.8
Jefferson Boulevard at						
Entrance						
Entrance Eastbound			В			С
			10.7			16.8
Jefferson Boulevard			Α			Α
Northbound			7.7			9.3

Note: Level of Service, delay in seconds

Conclusions

Based upon the volume of traffic generated by the development and the amount of traffic forecasted for the year 2017, there will be manageable impacts to the existing highway network. The existing westbound left turn lane on Fern Valley Road will need to meet KYTC requirements.



Appendix A
Traffic Counts

Study Name Fern Valley Rd & Jefferson Blvd Start Date 01/07/2016 Start Time 7:00 AM Site Code

	Southbo	•	proach	Westbo	•	proach	Northbo Northbo		proach	Eastbo	und App und	roach	
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Total
7:00 AM	12	9	10	7	182	14	17	30	6	9	95	7	398
7:15 AM	17	14	0	13	215	7	22	42	5	1	137	9	482
7:30 AM	11	22	5	11	256	21	33	49	10	4	140	7	569
7:45 AM	16	18	1	10	258	24	30	71	15	7	133	13	596
8:00 AM	15	13	6	10	275	16	22	30	2	7	108	8	512
8:15 AM	14	18	4	18	217	9	28	27	10	7	99	12	463
8:30 AM	18	16	3	9	194	11	26	20	6	10	89	8	410
8:45 AM	16	27	6	12	224	15	21	20	11	21	120	7	500
4:00 PM	22	63	23	11	185	37	46	51	27	35	238	12	750
4:15 PM	30	73	16	10	192	50	53	41	28	46	272	14	825
4:30 PM	28	63	21	12	211	53	60	43	19	46	250	10	816
4:45 PM	53	90	39	10	234	38	56	45	31	45	270	7	918
5:00 PM	37	105	26	12	235	61	47	50	26	45	304	10	958
5:15 PM	48	99	35	10	250	47	52	49	28	42	348	8	1016
5:30 PM	30	68	25	11	278	22	62	35	10	42	301	12	896
5:45 PM	29	72	16	11	234	28	59	51	25	54	278	7	864

	Southbo	ound Ap	proach	Westbo	ound Ap	proach	Northbo	ound Ap	proach	Eastboo	und App	roach	
	Sc	outhbour	nd	W	estbour	nd	No	orthbour	nd	Е			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Total
7:15 AM	17	14	0	13	215	7	22	42	5	1	137	9	482
7:30 AM	11	22	5	11	256	21	33	49	10	4	140	7	569
7:45 AM	16	18	1	10	258	24	30	71	15	7	133	13	596
8:00 AM	15	13	6	10	275	16	22	30	2	7	108	8	512
TOTAL	59	67	12	44	1004	68	107	192	32	19	518	37	2159
4:45 PM	53	90	39	10	234	38	56	45	31	45	270	7	918
5:00 PM	37	105	26	12	235	61	47	50	26	45	304	10	958
5:15 PM	48	99	35	10	250	47	52	49	28	42	348	8	1016
5:30 PM	30	68	25	11	278	22	62	35	10	42	301	12	896
TOTAL	168	362	125	43	997	168	217	179	95	174	1223	37	3788



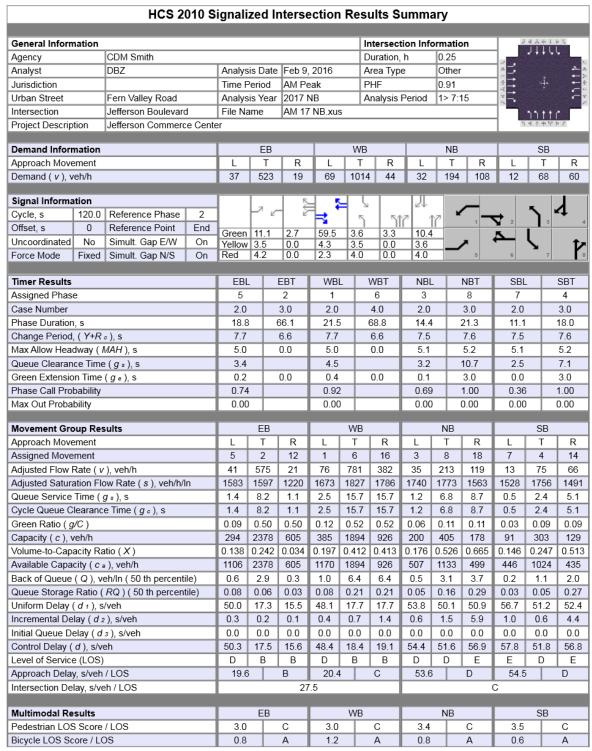
Appendix B HCS Reports

		HCS 2	010 S	ignali	ized l	nters	ection	Res	sults S	umm	ary				
General Inforn	nation	T						\rightarrow	Intersec		_	on	- 6	1111	
Agency		CDM Smith				I=		Duration, h 0.25					- 10		
Analyst		DBZ		-		e Feb 9									===
Jurisdiction				Time F		AM P	eak	\rightarrow	PHF		0.91			₩] F	===
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Project Descrip	tion	Jefferson Commerc	ce Cente	er									ħ	বাক্স	k (1
Demand Inform	nation				EB			WE	3		NB		1	SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v				37	518		68	100	_	32	192	107	12	67	59
Signal Informa		D (D)			La .	_ 8		ן וּ	2	2		_		χ .	4
Cycle, s	120.0	Reference Phase	2			.	➡ 📑	12	1 5	M	17	1	▼ 2	3	4
Offset, s	0	Reference Point	End	Green	11.1	2.6	59.7	3.6	3.3	10.2	2		<u> </u>		
Uncoordinated	No	Simult. Gap E/W	On	Yellow		0.0	4.3	3.5		3.6		~		>	T V
Force Mode	Fixed	Simult. Gap N/S	On	Red	4.2	0.0	2.3	4.0	0.0	4.0		5	6	7	8
Timer Results				EBI		EBT	WB		WBT	NB		NBT	SBI		SBT
Assigned Phas	e			5		2	1	-	6	3		8	7		4
Case Number				2.0		3.0	2.0	+	4.0	2.0		3.0	2.0		3.0
Phase Duration, s					66.3	21.5	-	68.9	14.4	-	21.2	11.1	-	17.8	
Change Period, (Y+R ∘), s				7.7		6.6	7.7		6.6	7.5		7.6	7.5		7.6
Max Allow Hea	dway (/	MAH), s		5.0		0.0	5.0		0.0	5.1		5.2	5.1		5.2
Queue Clearan	ce Time	e (g s), s		3.4			4.4			3.2		10.7	2.5		7.0
Green Extension	Green Extension Time (g_e), s			0.2		0.0	0.4		0.0	0.1		2.9	0.0		2.9
Phase Call Pro	bability			0.74	1		0.92	2		0.69	9	1.00	0.36	3	1.00
Max Out Proba	bility			0.00)		0.00)		0.00)	0.00	0.00)	0.00
Movement Gro	oup Res	sults			EB			WB			NB			SB	
Approach Move	_			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow I	Rate (v), veh/h		41	569	21	75	773	378	35	211	118	13	74	65
Adjusted Satura	ation Flo	ow Rate (s), veh/h/	ln	1583	1597	1220	1673	1827	1786	1740	1773	1563	1528	1756	1491
Queue Service	Time (g ₅), s		1.4	8.1	1.1	2.4	15.5	15.5	1.2	6.7	8.7	0.5	2.4	5.0
Cycle Queue C	learanc	e Time (<i>g c</i>), s		1.4	8.1	1.1	2.4	15.5	15.5	1.2	6.7	8.7	0.5	2.4	5.0
Green Ratio (g	/C)			0.09	0.50	0.50	0.11	0.52	0.52	0.06	0.11	0.11	0.03	0.09	0.09
Capacity (c), v	/eh/h			294	2384	607	384	1898	928	200	402	177	91	300	127
Volume-to-Cap				0.138	0.239	_	0.195	0.408		0.176	0.525	0.664	0.146	0.246	0.510
Available Capa		,,		1109	2384	_	1173	1898	_	507	1133	499	446	1024	435
		eh/ln (50 th percent		0.6	2.9	0.3	1.0	6.3	6.3	0.5	3.1	3.7	0.2	1.1	2.0
		RQ) (50 th percent	tile)	0.08	0.06	0.03	0.08	0.20		0.05	0.16	0.29	0.03	0.05	0.27
Uniform Delay Incremental De				0.3	17.2 0.2	15.4	48.1 0.3	17.6 0.7	17.6	53.8 0.6	50.2 1.5	51.0 6.0	56.7 1.0	51.3 0.6	52.5 4.4
Initial Queue De				0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (•••		50.3	17.4	15.5	48.5	18.2		54.4	51.7	57.0	57.8	51.9	56.9
Level of Service	71			D	B	B	D D	B	B	D	D D	E	57.6 E	D	E
Approach Dela				19.5		В	20.3		C	53.7		D	54.5		D
Intersection De						2	7.4						С		
Multimodal Re		/1.00			EB		WE					NB		SB	
Pedestrian LOS				3.0	-	C	3.0	-	C	3.4	-	C	3.5	_	C
Bicycle LOS So	ore / LC	75		0.8		Α	1.2		Α	0.8		Α	0.6		Α

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Intersection		Jefferson Boulevard	t	File Na	ame	AM 17	B.xus							זורר	r		
Project Descript	tion	Jefferson Commerc	e Cente	er									h	াৰ 1 ቀ শ	* I		
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Approach Move	ment			L	Т	R	L	Т	R	L	Т	R	L	Т	R		
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Uncoordinated			On	Yellow		0.0	4.3	3.5	0.0	3.6		^	`	/	- P		
Force Mode	Fixed	Simult. Gap N/S	On	Red	4.2	0.0	2.3	4.0	0.0	4.0		5	6	7	8		
Timer Results				EBI		EBT	WB	L	WBT	NBI	L	NBT	SBI		SBT		
Assigned Phase	·			5		2	1		6	3		8	7		4		
Case Number				2.0		3.0	2.0		4.0	2.0		3.0	2.0		3.0		
Phase Duration	, s			19.2	2	62.6	22.0		65.3	16.4	1	24.4	11.1		19.1		
Change Period,	(Y+R	c), S		7.7		6.6	7.7		6.6	7.5		7.6	7.5		7.6		
Max Allow Head	Max Allow Headway (MAH), s			5.0		0.0			0.0 5.1		\neg	5.2	5.1	\neg	5.2		
Queue Clearand	ax Allow Headway (<i>MAH</i>), s ueue Clearance Time (<i>q</i> s), s			3.5			4.9			4.1		10.5	2.5		8.3		
	ueue Clearance Time ($g _{s}$), s een Extension Time ($g _{e}$), s			0.2	-	0.0	0.5 0.		0.0	0.2	-	3.3	0.0	-	3.2		
	een Extension Time (g e), s ase Call Probability			0.77	7		0.95	5		0.89	9	1.00	0.36	3	1.00		
Max Out Probab				0.00	-		0.00	-		0.00	-	0.00	0.00	-	0.01		
					- ED			WD			ND			OD			
		uits			EB	I D		WB T	T D		NB	_ D		SB	П		
				L	T	R	L	_	R	L	T	R	L	T	R		
				5	2	12	1	6	16	3	8	18	7	4	14		
				44	586	23	90	801	392	66	221	119	13	102	81		
			n	1583	1597	1220	1673	1827	_	1740	1773	1563	1528	1756	1491		
				1.5	8.9	1.2	2.9	17.2		2.1	6.9	8.5	0.5	3.3	6.3		
-		e ⊓me (<i>g ₀</i>), s		1.5	8.9	1.2	2.9	17.2	17.2	2.1	6.9	8.5	0.5	3.3	6.3		
Green Ratio (g/				0.10	0.47	0.47	0.12	0.49	0.49	0.07	0.14	0.14	0.03	0.10	0.10		
Capacity (c), v				304	2235	569	398	1787		258	497	219	91	337	143		
				0.144	0.262	-	0.227	0.448		0.256	0.444	0.541	0.146	0.304	0.569		
			ilo)	1024 0.6	2235 3.2	569 0.4	1082	1787 7.1	7.2	1.0	1133 3.1	499 3.5	0.2	966	0.2		
				0.08	0.07	0.4	0.09	0.23	0.23	0.09	0.16	0.27	0.2	0.07	0.2		
			iiie)	49.7	19.5	17.4	47.9	20.1	20.1	52.4	47.3	48.0	56.7	50.5	51.9		
- ,				0.3	0.3	0.1	0.4	0.8	1.7	0.7	0.9	2.9	1.0	0.7	5.0		
	- '	**		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
	- , ,			50.0	19.8	17.6	48.3	20.9	-	53.2	48.2	50.9	57.8	51.2	56.9		
Level of Service				D	13.6 B	B	D D	C C	C	D	D T	D	57.6 E	D D	E		
	. ,			21.7		С	23.1		C	49.8		D	54.0		D		
							9.4						С				
Multimodal Res	sults				EB			WB			NB			SB			
				3.0		С	3.0		С	3.4		С	3.5		С		
Bicycle LOS Sc	ore / LC)S		0.8		Α	1.2		Α	0.8		Α	0.6		Α		

HCS 2010™ Streets Version 6.70

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General Inform	nation								Intersect	tion Infe	ormatic	n		14441	
Agency		CDM Smith						<u> </u>	Duration,	h	0.25			httr	٠ _
Analyst		DBZ		Analys	sis Date	Feb 9	2016								
Jurisdiction				Time Period PM Peak					PHF		0.93		→	₩ j F	÷
Urban Street		Fern Valley Road		-	sis Year	-			Analysis	Period	1> 4:4	1 5			
Intersection		Jefferson Boulevard	t	File Na		PM 16	3.xus				-			55++	, _
Project Descrip	tion	Jefferson Commerc											- B	4147	1- 1
							1	14/5							
Demand Inforr					EB			WE	_	-	NB		-	SB	_
	proach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), v	eh/h			37	1223	174	168	997	7 43	95	179	217	125	362	16
Signal Informa	tion				Т	1 8		J							Т
Cycle, s	120.0	Reference Phase	2	1	7 6	- 2	₹, 1	٦ ,	7 547	124	N21	_ _	→	\	4
Offset, s	0	Reference Point	End	<u></u>	44.0	100	44.0	1,2		100.0		1	2	3	
Uncoordinated	No	Simult. Gap E/W	On	Green Yellow		0.0	41.9	9.7	0.2	23.8 3.6		,	4		1
Force Mode	Fixed	Simult. Gap N/S	On	Red	4.2	0.0	2.3	4.0	0.0	4.0		5	6	7	
Timer Results				EBI	-	EBT	WB	L	WBT	NBI	-	NBT	SBL	-	SBT
Assigned Phase	е			5		2	1		6	3		8	7		4
Case Number				2.0		3.0	2.0		4.0	2.0		3.0	2.0		3.0
Phase Duration, s				18.7 4		48.5	22.7	<u>' </u>	52.5	17.2	2	31.4	17.4	Į .	31.7
Change Period, (Y+R ∘), s				7.7		6.6	7.7 6.6		7.5		7.6	7.5		7.6	
Max Allow Headway (MAH), s				5.0		0.0	5.0		0.0	5.1		5.2	5.1		5.2
Queue Clearan	ce Time	e (g s), s		3.3			7.8			5.4		18.7	7.0		14.7
Green Extension	n Time	(ge), s		0.1		0.0	0.9		0.0	0.3		5.2	0.5		5.9
Phase Call Pro	bability			0.73	3		1.00)		0.97		1.00	0.99)	1.00
Max Out Proba	bility			0.00)		0.00)		0.00		0.35	0.03	3	0.20
Movement Gro	un Das	ulte			EB			WB			NB			SB	
Approach Move	_	Juita		L	T	R	L	T	R	L	T	R	L	T	TR
Assigned Move				5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		·) veh/h		40	1315	187	181	751	368	102	192	233	134	389	18
		ow Rate (s), veh/h/l	n	1740	1675	1388	1723	1827	-	1723	1809	1579	1555	1809	154
Queue Service				1.3	27.7	12.2	5.8	19.2		3.4	5.4	16.7	5.0	11.6	12.
Cycle Queue C				1.3	27.7	12.2	5.8	19.2	-	3.4	5.4	16.7	5.0	11.6	12.
Green Ratio (g		5 .and (gt), 3		0.09	0.35	0.35	0.12	0.38	_	0.08	0.20	0.20	0.08	0.20	0.2
Capacity (c), v				319	1755	485	430	1396	-	278	718	314	256	725	31
Volume-to-Cap		atio (X)		0.125	0.749	0.386	0.420	0.538		0.368	0.268	0.744	0.525	0.537	0.5
Available Capa				739	1755	485	732	1396	-	502	965	421	454	972	41
		eh/ln (50 th percent	ile)	0.5	11.2	4.2	2.5	8.3	8.5	1.5	2.4	7.0	2.0	5.2	5.0
		RQ) (50 th percent		0.07	0.24		0.18	0.27		0.14	0.12	0.55	0.28	0.26	0.6
Uniform Delay			0,	50.1	34.4	29.4	48.5	28.8		52.3	40.7	45.2	52.8	43.0	43.
Incremental De				0.2	3.0	2.3	0.9	1.5	3.0	1.2	0.3	6.1	2.4	0.9	2.
Initial Queue De				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (•••		50.3	37.4	31.7	49.4	30.3	-	53.4	41.0	51.3	55.2	43.9	45.
Level of Service				D	D	C	D	C	C	D	D	D	E	D	D
Approach Delay				37.0		D	33.4		c	47.9		D	46.5		D
Intersection De							3.9						D		
Multimodal Re	sults				EB			WB			NB			SB	
Pedestrian LOS	Score	/ LOS		3.1		С	3.1		С	3.4		С	3.5		С
	ore / LO	26		1.3		Α	1.2		Α	0.9		Α	1.1		Α

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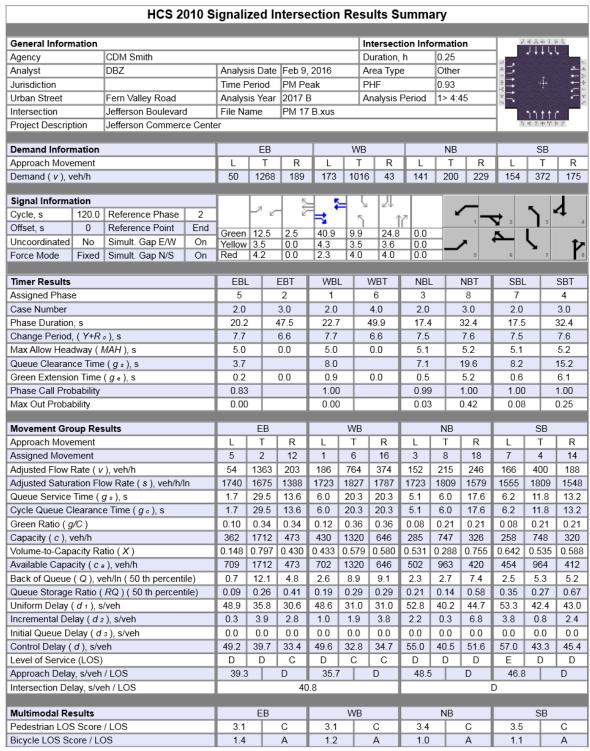


	HCS 2	010 S	ignali	zed l	nterse	ection	Res	ults S	umm	ary				
General Information							1	ntersect	tion Inf	ormatic	n		1	
Agency	CDM Smith						[Duration,	h	0.25			••••	
Analyst	DBZ		Analys	sis Date	Feb 9	2016	A	Area Typ	е	Other		<u> </u>		÷
Jurisdiction			Time F	Period	PM Pe	eak	F	PHF		0.93		\$ -	₩ Î F	= =
Urban Street	Fern Valley Road		Analys	sis Year	2017	NB	1	Analysis	Period	1> 4:4	1 5	¥ = .		7
Intersection	Jefferson Boulevard	d	File Na	ame	PM 17	NB.xu	3						11 tt	ď
Project Description	Jefferson Commerc	e Cente	er									ħ	বাক্স	11
Demand Information				EB			WB			NB			SB	
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h			37	1235		170	100		97	181	229	154	366	170
Signal Information	D-f Dh			L .	- 5	╡.३	ا إ	. 211	2		_		τ.	4
Cycle, s 120.0		2		L 2	"	₹	5				1	7 2	3	4
Offset, s 0	Reference Point	End	Green		3.9	41.0	9.7	0.3	24.7	7		لک		
Uncoordinated No	Simult. Gap E/W	On	Yellow		0.0	4.3	3.5	0.0	3.6		^		/	1
Force Mode Fixed	Simult. Gap N/S	On	Red	4.2	0.0	2.3	4.0	0.0	4.0		5	6	7	8
Timer Results			EBI	.	EBT	WB		WBT	NBI		NBT	SBL		SBT
Assigned Phase			5		2	1	\neg	6	3		8	7		4
Case Number			2.0		3.0	2.0		4.0	2.0		3.0	2.0		3.0
Phase Duration, s			18.7	7	47.6	22.7	·	51.6	17.2	2	32.3	17.5		32.5
Change Period, (Y+R	c), s		7.7		6.6	7.7		6.6	7.5		7.6	7.5		7.6
Max Allow Headway (MAH), s		5.0		0.0	5.0		0.0	5.1		5.2	5.1		5.2
Queue Clearance Tim	e (g₅), s		3.3			7.9			5.4		19.6	8.2		14.7
Green Extension Time	e (g e), s		0.1		0.0	0.9		0.0	0.4		5.0	0.6		6.0
Phase Call Probability			0.73	3		1.00			0.97	7	1.00	1.00		1.00
Max Out Probability			0.00			0.00			0.00)	0.41	0.08		0.22
Movement Group Re	sults			EB			WB			NB			SB	
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	T	R
Assigned Movement			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (/) veh/h		40	1328	189	183	759	371	104	195	246	166	394	183
Adjusted Saturation FI	••	ln	1740	1675	1388	1723	1827	1787	1723	1809	1579	1555	1809	1548
Queue Service Time (1.3	28.4	12.5	5.9	19.7	19.7	3.4	5.4	17.6	6.2	11.6	12.7
Cycle Queue Clearand			1.3	28.4	12.5	5.9	19.7	19.7	3.4	5.4	17.6	6.2	11.6	12.7
Green Ratio (g/C)	(3-71-		0.09	0.34	0.34	0.12	0.37	0.37	0.08	0.21	0.21	0.08	0.21	0.21
Capacity (c), veh/h			319	1718	474	430	1369	670	278	743	324	258	751	322
Volume-to-Capacity R	atio (X)		0.125	0.773	0.399	0.425	0.554	_	0.375	0.262	0.759	0.642	0.524	0.568
Available Capacity (c	· · ·		713	1718	474	706	1369	670	502	963	420	454	971	416
Back of Queue (Q), v	**	ile)	0.5	11.6	4.4	2.5	8.6	8.7	1.5	2.4	7.4	2.5	5.2	5.0
Queue Storage Ratio	(RQ) (50 th percen	tile)	0.07	0.25	0.38	0.18	0.28	0.28	0.14	0.12	0.58	0.35	0.26	0.65
Uniform Delay (d 1),	s/veh		50.1	35.3	30.1	48.5	29.6	29.6	52.3	40.0	44.9	53.3	42.3	42.7
Incremental Delay (d	2), s/veh		0.2	3.5	2.5	1.0	1.6	3.3	1.2	0.3	7.0	3.8	0.8	2.2
Initial Queue Delay (a	1 ₃), s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/v	/eh		50.3	38.8	32.6	49.5	31.2	32.9	53.5	40.3	51.9	57.0	43.1	44.9
Level of Service (LOS)		D	D	С	D	С	С	D	D	D	E	D	D
Approach Delay, s/veh	n/LOS		38.3	3	D	34.2		С	48.1		D	46.7		D
Intersection Delay, s/v	eh / LOS				39	9.8						D		
				EB			WB			NB			SB	
Multimodal Results														
Multimodal Results Pedestrian LOS Score	2/LOS		3.1		С	3.1	_	С	3.4	_	С	3.5	_	С

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	HCS 2010 Two-Way Stop	Control Summary R	eport
General Information		Site Information	
Analyst	DBZ	Intersection	Fern Valley Entrance
Agency/Co.	CDM Smith	Jurisdiction	
Date Performed	2/11/2016	East/West Street	Fern Valley Road
Analysis Year	2017	North/South Street	Entrance
Time Analyzed	AM Peak	Peak Hour Factor	0.91
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Fern Valley Commerce Center		
Lanes			

Vehicle Volumes and Adjustments

	•															
Approach		Eastb	oound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	3	0	0	1	3	0		1	0	1		0	0	0
Configuration			Т	TR		L	Т			L		R				
Volume (veh/h)			579	78		56	1120			5		15				
Percent Heavy Vehicles						1				1		1				
Proportion Time Blocked																
Right Turn Channelized		Ν	٧o			١	lo			Ν	lo			٨	lo	
Median Type								Left	Only							
Median Storage									1							

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)			62			5		16		
Capacity			535			254		545		
v/c Ratio			0.12			0.02		0.03		
95% Queue Length			0.4			0.1		0.1		
Control Delay (s/veh)			12.6			19.4		11.8		
Level of Service (LOS)			В			С		В		
Approach Delay (s/veh)			0	.6		13	3.6			
Approach LOS			,	4			В			

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	HCS	5 201	LO Tw	o-W	ay St	top C	ontro	ol Su	mma	ry Re	eport					
General Information							Site 1	inforn	nation							
Analyst	DBZ						Inters	ection			Fern \	/alley En	trance			
Agency/Co.	CDM :	Smith					Jurisd	iction								
Date Performed	2/11/2	2016					East/V	Vest Stre	et		Fern \	/alley Ro	ad			
Analysis Year	2017						North	/South S	Street		Entrai	nce				
Time Analyzed	PM Pe	ak					Peak I	Hour Fac	tor:		0.93					
Intersection Orientation	East-V	Vest					Analy	sis Time	Period (h	nrs)	0.25					
Project Description	Fern V	alley Co	mmerce	Center												
Lanes																
				0744717		ገ በ ቀ Y or Street Ea	t P (*)	++++								
Vehicle Volumes and A	djustmen															
Approach	-		oound				bound 	_			bound				bound	
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority Number of Lanes	1U 0	0	2	3	4U 0	1	5 3	6 0		7	8	9		10	11 0	12
	0	0	T T	TR	0	L	3 T	0		L	0	_		0	"	H
Configuration Volume (veh/h)			1448	24		17	1315			27		R 59				
			1440	24	-	1	1313			1		1				-
Percent Heavy Vehicles Proportion Time Blocked						1				1		1				
Right Turn Channelized			l				lo				lo lo				l	
Median Type			•••					Left	Only		•••				•••	
Median Storage									1							
Delay, Queue Length, a	nd Level	of Se	rvice						-							
	III LEVEI	01 30	1							_						_
Flow Rate (veh/h)						18				29		63				
Capacity						205				88		287				
v/c Ratio						0.09				0.33		0.22				
95% Queue Length						0.3				1.3		0.8				
Control Delay (s/veh)						24.3				64.7		21.1				
Level of Service (LOS)						С				F		С				
Approach Delay (s/veh)						0	.3			34	4.8					

Approach LOS

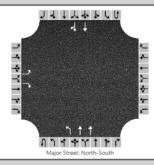
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	HCS 2010 Two-Way Stop C	ontrol Summary Re	eport
General Information		Site Information	
Analyst	DBZ	Intersection	Jefferson Boulevard Entra
Agency/Co.	CDM Smith	Jurisdiction	
Date Performed	2/11/2016	East/West Street	Entrance
Analysis Year	2017	North/South Street	Jefferson Boulevard
Time Analyzed	AM Peak	Peak Hour Factor	0.91
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Fern Valley Commerce Center	-	

Lanes



Vehicle Volumes and Adjustments

Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6
Number of Lanes		1	0	1		0	0	0	0	1	2	0	0	0	2	0
Configuration		L		R						L	Т				Т	TR
Volume (veh/h)		21		6						25	348				158	38
Percent Heavy Vehicles		1		1						1						
Proportion Time Blocked																
Right Turn Channelized		N	lo			N	lo			Ν	lo			1	lo	
Median Type								Left	Only							
Median Storage									1							

Delay, Queue Length, and Level of Service

Flow Rate	(veh/h)	23		7			27				
Capacity		598		928			1359				
v/c Ratio		0.04		0.01			0.02				
95% Que	ue Length	0.1		0.0			0.1				
Control D	elay (s/veh)	11.3		8.9			7.7				
Level of S	ervice (LOS)	В		А			А				
Approach	Delay (s/veh)	10	0.7				0	.5			
Approach	LOS		В				,	Д			

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	HCS	S 20 1	0 Tw	vo-W	ay St	op <u>C</u>	ontro	ol Su	mma	ry Re	eport					
General Information							Site I	inforn	nation							_
Analyst	DBZ						Inters	ection			Jeffer	on Boul	evard En	tra		
Agency/Co.	CDM S	Smith					Jurisd	iction								
Date Performed	2/11/2	2016					East/V	Vest Stre	et		Entrar	nce				
Analysis Year	2017						North	/South S	Street		Jeffer:	on Boul	evard			
Time Analyzed	PM Pe	ak					Peak I	Hour Fac	tor		0.91					
Intersection Orientation	North	-South					Analy	sis Time	Period (h	nrs)	0.25					
Project Description	Fern V	alley Co	mmerce	Center												
Lanes																
				741747 76												
Vehicle Volumes and A	diustmen	ts				T T Y										
Vehicle Volumes and A	djustmen		ound			Street: No	th-South		I	North	bound			South	bound	
Vehicle Volumes and A Approach Movement	djustmen		ound	R		Street: No		R	U	North	bound	R	U	South	bound T	R
Approach Movement		Eastb	_	R 12	Major	Street: Noi	oound	R 9	U	_	_	R 3	U 4U			-
Approach		Eastb	Т	_	Major	Westl	oound T	_		L	Т			L	Т	6
Approach Movement Priority		Eastb L 10	T 11	12	Major	Westl	oound T	9	10	L 1	T 2	3	4U	L 4	T 5	6
Approach Movement Priority Number of Lanes		Eastb L 10	T 11	12	Major	Westl	oound T	9	10	1 1	T 2 2	3	4U	L 4	T 5	R 6 0 TR 9
Approach Movement Priority Number of Lanes Configuration		Eastb L 10 1	T 11	12 1 R	Major	Westl	oound T	9	10	1 1 L	T 2 2 T	3	4U	L 4	T 5 2 T	6 0 TR
Approach Movement Priority Number of Lanes Configuration Volume (veh/h)		Eastb L 10 1 L	T 11	12 1 R 19	Major	Westl	oound T	9	10	L 1 1 L 6	T 2 2 T	3	4U	L 4	T 5 2 T	6 0 TR
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles		Eastb L 10 1 L 59	T 11	12 1 R 19	Major	Westi	oound T	9	10	L 1 1 L 6 1	T 2 2 T	3	4U	4 0	T 5 2 T	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked		Eastb L 10 1 L 59	T 11 0	12 1 R 19	Major	Westi	oound T 8 0	9	10	L 1 1 L 6 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TR
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized		Eastb L 10 1 L 59	T 11 0	12 1 R 19	Major	Westi	oound T 8 0	9 0	10 0	L 1 1 L 6 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage	U	Eastb L 10 1 L 59 1	T 11 0	12 1 R 19	Major	Westi	oound T 8 0	9 0	1U 0	L 1 1 L 6 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage	U	Eastb L 10 1 L 59 1	T 11 0	12 1 R 19	Major	Westi	oound T 8 0	9 0	1U 0	L 1 1 L 6 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, a	U	Eastb L 10 1 L 59 1	T 11 0	12 1 R 19 1	Major	Westi	oound T 8 0	9 0	1U 0	L 1 1 L 6 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, a	U	Eastb L 10 1 L 59 1 N of Ser	T 11 0	12 1 R 19 1	Major	Westi	oound T 8 0	9 0	1U 0	L 1 1 L 6 1 1	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, a Flow Rate (veh/h) Capacity	U	Eastb L 10 1 L 59 1 N 65 329	T 11 0	12 1 R 19 1	Major	Westi	oound T 8 0	9 0	1U 0	L 1 1 L 6 1 1 N N N N N N N N N N N N N N N N N	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF
Approach Movement Priority Number of Lanes Configuration Volume (veh/h) Percent Heavy Vehicles Proportion Time Blocked Right Turn Channelized Median Type Median Storage Delay, Queue Length, a Flow Rate (veh/h) Capacity v/c Ratio	U	Eastb L 10 1 L 59 1 N N 65 65 329 0.20	T 11 0	12 1 R 19 1 1	Major	Westi	oound T 8 0	9 0	1U 0	1 1 1 6 1 7 844 0.01	T 2 2 T 511	3	4U	4 0	T 5 2 T 695	6 0 TF

16.8

Approach Delay (s/veh)

Approach LOS

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